Town of Hopkinton Planning Department



330 Main Street, Hopkinton NH 03229-2627 - (603) 746-8243 -planzone@hopkinton-nh.gov

HOPKINTON PLANNING BOARD <u>PUBLIC NOTICE – AGENDA</u> AUGUST 10, 2021

Notice is hereby given that the **Hopkinton Planning Board** will meet on Tuesday, August 10, 2021, at 6:00 PM in the Hopkinton Town Hall, 330 Main Street, Hopkinton and via Zoom through the website: <u>https://us06web.zoom.us/j/98763598862</u> or by dialing the following phone #:1-929-205-6099 and using Meeting ID: 987 6359 8862. The Planning Board will review and take action on the following:

- I. Call to Order
- II. Review of Meeting Minutes and Notice of Decision of June 8 and July 13, 2021.
- III. Conceptual Consultations.
- **IV.** Applications/Public Hearings (Determine Complete, Public Hearing, Deliberation and Action on Application).

#2021-21 T.F. Bernier, Inc. Lot Line Adjustment between Lot 51 owned by John H. Lynch Irrevocable Trust of 2012 and Lot 52 owned by Rix Family Trust of 2016, referenced on Tax Map 240, located off Gould Hill Road, R-2 district.

#2021-20 Baystone Properties, LLC Site Plan Review, Architectural Design Review, and Condominium Subdivision for the construction of 12 attached, townhouse-style, residential units at 71 Cedar Street, Tax Map 102, Lot 35, VR-1 district.

V. Other Business.

- a) Discussion on zoning amendments for 2021
- b) Report on Master Plan update.
- c) Any other business to legally come before the meeting.
- VI. Adjournment (Next regular meeting on Tuesday, September 14, 2021).

Application #2021-21

T. F. BERNIER, INC. Lot Line Adjustment – Lynch and Rix.

Town of Hopkinton Planning Department



330 Main Street, Hopkinton NH 03229-2627 - (603) 746-8243 -planzone@hopkinton-nh.gov

Date:	July 29, 2021
To:	Planning Board
From:	Karen Robertson, Planning Director
Re:	T. F. Bernier, Inc. – Lot Line Adjustment (PB #2021-21)

The Applicant is seeking a Lot Line Adjustment (LLA) between property owned by John H. Lynch Irrevocable Trust of 2012, known as Lot 51, and the abutting property owned by the Rix Family Trust of 2016, known as Lot 52. The proposed LLA will allow for the conveyance of .007 acres (321 SF) of lot area with fourteen feet (14') of frontage from the Rix property to be transferred and combined with the existing +/-155-acre Lynch property. The subject properties are off Gould Hill Road in the R-2, Medium Density Residential zoning district.

Following the LLA, the Lynch property (Lot 51) will contain +/-155-acres with 750 feet of contiguous frontage, and the Rix property (Lot 52) will contain 23.7 acres with 762 feet of contiguous frontage along Gould Hill Road.

The Applicant is requesting waivers from the following: Subdivision Regulations 3.3.1(b) natural features, 3.3.1(c) contours, 3.3.1(d) surveyed exterior lines, 3.3.1(h) soil locations, 3.3.2(a) property lines in feet and decimals, and Zoning Ordinance 12.4 delineation of wetlands.

To better understand the size of the properties, I have included an aerial showing the lots. Note: The aerial does not exclude that area of the Lynch property that, in March 2021, was annexed to Lot 49.

Planning Board Motions:

- 1. Acceptance Motion: I move that the request of T. F. Bernier, Inc. (PB #2021-21) for a Lot Line Adjustment be ACCEPTED as complete and for consideration.
- Lot Line Adjustment Motion: I move that the request of T. F. Bernier, Inc. (PB #2021-21) for a Lot Line Adjustment be APPROVED / APPROVED WITH THE FOLLOWING CONDITIONS / TABLED / DENIED.





T.F. BERNIER, INC.

Land Surveyors~Designers~Consultants

Environmental Permitting State and Local Permitting Land Surveying Aerial Mapping Aerial Photography

50 Pleasant Street, P.O. Box 3464 Concord, NH 03302-3464 Tel. (603) 224-4148 Fax (603) 224-0507

July 14, 2021

Bruce Ellsworth, Chair Hopkinton Planning Board 330 Main Street Hopkinton, NH 03229

RE: Application for Lot Line Adjustment Land of John H. Lynch Irrevocable Trust of 2012 and Rix Family Trust of 2016 Assessors Map 240 Lots 51 & 52

Dear Chair Ellsworth and Members of the Board:

Please find enclosed the application for a Lot Line Adjustment between Lot 51 and 52. The lot lines between the two parcels will be adjusted such that an area of 321 square feet will be transferred from Lot 52 to Lot 51. There are no new lots proposed and there is no new development proposed. Lot 52 will be approximately 23.7 acres and Lot 51 will be 155 acres after the adjustment. State subdivision approval is not required.

As a part of this application, we are requesting waivers of the following subdivision checklist items:

- Natural features, water bodies, tree lines, vegetation, topography etc.
- Contours.
- Surveyed exterior property lines.
- Wetlands.
- Soils locations.
- Location and dimensions of property lines in feet and decimals.

Reason for waiver requests: The lot line adjustment is fairly simple, no new lots are created, no development is proposed and the lots are both quite large after the adjustment. A portion of the lots was surveyed in the area of the adjustment. The remaining boundaries of the lots are from plans of record and deed descriptions or Town GIS information.

Thank you for your time and consideration of this request. If you have any questions or need additional information, please give us a call.

Sincerely T.F. BERNIER, INC. imothy F. Bernier, PLS LLS CWS President

enclosures

cc: file 663-01



Town of Hopkinton

330 Main Street • Hopkinton, New Hampshire 03229 • www.hopkinton-nh.govTel: 603-746-3170Fax: 603-746-3049

PLANNING BOARD APPLICATION

PROJECT LOCATION: Gould Hi	ill Road	MAP/LOT: 240	/51	240 / 52	ZONE: R-2
APPLICANT: T.F. Bernier, Inc -	Timothy Bernier				
Address: P.O. Box 3464	Maily on the	City: Concord	State: NI	H Zip:	03302
Phone: 603-224-4148		Email: tim@tfbinc.com	1	and see	110 Page 4
OWNER(s) _{Name:} Rix Family Trust of 2016	, Erica and Robe	rt Rix Trustees			
Address: 248 Gould Hill Road	and of a select	City: Hopkinton	State: NI	H Zip:	03229
and the first state of the first		Email:			
Phone:					
Phone: Name: John H. Lynch Irrevocable	e Trust of 2012, S	usan E. Upton Lynch & Willia	am G Stee	le, Jr CPA-T	rustees
Name: John H. Lynch Irrevocable	e Trust of 2012, S	usan E. Upton Lynch & Willia			
Phone: Name:_John H. Lynch Irrevocable Address: 2 Watchtower Road Phone:	e Trust of 2012, S	usan E. Upton Lynch & Willia City: Hopkinton	_ State: N	H Zip:	
Name: John H. Lynch Irrevocable Address: 2 Watchtower Road Phone: PROFESSIONALS (engineer, arc	e Trust of 2012, S chitect, surveyor, a	usan E. Upton Lynch & Willia City: <u>Hopkinton</u> Email:	State: <u>Ni</u> t, etc.):	HZip:_	
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- Application checklist.
- Planning Board/Zoning Board of Adjustment Minutes of Conceptual, Preliminary Review, or approval.
- Property deed and existing/proposed easements, covenants, and restrictions.
- Tax Map of subject parcel and abutting properties.
- Waiver(s) request from provisions of the Subdivision and/or Site Plan Regulations.

	Test Pit Data, Storm Water Management Plan, Traffic, School, Environmental and Fiscal Impact Analyses, and Phasing Plan (when applicable).
	Abutters List as defined by RSA 672:3 – Include Tax Map, Lot Number, Name and Mailing Addresses. If abutting property is under a condominium or other collective form of ownership, the term "abutter" means the officers of the collective or association. If abutting property is under a manufactured housing park form of ownership, the term "abutter" includes the manufactured housing park owner and the tenants who own the manufactured housing.
	Mailing labels – Include Applicant, Owner, Architect, Soil/Wetland Scientists, Abutters, and holders of Conservation/Preservation restrictions or easements.
	Four (4) paper prints of the plan(s) at full scale.
	Appropriate Filing Fee: (Non-refundable) Made payable to Town of Hopkinton Major Subdivision
	Site Plan Review
	Public Notice Mailing
	Newspaper Notice
Fii	Conditional Use Permit (Wireless Telecommunications): If application is for Conditional Use Permit, please attach a detailed explanation of compliance with Section 3.10 of the Hopkinton Zoning Ordinance, along with an explanation of compliance with the Site Plan Review Regulations of the Town of Hopkinton. Conditional Submission Requirements (after Planning Board action): Four (4) paper prints of the final plan set at full scale.
	Mylar(s) – The Merrimack County Registry of Deeds requires that the UPPER LEFT-HAND CORNER, INSIDE THE BORDER, of the plat to be RESERVED for recording information entered by the Registry - No smaller than 7" long X 1" wide.
	PDF of the final plan set, including architectural and site photographs - emailed or thumb drive.
	Recording Fees: (Separate Checks) Made payable to Merrimack County Registry of Deeds Recording Fee\$ 26.00 per Page (22" x 34") LCHIP Fee\$ 25.00 per Document
egulat as grar nspect vork o	Sent to the best of my knowledge and belief that this application is being submitted in accordance with applicable tions and ordinances of the Town of Hopkinton. I also understand that submittal of this application shall be deemed ating permission for the Planning Board members and their designees to enter onto the property for purposes of ions and review. Permission to visit the property extends from the date an application is submitted until approved r construction is complete and any or all of the financial guarantees, if any, have been returned to the applicant, or e application is formally denied. Furthermore, I agree that the proposed project will be performed in accordance s application, the attached plans and specifications, and the regulations and ordinances of the Town of Hopkinton.

Application Filed: 1/15/21	Fees: 305 / 1309 Application #:	
Notice(s) Posted/Mailed:	Complete/Consideration:	
Meeting(s)/Hearing(s):		
Approved/Denied:	Conditions MCRD Filing:	
MCRD Document #:		

PLANNING BOARD APPLICATION

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	Mailing labels – Include Applicant, Owner, Architect, Soil/Wetland Scientists, Abutters, and holders of Conservation/Preservation restrictions or easements.
	Four (4) paper prints of the plan(s) at full scale.
	Major Subdivision
	Site Plan Review
	Conditional Use Permit
-	Total= \$385.00
	Conditional Use Permit (Wireless Telecommunications): If application is for Conditional Use Permit, please attach a detailed explanation of compliance with Section 3.10 of the Hopkinton Zoning Ordinance, along with an explanation of compliance with the Site Plan Review Regulations of the Town of Hopkinton.
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regulati as gran inspect work or until the with this Applic	Sent to the best of my knowledge and belief that this application is being submitted in accordance with applicable ions and ordinances of the Town of Hopkinton. I also understand that submittal of this application shall be deemed iting permission for the Planning Board members and their designees to enter onto the property for purposes of ions and review. Permission to visit the property extends from the date an application is submitted until approved construction is complete and any or all of the financial guarantees, if any, have been returned to the applicant, or e application is formally denied. Furthermore, agree that the proposed project will be performed in accordance is application, the attached plans and specifications, and the resulations and ordinances of the Town of Hopkinton. Int's Signature: Signature: Bate: <u>7/15/21</u> Bate: <u>7/15/21</u> Just: <u>7/15/21</u>

		Office Use:
Application Filed:	_ Fees:	Application #:
Notice(s) Posted/Mailed:		Complete/Consideration:
Meeting(s)/Hearing(s):		
Approved/Denied:		Conditions MCRD Filing:
MCRD Document #:		

Return To:

n- 1

Mr. & Mrs. Robert Rix, Trustees Rix Family Trust 248 Gould Hill Road Hopkinton, NH 03229

WARRANTY DEED

Transfer Tax: \$5250

KNOW ALL MEN BY THESE PRESENTS, That, Peter N. Martin of 106 North Main Street, Newport, NH 03773 and Mark W. Martin of 73 West Shore Road, Munsonville, NH 03457 and Lee J. Martin of P. O. Box 317, Henniker, NH 03242 and Peter N. Martin as Trustee of the Philip C. and Gloria W. Martin Revocable Family Trust of 2016, with a mailing address of 106 North Main Street, Newport, NH 03773, for consideration paid grants to Erica C. Rix and Robert D. Rix, Trustees of the Rix Family Trust of 2016, under trust dated March 11, 2016, with a mailing address of 32 Dwinell Drive, Concord, NH 03301, with WARRANTY COVENANTS:

Two (2) certain tracts or parcels of land situate in Hopkinton, County of Merrimack and State of New Hampshire, bounded and described as follows:

TRACT 1:

A certain tract of land situated on the easterly side of Gould Hill Road in Hopkinton, County of Merrimack and State of New Hampshire, bounded and described as follows:

Beginning at a stone wall on the easterly line of Gould Hill Road, said stone wall being on the southerly line of land now or formerly of one Gage; thence running

Warranty Deed Peter N. Martin, Mark W. Martin, Lee J. Martin, and Philip C. & Gloria W. Martin Rev. Family Trust of 2008 to Rix Family Trust Page 1 of 3

D

southeasterly by said stone wall a distance of eighteen hundred fifty (1,850) feet, more or less to a stone bound at land now or formerly of Tyrus C. Houston and Eveline M. Houston; thence turning and running southerly by land now or formerly of Tyrus C. Houston and Eveline M. Houston a distance of five hundred (500) feet to a stone bound; thence turning and running westerly by land now or formerly of Tyrus C. Houston and Eveline M. Houston a distance of sixteen hundred (1,600) feet to a stone bound on the easterly line of said Gould Hill Road; thence turning and running northerly by the easterly line of said Gould Hill Road a distance of seven hundred (700) feet to the bound begun at.

TRACT 2:

A certain lot or parcel of land situated in Hopkinton in the County of Merrimack and State of New Hampshire and bounded and described as follows:

Beginning at a stone marker on the easterly side of Gould Hill Road at the southwesterly corner of the land now or formerly of Philip C. and Gloria F. Martin and running along the boundary of said Gould Hill Road and land of Houston a distance of seventy-six (76) feet to a stone marker; thence running due east approximately three hundred fifty (350) feet to a stone wall on this bound intersecting the boundary line of land of Martin; thence running westerly in a northerly direction along last named bound to the point of beginning.

Meaning and intending to describe and convey the same premises as conveyed to Peter N. Martin, Mark W. Martin, Lee J. Martin and Peter N. Martin, Trustee of the Philip C. & Gloria W. Martin Revocable Family Trust of 2008 by virtue of the will of Gloria W. Martin, see 5th Circuit-Probate Division-Newport, Case #320-2015-ET-392. See also deed to Philip C. Martin and Gloria F. Martin dated January 15, 1964 recorded in the Merrimack County Registry of Deeds at Book 934, Page 61 and a deed to Philip Clarence Martin and Gloria Frances Martin dated March 21, 1961 recorded in the Merrimack County Registry of Deeds at Book 876, Page 305. Philip C. Martin died on December 7, 2012, see Merrimack County Probate Court Docket No. 317-2013-ET-8.

This is not the homestead of the Grantors or their spouses.

Pursuant to New Hampshire RSA 564-A:7 II: (1) The undersigned Trustee, Peter N. Martin, as trustee of the Philip C. and Gloria W. Martin Revocable Family Trust of 2008 as Grantor under trust dated May 22, 2008 has full and absolute power in said trust agreement to execute, sign and deliver any deed or instrument necessary to sell and convey any interest in real estate and improvements thereon held in said trust and no purchaser or third party shall be bound to inquire whether the Trustee has

Warranty Deed Peter N. Martin, Mark W. Martin, Lee J. Martin, and Philip C. & Gloria W. Martin Rev. Family Trust of 2008 to Rix Family Trust Page 2 of 3 said power or is properly exercising said power or to see to the application of any trust asset paid to the Trustee for a conveyance thereof. (2) The Trustee has received all necessary or desirable direction from the beneficiaries of the trust agreement. (3) The trust agreement is a trust as defined by New Hampshire RSA 564-A:1 I. The trust has not been revoked and is still in full force and effect.

Executed this 28 day of June , 2019.

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Philip C. and Gloria W. Martin Revocable Family Trust of 2008 u/t/d May 22, 2008

eter N. Martin, Arustee

State of New Hampshire County of _Merrimack ; ss:

Personally appeared the above named Peter N. Martin, Mark W. Martin, Lee J. Martin and Peter N. Martin, as Trustee of the Philip C. and Gloria W. Martin Revocable Family Trust of 2008, before me this <u>28</u> day of <u>June</u>, 2019 known to be the persons whose names are subscribed to the foregoing instrument and acknowledged that they executed the same for the purposes therein contained.



Notary Public/Justice of the Peace My Commission Expires:

Warranty Deed Peter N. Martin, Mark W. Martin, Lee J. Martin, and Philip C. & Gloria W. Martin Rev. Family Trust of 2008 to Rix Family Trust Page 3 of 3 EFiled 202000019931 Recorded in Merrimack County, NH In the Records of Susan Cragin, Register BK: 3696 PG: 1739, 9/16/2020 12:15 PM LCHIP S25.00 RECORDING S34.00 SURCHARGE S2.00

Return to: McLane Middleton, Professional Association RAW/ cem 59370 P.O. Box 326 Manchester, NH 03105

WARRANTY DEED

I, JOHN H. LYNCH, a married individual with a mailing address of 2 Watchtower Road, Hopkinton, New Hampshire 03229, grant to SUSAN E. UPTON LYNCH and WILLIAM G. STEELE, JR., CPA, Trustees of THE JOHN H. LYNCH IRREVOCABLE TRUST OF 2012, a New Hampshire trust u/d/t dated December 15, 2012, with a mailing address of 2 Watchtower Road, Hopkinton, New Hampshire 03229, with WARRANTY COVENANTS:

A certain parcel, with the improvements thereon, located in The Town of Hopkinton, County of Merrimack, State of New Hampshire, described as follows:

Tract I:

That portion of a certain property, located in The Town of Hopkinton, County of Merrimack, State of New Hampshire, and known as the Gage Place, which is situated westerly of the highway leading from Hopkinton Village to Tyler Station, but

EXCEPTING from the said Premises conveyed a parcel of land situated on said highway surrounding the buildings which are situated thereon, being ten (10) acres, more or less, which ten acre parcel is bounded and described as follows:

Beginning on the southerly side of the lane leading to the woodland on the westerly side of the road from Hopkinton Village to Tyler Station at the corner of the wall on the south side of said lane; westerly along this wall about five hundred and fifty (550) feet to the easterly side of an opening in that wall; thence southerly in a straight line to a point in the south line of said property at a point approximately five hundred four (504) feet west of the above-mentioned highway; easterly along said wall about five hundred four (504) feet to the highway; northerly along said highway to the point of beginning.

EFiled 202000019931 Recorded in Merrimack County, NH In the Records of Susan Cragin, Register BK: 3696 PG: 1740, 9/16/2020 12:15 PM LCHIP \$25.00 RECORDING \$34.00 SURCHARGE \$2.00

Said premises are shown on a plan entitled, "TYPE MAP OF THE BRIER HILL FARM WOODLOT, JULY, 1916," filed in the Registry of Deeds as Map #750, the said granted premises being bounded and hatched in red on said plan.

The above parcel is believed but not warranted to be further described as follows:

A certain tract or parcel of land with all improvements and appurtenances situate on the east side of Gould Hill Road and westerly of Briar Hill Road in the Town of Hopkinton, County of Merrimack, and State of New Hampshire, as shown on a plan entitled, "THE GOULD HILL TRUST, WILLIAM G. STEELE, JR., TRUSTEE", prepared by Bristol, Sweet & Associates, Inc., dated September 22, 1999, recorded as Plan #14886 in the Merrimack County Registry of Deeds (the "Plan"), which tract or parcel is more particularly bounded and described as follows:

- Beginning at an intersection of two stone walls at an iron rod on the easterly sideline of Gould Hill Road at the westernmost corner of the within premises and the northwesterly corner of land now or formerly of Philip C. and Gloria F. Martin;
- running in a northerly directly along a stone wall and the easterly sideline of Gould Hill Road a distance of 986.5 feet, more or less, to an iron pipe at an intersection of stone walls at land now or formerly of Arnold C. & Alice R. Coda, (shown as Tax Lot #240-50 on the Plan), which iron pipe is North 11° 33' 55" East a distance of 983.17 feet from the previously mentioned iron rod;
- turning and running in an easterly direction along a stone wall and said Coda land a distance of 431.0 feet, more or less, to an iron pipe at an intersection of stone walls and a barbed wire fence, which iron pipe is North 87° 44' 23" East a distance of 430.88 feet from the previously mentioned iron pipe;
- turning and running along land now or formerly of Erik Leadbeater, (shown as Tax Lot #240-49 on the Plan), North 88° 00' 35" East a distance of 1071.76 feet to a 1" iron rod at the beginning of a barbed wire fence;
- turning and running still along said Leadbeater land North 07° 11' 20" West a distance of 713.15 feet to an iron rod set in a drill hole at the end of a stone wall at the end of the barbed wire fence;
- continuing along the stone wall and land of Leadbeater North 09° 22' 17" West a distance of 153.76 feet to an iron rod set in a drill hole in the stone wall at land now or formerly of Walter W. Dwyer Jr. 1998 Trust, (shown as Tax Lot #241-43 on the Plan);
- turning and running along said Dwyer Trust land North 49° 02' 30" East a distance of 448.94 feet to an iron rod at a bend in a barbed wire fence;

turning and running still along said Dwyer Trust land, North 80° 55' 36" East a distance of 757.50 feet to a drill hole at the end of a stone wall near a corner of barbed wire fences, at land now or formerly of Mary H. Small, (shown as Tax Lot #241-38.2 on the Plan);

turning and running along said Small land South 16° 39' 06" East a distance of 898.18 feet to a drill hole at the end of a stone wall;

continuing along the stone wall and said Small land a distance of 469.60 feet to a drill hole in the stone wall, which drill hole is South 16° 40' 35" East, and a distance of 469.60 feet from the next previously mentioned drill hole;

continuing along the stone wall and land now or formerly of David L. & Judith Poole, (shown as Tax Lot #241-38.1 on the Plan) a distance of 541.50 feet to a drill hole at the end of the stone wall, which drill hole is South 16° 24' 19" East, and a distance of 541.46 feet from the next previously mentioned drill hole;

continuing in a southeasterly direction along said Poole land South 16° 36' 36" East, a distance of 82.47 feet to a drill hole at the end of a stone wall;

continuing in a southeasterly direction along the stone wall and said Poole land a distance of 257.10 feet, more or less, to a drill hole in the stone wall, which drill hole is South 16° 20' 41" East, a distance of 256.98 feet from the next previously mentioned drill hole;

- continuing in a southeasterly direction along the stone wall and said Poole land a distance of 288.90 feet, more or less, to a drill hole at an intersection of stone walls, which drill hole is South 16° 40' 26" East, a distance of 287.63 feet from the next previously mentioned drill hole;
- turning and running in an easterly direction along a stone wall and said Poole land a distance of 392.30 feet, more or less, to a drill hole 3.85 feet easterly of a corner of stone walls at the westerly sideline of Briar Hill Road, which drill hole is North 83° 41' 42" East, a distance of 392.31 feet from the next previously mentioned drill hole;
- turning and running South 01° 05' 05" West, a distance of 45.39 feet along the westerly sideline of Briar Hill Road to a disk set in a drill hole at an intersection of stone walls at land now or formerly of Sandra Schneider, (shown as Tax Lot #249-5 on the Plan);
- turning and running in a westerly direction along a stone wall and said Schneider land a distance of 558.50 feet, more or less, to an iron pipe in a gap in the stone wall 1.86 feet westerly of the end of the stone wall, which iron pipe is

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South 83° 26' 17" West, a distance of 557.61 feet from the disk referred to in the previous course;

- turning and running still along said Schneider land South 06° 37' 11" East, a distance of 1123.91 feet to an iron rod in a stone pile on a stone wall at land now or formerly of Donald & Sandra P. Saxon, (shown as Tax Lot #249-4 on the Plan);
- turning and running along a stone wall and said Saxon land North 63° 16' 34" West, a distance of 225.14 feet to a point at an intersection of stone walls at land now or formerly of Robert A. & Nancy N. Sweatt, (shown as Tax Lot #239-22 on the Plan);
- turning and running in a westerly direction along the stone wall and said Robert Sweatt land a distance of 559.60 feet, more or less, to a drill hole at an intersection of stone walls at land now or formerly of Dana L. & Alice Sweatt, (shown as Tax Lot #239-21 on the Plan), which drill hole is North 86° 26' 54" West, a distance of 559.33 feet from the next previously mentioned point of intersection of stone walls;
- turning and running along said Dana Sweatt land North 65° 05' 14" West, a distance of 690.41 feet to an iron rod in a drill hole at the end of a stone wall at land now or formerly of Martha Houston Jones Revocable Trust of 1997, (shown as Tax Lot #240-54 on the Plan);
- continuing in a northwesterly direction along the stone wall and said Jones Trust land a distance of 323.80 feet, more or less, to a drill hole at an intersection of the stone wall and a row of stones, which drill hole is North 66° 21' 18" West, a distance of 319.37 feet from the last mentioned iron rod;
- continuing in a northwesterly direction along the stone wall and said Jones Trust land a distance of 909.90 feet, more or less, to a drill hole at the end of the stone wall, which drill hole is North 64° 01' 06" West, a distance of 901.84 feet from the next previously mentioned drill hole;
- continuing in a northwesterly direction along said Jones Trust land a distance of 300.40 feet, more or less, to an iron pipe at the end of a stone wall at land now or formerly of Philip C. & Gloria F. Martin, (shown as Tax Lot #240-52 on the Plan), which iron pipe is North 63° 47' 13" West, a distance of 300.30 feet from the last mentioned drill hole;
- continuing in a northwesterly direction along the stone wall and said Martin land a distance of 648.20 feet, more or less, to the point of beginning, which point is North 68° 12' 49" West, a distance of 647.78 feet from the last mentioned iron pipe.

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Tract II:

A certain tract of land with the improvements situated thereon located on the northeasterly side of Gould Hill Road, in the Town of Hopkinton, County of Merrimack, State of New Hampshire, and more particularly bounded and described as follows:

- Commencing at a point marking the intersection of stone walls, which said point is 323 feet, more or less, northeasterly from the northeasterly line of the Gould Hill Road, so-called;
- running northeasterly along a stone wall and the southeasterly line of land now or formerly of Concord Kitchen Corporation (said land being formerly owned by one Shreve and by one Sweatt) and by land formerly of one Loverin, to a stake and stones marking the line of land now or formerly of one Hopkins, of one Sanborn and of one Loverin;
- running southeasterly along line of land now or formerly of Hopkins, Sanborn and Loverin and land now or formerly of the Gage heirs (said latter land being formerly owned by the Stephen Sibley heirs) to a stake and stones at corner of land of said Gage heirs (formerly Sibley heirs);
- running southwesterly along line of said land of said Gage heirs to an intersection of stone walls which said intersection is 439 feet, more or less, northeasterly from the northeasterly line of said Gould Hill Road;
- running northwesterly along land now or formerly of George L. Butterfield, Jr. and Ann S. Butterfield, 475 feet, 4 inches, more or less, to an iron pipe driven in the ground, said iron pipe lying within the right of way hereinafter described;
- continuing in the same direction 40 feet, more or less, to another iron pin driven in the ground, said iron pin marking the northeasterly corner of said right of way hereinafter described;
- continuing in the same direction along other land of said Butterfields 580 feet, 8 inches, more or less to the point of beginning.

TOGETHER WITH a RIGHT OF WAY 50 feet in width leading from Gould Hill Road to the above described land, said right of way being bounded and described as follows:

Commencing at an iron pipe driven into the ground on the northeasterly line of Gould Hill Road, which said iron pin is 540 feet, 8 inches southeasterly from the northwesterly corner of land of said Butterfields and the southwesterly corner of land of Concord Kitchen Corporation, said corners joining on the northeasterly line of said Gould Hill Road; EFiled 202000019931 Recorded in Merrimack County, NH In the Records of Susan Cragin, Register BK: 3696 PG: 1744, 9/16/2020 12:15 PM LCHIP S25.00 RECORDING S34.00 SURCHARGE S2.00

running northeasterly through said Butterfields land to an iron pipe driven in the ground and referred to as the northeasterly corner of said right of way in the above described land;

running southeasterly 40 feet to an iron pipe driven in the ground; and referred to as lying within said right of way in the above described land;

continuing southeasterly an additional ten feet (10') to a point which is the southeasterly corner of said right of way;

running southwesterly 50 feet from and parallel to the first line described in this right of way, to the northeasterly line of Gould Hill Road;

running northwesterly along said Gould Hill Road 10 feet to an iron pipe;

continuing northwesterly along said Gould Hill Road 40 feet to the point of beginning.

The Premises are conveyed together with all appurtenant rights and easements.

These premises are conveyed subject to the restriction that only a single-family residence may be constructed on the premises conveyed herein. This restriction shall run with the land and bind future grantees or successors in interest.

SUBJECT TO and TOGETHER WITH all reservations, restrictions and/or covenants, easements, liens, encumbrances and mortgages of record, if any, insofar as the same may now be in force and applicable.

MEANING AND INTENDING to describe and convey the same property conveyed to John H. Lynch by deed of John H. Lynch and Susan E. Upton Lynch as Trustees of The John H. Lynch Trust, of near or even date and recorded herewith.

This instrument was prepared from information supplied by the Grantor herein and no independent title search has been conducted.

This transfer is exempt from transfer tax pursuant to RSA 78-B:2, IX.

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Signed this 31st day of August, 2020.

JOHN H. L

STATE OF NEW HAMPSHIRE COUNTY OF MERRIMACK

This instrument was acknowledged before me on the 31st day of August, 2020, by John H. Lynch.

(seal)

Notary Public/Justice of the Peace Printed Name: My Commission Expires:

> ROBERT A. WELLS, Noter Public S My Commission Expires Junuary 48, 2028



T.F. BERNIER, INC.

Land Surveyors~Designers~Consultants

50 Pleasant Street, P.O. Box 3464 Concord, NH 03302-3464 Environmental Permitting State and Local Permitting Land Surveying Aerial Mapping Aerial Photography

> Tel. (603) 224-4148 Fax (603) 224-0507

Abutters List John H. Lynch Irrevocable Trust of 2012 & Rix Family Trust of 2016 Lot Line Adjustment Application Assessors Map 240 Lot 51 & 52 Gould Hill Road Hopkinton, NH

MAP	LOT	OWNER
240	51	John H. Lynch Irrevocable Trust of 2012 2 Watchtower Road Hopkinton, NH 03229
240	52	Rix Family Trust of 2016 248 Gould Hill Road Hopkinton, NH 03229
239	21	Virginia L. Pastuszczak Timothy D. Sweatt 373 Old Stagecoach Road Contoocook, NH 03229
239	22	Five Rivers Conservation Trust 10 Ferry Street Suite 311A Concord, NH 03301
240	26	Neola D. Crathern Trust 163 Gould Hill Road Contoocook, NH 03229
240	27	Richard A. Newcombe Trust & Sally T. Newcombe Trust 221 Gould Hill Road Contoocook, NH 03229
240	28	Karen L Whiteknact Rev. Living Trust 239 Gould Hill Road Hopkinton, NH 03229
240	29	Alison Josefiak Christopher Navarro 257 Gould Hill Road Contoocook, NH 03229

240	30	Thomas R. & Hilary A. Chapman 283 Gould Hill Road Contoocook, NH 03229
240	31	Bradford W. & Ann McClane Kuster 331 Gould Hill Road Contoocook, NH 03229
240	32	Jane D. W. & Frederic Bradstreet P.O. Box 149 333 Gould Hill Road Contoocook, NH 03229
240	36	Irvin D. Gordon 63 Roberts Road Hopkinton, NH 03229
240	49	April Dunn 59 Blaze Hill Road Hopkinton, NH 03229
240	54	Richard Jones Irrevocable Trust 18 Green Street Newport, NH 03773
241	38-1	Kirk Hemphill 831 Briar Hill Road Contoocook, NH 03229
241	38-2	R and J Case Trust 1030 Briar Hill Road Hopkinton, NH 03229
241	43	Jeanne C. Dwyer GST Exempt Trust P.O. Box 600 Concord, NH 03302
249	.4	Kurt K. & Betsey F. Rhynhart 675 Briar Hill Road Hopkinton, NH 03229
249	5	The Viking Trust 745 Briar Hill Road Contoocook, NH 03229
249	8	S. Wayne & Elizabeth A. Clarke 812 Briar Hill Road Hopkinton, NH 03229

Professional Consultant

Timothy F. Bernier LLS, CWS T. F. Bernier, Inc. PO Box 3464 Concord, NH 03302-3464

TOWN OF HOPKINTON, NH SUBDIVISION CHECKLIST

Applicants shall use the General Principal and Design and Construction Standards (Section IV, Subdivision Regulations) when designing and laying out a subdivision. These principles and requirements shall be construed as the minimum requirements. The Planning Board may require higher standards in individual cases or may waive certain requirements for good cause.

Submittal Material

- An application, either signed by <u>all</u> the current owner(s) of the property, <u>or</u> signed by an individual authorized by the owner(s) to act as their agent. NOTE: A letter must be submitted with the application authorizing the individual to act as agent on the owner(s) behalf when the agent signs the application.
- The appropriate application fee.
- X A deed showing property description and ownership.
- List of the current abutters to the property including those property owners located across street, brook or stream from the property being subdivided. Please include the name, address and profession of the professionals responsible for the preparation of the subdivision plans.
- One (1) set of addressed mailing labels of abutters, applicant, engineer, architect, soil or wetland scientist, land surveyor, and holder of conservation preservation, or agricultural preservation restrictions or easements.
- NA Copies of any approvals or permits required from state and federal agencies.
 - X Written request for any waivers from the Subdivision Regulations, if any.
- NA A copy of any variances or special exceptions which have been granted by the Zoning Board of Adjustment.
 - Four (4) copies of the subdivision plat which contains all the information outlined in the Subdivision Regulations.
 - Eleven (11) copies of the application, along with all supporting document(s), including reductions of the plan(s) reduced to no more than 11" x 17".

General Information

- A subdivision shall be shown at a scale of not less than one inch equals one hundred feet (1*=100') or at a greater detail as directed by the Planning Board.
- Plans shall be presented on sheets sized at 22" x 34". Recordable drawings must conform to the requirements of the Merrimack County Registry of Deeds.
- X Title of plat and Name and address of the owner and that of agent, if any.
- X Date the plan was prepared and the date of all revisions.
- X North arrow, bar scale and Tax Map/Lot references.
- X Name, address, seal, and signature of the licensed surveyor, engineer, and certified soils or wetland scientist.

Design and Sketch Plan

A vicinity sketch showing location of property in relation to surrounding streets systems and other pertinent features.

Town of Hopkinton, NH SUBDIVISION CHECKLIST

(partial)WR		A sketch of the site showing existing natural features, including watercourses, waterbodies, tree lines, and other significant vegetation cover, topographic features and any other features that are significant to the site design.
WR		Contours at intervals not exceed five feet (5') with spot elevations provided when the grade is less than five percent (5%).
(partial) WI	۲	Surveyed exterior property lines showing their bearings and distances and showing monumentation locations.
(partial) WF		Location and dimensions of uplands and wetlands as certified by a certified soils or wetland scientist.
	X	Lines and right-of-way of existing abutting streets.
NA		Location, elevation, and layout of existing and proposed catch basins and other surface drainage features.
	X	Location and size of all utilities serving the site.
(partial) WR		Soils location and types.
		Any other features that would fully explain the concept of the proposal, existing conditions, and future development.
	<u>Subdivisi</u>	on Plan
(partial)WF		Location and dimensions of all boundary lines of the property to be expressed in feet and decimals of a foot.
	X	Location and width of existing and proposed streets and easements, alleys, and other public ways, easements and proposed street rights-of-ways.
WR	X	Building setbacks lines, including location and setback dimensions of existing structures within 50-feet of the parcel to be subdivided.
	X	Location, dimensions, and areas of all proposed or existing lots (calculated in acreage and square feet).
NA		Location and dimensions of all property proposed to be set aside for a park or playground use, public or
NA		private reservation, with designation of the purpose and conditions, if any, of dedication or reservation. Location of all parcels of land proposed to be dedicated to public or common use and the covenants, conditions of such dedications, and a copy of such private deed declarations, covenants or restrictions.
(partial)WR		Location, bearing and lengths of all lines; and sufficient data to be able to reproduce such lines upon the ground; and location of all proposed monuments.
	X	Statement as to the proposed use of all lots, sites, or other realty (whether single-family, two-family, etc.) and all other uses proposed.
NA		Lots consecutively numbered or lettered in alphabetical order.
NA		Location and explanation of proposed drainage easements and any other site easements, if any.
	X	Form of approval by the Planning Board.

TOWN OF HOPKINTON, NH SUBDIVISION CHECKLIST

Construction Plan

NA

- Profiles plotted with the same horizontal scale as the plans and a horizontal to vertical scale ratio of 5 to 1 respectively showing existing and proposed elevations along center lines of all roads. Where a proposed road intersects an existing road or roads, the elevation along the center line of the existing road or roads within one hundred (100) feet of the intersection, shall be shown. Curve data of all horizontal curves, lengths of tangents, central angles and stationing of all streets shall be shown. Vertical curve data, percent grade and elevation shall be shown on the profiles.
- Plans and profiles showing the locations and typical cross-section of street pavements including curbs and gutters, sidewalks, drainage easements, rights-of-way, manholes, and catch basins; the locations of street trees, street lighting standards, and street signs; the location, size and invert elevations of existing and proposed sanitary sewers, storm water drains, and fire hydrants, showing connection to any existing or proposed utility systems; and exact location and size of all water or other underground utilities or structures.
- Location, size, elevation, and other appropriate description of any existing facilities or utilities, including, but not limited to, existing streets, septic disposal facilities, sewers, drains, water mains, wells, easements, water bodies, streams, and other pertinent features, such as surface drainage areas, swamps, buildings, at the point of connection to proposed facilities and utilities within the subdivision. The water elevations of adjoining lakes or streams at the date of the survey, and the approximate high and low water elevations of such lakes or streams.
- Topography at the same scale as the sketch plat with a contour interval of two (2) feet, in the area of new roadway construction, referred to sea-level datum. All datum provided shall be referenced to U.S. Coast and Geodetic Survey datum, where practical, and should be so noted on the plat.
- Cross sections at a minimum of one hundred (100) foot intervals, all cross pipes and at other critical locations drawn at a scale of 1" = 10 feet.

Additional Information

In order to evaluate the subdivision proposal, the applicant is expected to supply or the Planning Board may specifically require the following information, as appropriate:

- Draft of any protective covenants or easements.
- Warranty deeds conveying to the Town Streets, right-of-way, and any sites for public use in fee simple, free from all encumbrances, unless waived by the Planning Board.
- Calculations specifying the quantity of storm water run-off and a statement from applicant's engineer certifying the adequacy of the proposed drainage facility to handle such run-off.
- Calculations on the type and quantity of sanitary waste generated and a statement from the engineer or licensed designer certifying that the proposed facilities will adequately handle the projected effluent.
- Traffic Impact Analysis, Fiscal Impact Analysis, School Impact Analysis, Environmental Impact Analysis and/or Community Services Impact Assessment.
- Necessary State and local permits.

Performance Guarantees

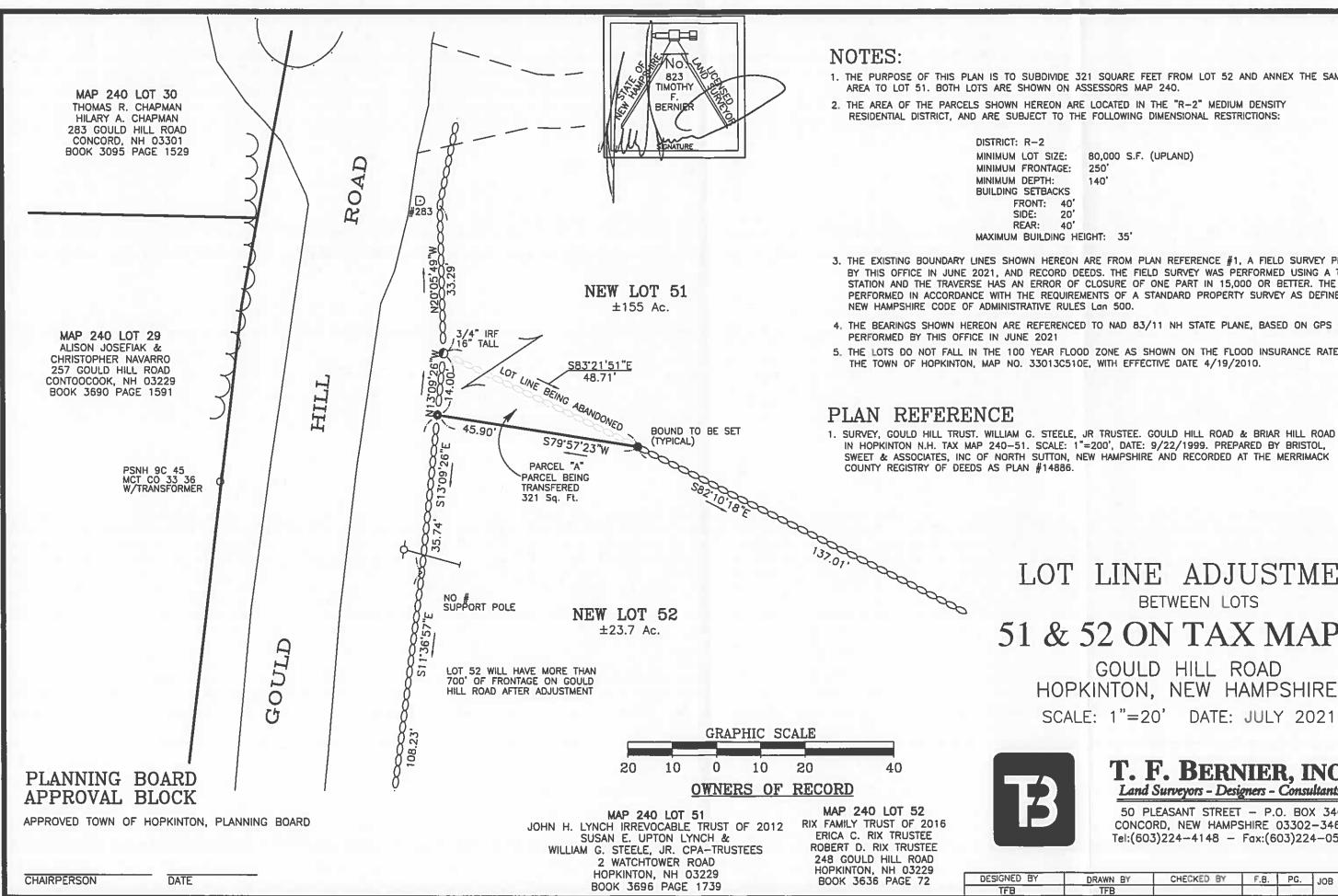
Except in the case of a subdivision in which each lot is on an existing Town road, before the plat is signed by the Chairperson of the Planning Board, all applicants shall be required to submit to the Planning Board the following:

TOWN OF HOPKINTON, NH SUBDIVISION CHECKLIST

Cash, irrevocable letter of credit or passbook (in the name of the Town) issued by a Banking Institution doing business in New Hampshire, in the amount approved by the Planning Board and deposited with the Board of Selectmen.

NA

- The performance guarantee shall comply with all statutory requirements and be satisfactory to the Board of Selectmen as to form, sufficiency, and manner of execution. The amount of the performance guarantee shall be in the amount representing 100% of the cost of completion of the streets, the installation of utilities, and other proposed facilities. Upon partial completion of the subdivision improvements and inspected by the Town's Consultant Engineer, the Board of Selectmen may authorize in writing a prorated reduction in the performance guarantee relating to the remaining cost to complete.
- The entity responsible for the constructing of the roadway and utilities shall provide the Town with cash, irrevocable letter of credit, or passbook (in the name of the Town) issued by a Banking Institution doing business in New Hampshire to cover the cost of inspection services. As a minimum this security shall be \$3.00/foot of roadway to be constructed plus ten percent (10%). Ten percent of the total construction observation costs shall be retained by the Town of Hopkinton to cover administrative costs.



1. THE PURPOSE OF THIS PLAN IS TO SUBDIVIDE 321 SQUARE FEET FROM LOT 52 AND ANNEX THE SAME

80,000 S.F. (UPLAND) 250' 140

3. THE EXISTING BOUNDARY LINES SHOWN HEREON ARE FROM PLAN REFERENCE #1, A FIELD SURVEY PERFORMED BY THIS OFFICE IN JUNE 2021, AND RECORD DEEDS. THE FIELD SURVEY WAS PERFORMED USING A TOTAL STATION AND THE TRAVERSE HAS AN ERROR OF CLOSURE OF ONE PART IN 15,000 OR BETTER. THE SURVEY WAS PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF A STANDARD PROPERTY SURVEY AS DEFINED IN THE

4. THE BEARINGS SHOWN HEREON ARE REFERENCED TO NAD 83/11 NH STATE PLANE, BASED ON GPS OBSERVATIONS

5. THE LOTS DO NOT FALL IN THE 100 YEAR FLOOD ZONE AS SHOWN ON THE FLOOD INSURANCE RATE MAP FOR

IN HOPKINTON N.H. TAX MAP 240-51. SCALE: 1"=200', DATE: 9/22/1999. PREPARED BY BRISTOL, SWEET & ASSOCIATES, INC OF NORTH SUTTON, NEW HAMPSHIRE AND RECORDED AT THE MERRIMACK

> LOT LINE ADJUSTMENT BETWEEN LOTS

51 & 52 ON TAX MAP 240

GOULD HILL ROAD HOPKINTON, NEW HAMPSHIRE SCALE: 1"=20' DATE: JULY 2021



50 PLEASANT STREET - P.O. BOX 3464 CONCORD, NEW HAMPSHIRE 03302-3464 Tel:(603)224-4148 - Fax:(603)224-0507

	DRAWN BY	CHECKED BY	F.B.	PG.	JOB #	667 01
	TFB					663-01
1	DRAWING NAME					

Application #2021-20

BAYSTONE PROPERTIES LLC

Site Plan Review, Architectural Design Review, Condominium Subdivision



Town of Hopkinton

330 Main Street • Hopkinton, New Hampshire 03229 • www.hopkinton-nh.gov Tel: 603-746-3170 Fax: 603-746-3049

PLANNING BOARD APPLICATION

Site Plan Review Architectural Design Review (Commercial/Industrial - ZO Section IV-A) Preliminary Review (SD Section II) X Subdivision Lot Line Adjustment/Annexation

Conditional Use Permit (ZO Section III) Special Use Permit (ZO Section VIII)

PROJECT LOCATION: Cedar Street	MAP/LOT:	102/ 35	ZONE: VR-1
APPLICANT: Baystone Properties, LLC		,,	ZONE:
Address: 44 North Shore Road	City: Derry	State: NH	7in: 03038
Phone: 774-327-8331	Email: shaungear		
OWNER(s) _{Name:} Same as applicant		0	
Address:	City:	State:	Zin
Phone:	Email:	Otate	Zip:
Name:			
Address:	City:	State	Zini
Phone:	Email:		zip
PROFESSIONALS (englneer, architect, surveyor, Name: Scott R Frankiewicz, LLS (New Ha Address: 683C First NH Turnpike	attorney, wetland/soil scienti mpshire Land Consultar City: Northwood	its, PLLC)	_ Zip: 03261
Phone: 003-942-9220		indconsultants.	.com
Name: Bernie Temple, PE			
Address: P.O. Box 7	City: Gilmanton	State: NH	Zip:03837
Phone: 603-630-1008	Email: temple2@		
X Residential □ Recreational □ Agricultural X Public Water X Public Sewer □ Well □ S ots/units proposed: <u>12</u> Existing Buil % Open Space: <u>45.9%</u> (Note: Buil	eptic ding Area: 2.070 sq ft = F	Pronosed Building	
Application Submission Requirements	and real of the gloss ales	a)	

Original and ten (10) copies of the application, along with all supporting document(s), including reductions of the final plan(s) to no more than 11" x 17".

- Narrative description of proposal (include existing conditions and albrelated improvements).
- X Application checklist.
- Planning Board/Zoning Board of Adjustment Minutes of Conceptual, Preliminary Review, or approval.
- Property deed and existing/proposed easements, covenants, and restrictions.
- X Tax Map of subject parcel and abutting properties.
- □ Waiver(s) request from provisions of the Subdivision and/or Site Plan Regulations.

PLANNING BOARD APPLICATION

- Test Pit Data, Storm Water Management Plan, Traffic, School, Environmental and Fiscal Impact Analyses, and Phasing Plan (when applicable).
- Abutters List as defined by RSA 672:3 Include Tax Map, Lot Number, Name and Mailing Addresses. If abutting property is under a condominium or other collective form of ownership, the term "abutter" means the officers of the collective or association. If abutting property is under a manufactured housing park form of ownership, the term "abutter" includes the manufactured housing park owner and the tenants who own the manufactured housing.
- Mailing labels Include Applicant, Owner, Architect, Soil/Wetland Scientists, Abutters, and holders of Conservation/Preservation restrictions or easements.
- Four (4) paper prints of the plan(s) at full scale.

Major Subdivision	\$500.00 Application Fee, \$100.00 per Lot/Unit
Minor Subdivision	\$250.00 Application Fee, \$100.00 per Lot/Unit
Lot Line Adjustment/Annexation	
Site Plan Review	\$300.00 Application Fee, \$100.00 per Unit (Res./Non-Res.)
Site Plan Review Change of Use	
Conditional Use Permit	\$500.00 Application Fee (Wireless Telecommunications)
	\$ 10.00 per Address (Owner, Applicant, Agent, Abutter)
Newspaper Notice	

Conditional Use Permit (Wireless Telecommunications): If application is for Conditional Use Permit, please attach a detailed explanation of compliance with Section 3.10 of the Hopkinton Zoning Ordinance, along with an explanation of compliance with the Site Plan Review Regulations of the Town of Hopkinton.

Final Submission Requirements (after Planning Board action):

- Four (4) paper prints of the final plan set at full scale.
- Mylar(s) The Merrimack County Registry of Deeds requires that the UPPER LEFT-HAND CORNER, INSIDE THE BORDER, of the plat to be RESERVED for recording information entered by the Registry - No smaller than 7" long X 1" wide.
- PDF of the final plan set, including architectural and site photographs emailed or thumb drive.
- Recording Fees: (Separate Checks) Made payable to Merrimack County Registry of Deeds Recording Fee.....\$ 26.00 per Page (22" x 34") LCHIP Fee.....\$ 25.00 per Document

I represent to the best of my knowledge and belief that this application is being submitted in accordance with applicable regulations and ordinances of the Town of Hopkinton. I also understand that submittal of this application shall be deemed as granting permission for the Planning Board members and their designees to enter onto the property for purposes of inspections and review. Permission to visit the property extends from the date an application is submitted until approved work or construction is complete and any or all of the financial guarantees, if any, have been returned to the applicant, or until the application is formally denied. Furthermore, I agree that the proposed project will be performed in accordance with this application, the attached plans and specifications, and the regulations and ordinances of the Town of Hopkinton.

Applicant's Signature **Owner's Signature(s**

		Office Use:	
Application Filed:	Fees:	Application #:	
Notice(s) Posted/Mailed: Meeting(s)/Hearing(s):		Complete/Consideration:	
Approved/Denied:		Conditions MCRD Filing:	
MCRD Document #:			

Town of Hopkinton, NH Site Plan Review Checklist

In cases where not all items are applicable, draw a line through the items that are not applicable. All requests for waivers must be in writing with the application.

ALL APPLICATIONS

- X 1. Ten (10) copies of completed application, all associated documentation and checklist;
- List of names and addresses of abutters and use of abutting properties, identified with location of the structures and access roads;
- One (1) set of address mailing labels of abutters, applicant, engineer, architect, soil scientist, wetland scientist, land surveyor, and any holders of conservation preservation, or agricultural preservation restrictions or easements;
- X 4. The appropriate fee;
- 5. Site plan: 24" by 36" sheet size maximum, scale not less than 1" = 100', match lines where needed, date, title, graphic scale, north arrow, location map, legend, name & address of developer/applicant, designer/engineer, and owner of record;
- Four (4) prints of each plan sheet at full scale and one (1) reduction of each plan reduced to no more than 11" x 17";
- X 7. All existing and proposed easements;
- 8. Site plan showing boundaries, existing natural features including watercourses & water bodies, trees & other vegetation, topographical features, and other pertinent features that should be considered in the site design process;
- Plan of all buildings depicting their type, size, and location (setbacks);
- 10. Location of off-street parking and loading spaces with a layout or the parking indicated;
- 11. The location, width, curbing and type of access ways and egress ways (driveways), plus streets and sidewalks within and around site;
- 12. Location, size, and design of proposed signs and advertising or instructional devices;
- X 13. Location and type of lighting for all outdoor facilities, including direction and area of illumination;
- x 14. Right-of-way lines of all existing adjoining streets;
- X 15. Location and type of Water supply & sewage disposal facilities;
- X 16. Zoning districts and boundaries for site and within 1000 feet of site;
- X 17. 100 year flood elevation line, where applicable;
- X 18. An elevation view or photograph of all buildings indicating their height, width and surface treatment;
- 19. Landscaping plan showing required details described within the Site Plan Review Regulations, and
- 20. Other required exhibits or data in order to adequately evaluate the proposal

Town of Hopkinton, NH Site Plan Review Checklist

PROJECTS REQUIRING NEW OR ADDITIONAL BUILDINGS OR CHANGES TO THE EXTERIOR DIMENSIONS OF EXISTING OR CHANGES TO EXISTING CONTOURS AND FINISHED GRADE ELEVATIONS, INCLUDING TYPE, EXTENT, AND LOCATION OF LANDSCAPING, PARKING AND OPEN SPACE AREAS SHALL SUBMIT THE FOLLOWING IN ADDITION TO ABOVE:

- Reproducible mylar, to be retained by the Planning Board at its option;
- Plan of all buildings with their type, size, location (setbacks) and elevation of first floor indicated: (assume permanent onsite elevation);
- The size and proposed location of water supply and sewage facilities and provision for future expansion of sewage and water facilities, and all distances from existing water and sewage facilities on the site and on abutting properties to a distance of 200 feet;
- x 4. The location, elevation and layout of catch basins and other surface drainage features;
- Existing and proposed contours and finished grade elevations all contours shall be a minimum of 2foot intervals;
- X 6. The type, extent and location of existing and proposed landscaping and open space areas indicating what existing landscaping and open space areas will be retained;
- The size and location of all public service connections gas, power, telephone, fire alarm, (overhead or underground);
- 8. Surveyed property lines showing their angles, distances, radius, lengths of arcs, control angles, along property lines and monument locations and names of all abutters;
- 9. If a subdivision, the lines and names of all proposed streets, lanes, ways, or easements intended to be dedicated for public use shall be indicated and all Subdivision Regulations shall apply, and
- X 10. Erosion and sedimentation control plan.

TOWN OF HOPKINTON, NH ARCHITECTURAL DESIGN REVIEW CHECKLIST

Planning Board approval of an Application for Architectural Design Review shall be required prior to the issuance of a building permit for the following activities (check as applicable):

- New building construction to be used for non-residential or multi-family purposes; or To be submitted after Planning Board approval
- Additions or alterations to buildings used for non-residential or multi-family purposes which increase or decrease the square footage of the building; or

Renovation, rehabilitation or reconfiguration of building exteriors where such buildings are used for non-residential or multi-family purposes.

The **Performance Criteria** is intended to encourage building architecture that is complementary to the community. It is intended that the criteria be administered with flexibility and consistency in order to allow for responsive, creative and innovative architectural designs. The criteria is not intended to dictate specific building styles, or to mandate historical preservation, restoration or replication.

In order to approve an Application for Architectural Design Review, the Planning Board shall find that the application demonstrates substantial conformity with the following Performance Criteria:

- (a) The proposed building design is consistent with the purposes of the Architectural Design Review Ordinance.
- (b) The proposed building design demonstrates sensitivity towards and is complementary to, the architectural heritage of Hopkinton.

Building Orientation: How a building is positioned or located on a site can complement or detract from the site and/or the architectural character of the surrounding area. The orientation of proposed buildings should take into consideration building setbacks, spacing between buildings, alignment of building(s), open spaces, access and circulation areas, as may be evidenced in the development pattern of the surrounding area or as determined to be appropriate by the Planning Board; and

Building Scale and Proportion: Building elevations, scale, massing and the proportional relationship between structures can complement or detract from the architectural character of the surrounding area. Building designs should be compatible with or provide a harmonious transition from adjacent sites. The scale and proportion of proposed buildings should take into consideration the scale and proportion of surrounding buildings, as evidenced in the development pattern of the surrounding area, and should also take into consideration natural features, historically significant buildings or features and surrounding land uses. Visual conflicts between properties should be minimized; and

Roofline: Rooflines can provide visual interest and help to reduce the mass of a building. Traditional roofline types such as gabled, hipped, and gambrel that are evidenced in Hopkinton's architectural heritage are strongly encouraged. Type, shape, pitch and direction of roofs should be considered in the design. Flat roofs are strongly discouraged; and

Massing: The physical bulk or mass of buildings, particularly larger or elongated ones, can either enhance or detract from the architectural character of the community. Structures should be carefully designed to break up their mass into smaller visual components providing human scale, variation and depth; and

Architectural Features and Details: Architectural features and details such as cornices, columns, corner trim, doorways, entrances, windows/trim, awnings, dormers, porches, etc., can provide or enhance visual interest, provide a pedestrian scale and help mitigate negative effects of building mass. Architectural features and details should be considered in every building design. Traditional features should be considered in every building design. Traditional features should be considered in every building design. Traditional features are strongly encouraged; and

Materials, Texture and Color: Exterior building materials, texture and colors should be treated as significant design elements that help define the appearance of a structure and create visual interest. The use of

TOWN OF HOPKINTON, NH ARCHITECTURAL DESIGN REVIEW CHECKLIST

traditional materials that are consistent with Hopkinton's vernacular or indigenous architecture, or materials having the same visual effect, are strongly encouraged. Consideration should be given to the materials, textures and colors used in the neighborhood; and

Building Facade: Facades for new or renovated structures should provide visual interest from all visually accessible sides. Windows, doorways and architectural detailing and patterns should complement the building form and historical context. Facades should be designed to establish a complementary relationship with other site considerations such as pedestrian scale and orientation, signage, landscaping and lighting; and

Building Renovation or Addition: Where an existing building has features that are consistent with the Performance Criteria, proposed renovations or additions should be designed to respect the proportions, patterns, detailing, materials, etc., of the original building. Where the existing building does not have features that are consistent with the Performance Criteria, the owner/applicant is encouraged to upgrade the structure to meet the Performance Criteria; and

Signs: Signs should be designed to meet the needs of individual uses while complementing the building, the site and its surroundings. The design of building-mounted signs should complement, not detract from the architectural features of the building. Signs should be scaled to the architectural elements that surround it. Consideration should be given to sign form, color, lighting and materials that are compatible with the building and its surroundings; and

Gateways and Scenic Resources: Some places in Hopkinton contribute to the landscape character of the community because of their location and scenic qualities. Many such properties and approaches acts as gateways, providing first impressions and reinforcing Hopkinton's sense of place. Consideration should be given towards complementing these resources through the careful citing of new buildings, and the application of the Performance Criteria; and

Design Continuity: Each building design, from the simple to the complex, requires the coordination of multiple design elements such as architectural style, form, massing, materials, detailing, etc. The proposed building design shall demonstrate coordination of design elements and an overall design continuity.

WAIVER PROVISION

The Planning Board may grant waivers to the requirements of this Architectural Design Review Ordinance provided that a majority of the Planning Board finds that the criteria set forth in Section 15.8.3 of the Hopkinton Zoning Ordinance regarding "variances" have been satisfied.

In approving waivers, the Planning Board may impose such conditions, as it deems appropriate to substantially secure the objectives of the standards or requirements of the Architectural Design Review Ordinance.

A letter for any such waiver shall be submitted in writing by the applicant for Planning Board review. The letter shall state fully the grounds for the waiver and all of the facts relied upon by the applicant in support thereof.

Owner of Record

Tax Map 102 Lots 34 & 35 Baystone Properties, LLC 162 Barton Corner Road Hopkinton, NH 03229 **Property addresses:** 49 & 71 Cedar Street

Abutters list

Tax Map 101 Lot 15 Ernest & Matthew Thibodeau 35 Cedar Street Contoocook, NH 03229

Tax Map 101 Lot 14.1 & 14.2 Cedar Street Properties, LLC C/O Stephen Tate P.O. Box 1253 Grantham, NH 03753 Property address: 27 & 29 Cedar Street

Tax Map 101 Lot 13 Scott & Brett Crathern 163 Gould Hill Road Contoocook, NH 03229 Property address: 25 Cedar Street

Tax Map 102 Lot 46 United Methodist Church P.O. Box 356 Contoocook, NH 03229 Property address: 24 & 28 Maple Street

Tax Map 104 Lot 45 Joshua Smith 115 North Main Street Boscawen, NH 03303 Property address: 40 Maple Street Tax Map 102 Lot 36 Glenn & Melissa Smart 81 Cedar Street Contoocook, NH 03229

Tax Map 102 Lot 29 David Fisk 88 Cedar Street Contoocook, NH 03229

Tax Map 102 Lot 30 Daniel & Janice Aranki 70 Cedar Street Contoocook, NH 03229

Tax Map 102 Lot 31 Andris & Florence Serzans 60 Cedar Street Contoocook, NH 03229

Tax Map 102 Lot 32 Charles & Anne Rotondi 54 Cedar Street Contoocook, NH 03229

Tax Map 102 Lot 33 Robert MacNeil 50 Cedar Street Contoocook, NH 03229

Tax Map 101 Lot 16 O'Rourke & Greenblott Holdings, LLC P.O. Box 465 Hopkinton, NH 03229 Property Address: 44 Cedar Street

Tax Map 101 Lot 17 Town of Hopkinton 330 Main Street Hopkinton, NH 03229

Professional

New Hampshire Land Consultants, PLLC 683C First NH Turnpike Northwood, NH 03261

Bernie Temple, PE P.O. Box 7 Gilmanton, NH 03837



Town of Hopkinton

Wastewater Department

330 Main Street, Hopkinton NH 03229-2627 (603) 746-8261 - watersewer@hopkinton-nh.gov

June 23, 2021

NH Land Consultants 683c First Nh Turnpike Northwood NH,03291

To whom it may concern,

After review of the proposed plans, I the Superintendent for the Hopkinton Wastewater Facility, see there to be no issues from a flow standpoint and the extra flow for this project would not hinder the process of the wastewater treatment plant. As expressed to Scott Frankiewicz via e-mail, the decision-making process is not up to me what soever and can not make a decision. If project is permitted by the town but will be inspecting all work that is in connection process to the Gravity line on Cedar St. The one concern I have is that there is an effluent line and a force main sewer line on Cedar St. and connection needs to be made in the Gravity Main.

Sincerely,

Samuel V. Currier

TES Environmental Consultants, LLC

June 18, 2021

Ref: TES JN 21-0049

Scott R. Frankiewicz, Owner New Hampshire Land Consultants, PLLC Gray Properties, LLC 683C First NH Turnpike (Rte. 4) Northwood, NH 03261

Re: Environmental Services (Wetland Identification) 49 and 71 Cedar Street, Hopkinton (Contoocook), New Hampshire Tax Map 102, Lots 34 and 35

Dear Mr. Frankiewicz:

TES Environmental Consultants, L.L.C. (TES) has completed the site investigation that you requested on the above-referenced parcels in Hopkinton (Contoocook), New Hampshire. This investigation was completed on June 16, 2021 and consisted of an on-site review to determine if wetlands subject to local, state and/or federal jurisdiction were present on the property.

The wetland identification was performed according to the methodology presented in the <u>Corps of</u> <u>Engineers Wetland Delineation Manual (Technical Report Y-87-1), January 1987</u> and the <u>Regional</u> <u>Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region</u>, Version 2.0, January 2012, US Army Corps of Engineers. This methodology requires the presence of indicators for the three parameters: hydric soils, hydrophytic vegetation and evidence of hydrology at or near the surface for 14 days during the growing season.

I am pleased to report that no jurisdictional wetland areas were identified on or immediately adjacent to the parcel. All lower-lying areas on each parcel were closely examined, and all areas lack the required hydric soils, with soil colors of 2.5Y 5/4 or brighter in the 12-18 inch depth ranges. No evidence of wetland hydrology exists on the parcels. Some plant species that could exist in wetlands (hydrophytes) were observed, including sensitive fern (*Onoclea sensibilis*), horsetail (*Equisetum* spp.), jewelweed (*Impatiens capensis*) and buttercup (*Ranunculus acris*), but these species are also often found in uplands. Upland indicators including sugar maple (*Acer saccharum*) and Oriental bittersweet (*Celastrus orbiculatus*) were also present.

I hope that this information will beneficial in the future land use on these parcels. If I can be of further assistance in this process, please let me know.

Sincerely,

Thomas 2. John

Thomas E. Sokoloski New Hampshire Certified Wetland Scientist #127



1494 Route 3A, Unit 1, Bow, New Hampshire 03304 Phone: 603-856-8925 E-Mail: tom@tesenviro.comcastbiz.net

6/18/2021

PROPOSED CONDOMINIUM SUBDIVISION PLAN FOR BAYSTONE PROPERTIES, LLC

TAX MAP 102, LOT 35

PROJECT LOCATION:

71 CEDAR STREET, HOPKINTON, (CONTOOCOOK) NH MERRIMACK CO.

nsultants

TERSITE: INLANDCONSULTANTS.CO.

URVEYING . LAND PLANNING . REAL ESTATE ART COM 000, NH 03281 PH, 603-842-9220

	NOTES]				SHEE	<u>I INDE</u>
	1. THE PURPOSE OF THIS PLAN IS TO SHOW A CONDOMINUM SUBDIVISION ON TAX MAP 102 LOT 35.		HOW			DWG	SHT NO
	2. THE PROPERTY IS DESIGNATED AS TAX MAP 102, LOT 35.		Tel.			CVR	1 OF 16
	3. THE AREA OF THE EXISTING LOT 35 IS 0.81 ACRES (39,780 SF.)					ECP	2 OF 16
	4. THE CURRENT OWNER FOR LOT 30: BAYSTONE PROPERTIES, LLC 44 NORTH SHORE RD, DERRY HH 03038, LDT 30 BK 3087, PAGE 1784					PSP	3 OF 16
	1. THE ZONING DESIGNATION FOR THE PROPERTY IS VR-1 DISTRICT.		$\left(\right)$	о сонтоосоок		PCP	4 OF 16
	8. DIMENSIONAL REQUIREMENTS PROVIDED FOR ZONE VR-1 DISTRICT:					PGP	5 OF 16
	MIN, ROAD / RONTAGE =80' MIN, LOT \$22E =15,000 \$F (0.34 ACRES)		TAX MAP 102	POWWOOK PD		PUTP	6 OF 16
	LINK, ROAD SETBACK -30 LINK, SDE SETBACK -15 LINK, REAS SETBACK -40		LOT(8) 34 & 3			PLSC	7 OF 16
	MAN, ARAR SETBACK -400 MARMAN STRUCTURE HEICHT -35		16	SITE SA AUNTING		PLTP	8 OF 16
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			A ROAD US	то нориалтан		DET-2 DET-3	10 OF 16 11 OF 16
(7. NO WETLANDS WERE LOCATED ON SITE, THOMAS & SCHOLOSKI, CERTIFIED WETLAND SCENTIST VISITED THE SITE DETERMINING THERE WERE NO WETLANDS FOUND ON SITE.		10 8	1		DET-4	12 OF 16
	B. THE EXISTING USE OF TH 102 LOT 35 IS A 2 CAR GARAGE.					DET-5	13 OF 16
	9. THE PROPOSED USE OF THI TO2 LOT 35 IS A CONDOMINAUM SUBDIVISION.					DET-6	14 OF 16
	10. SEWER TO BE PROVIDED BY MUNICIPAL			LOCATION PLAN		DET-7	15 OF 16
	11. WATER TO BE PROVOED BY MUNICIPAL			SCALE: 1"-1,000"		DET-8	16 OF 16
SV	12. THERE IS SUFFICIENT CAPACITY WITH BOTH WATER AND SEWER PER THE TOWN OF HOPGHTON,						
	13. RIGHT OF WAY WOTH DETERMINED BY SURVEY, FIELD INVESTIGATION, RECORDED DEEDS AND PLANS OF REFERENCE.						
	14. ABUITING PROPERTY REFORMATION PROVIDED BY A COMBINATION OF DH-LINE TAX MAP DATA AND DATA PROVIDED BY grantwer.uvt.edu.	PI	ROFESSI	ONAL CONSULTANTS LIS	ST		
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]	FOR ROAD AND BROCE CONSTRUCTION.	A manufactor		BOW, NEW HAMPSHIRE 03304 PH: (603) 856-8925			CHAIRMAN
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		An. 15100	ENGINEER	P.O. BOX 7 GILMINGTON IRON WORKS, NH 03837			
	18. IF DURING CONSTRUCTION, IT BECOMES APPARENT THAT ADDITIONAL EROSION CONTROL, MEASURES ARE REQUIRED TO STOP ANY EROSION ON THE CONSTRUCTION STE DUE TO ACTUAL STEE CONDITIONS. THE OWNER SHALL BE REQUIRED TO INSTALL THE NECESSARY EROSION PROTECTION AT NO EXPENSE TO THE TOTAL	Pour Charles		PH: (603) 630-1008			
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	8				BAYSTO (SHAUN	E PROPERTIES, LLC	
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						H, 03038 - EK 3591 PG 1784	
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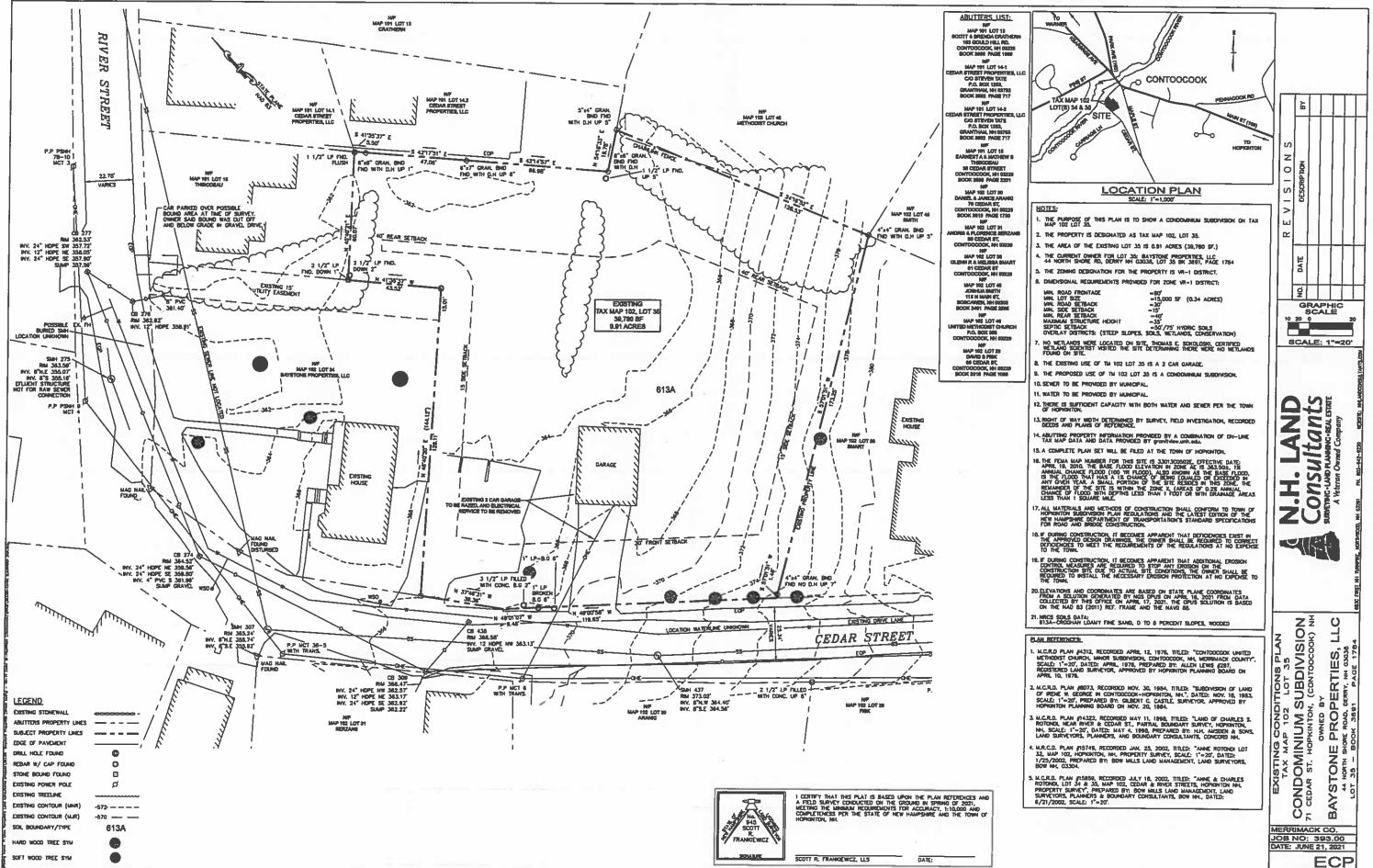


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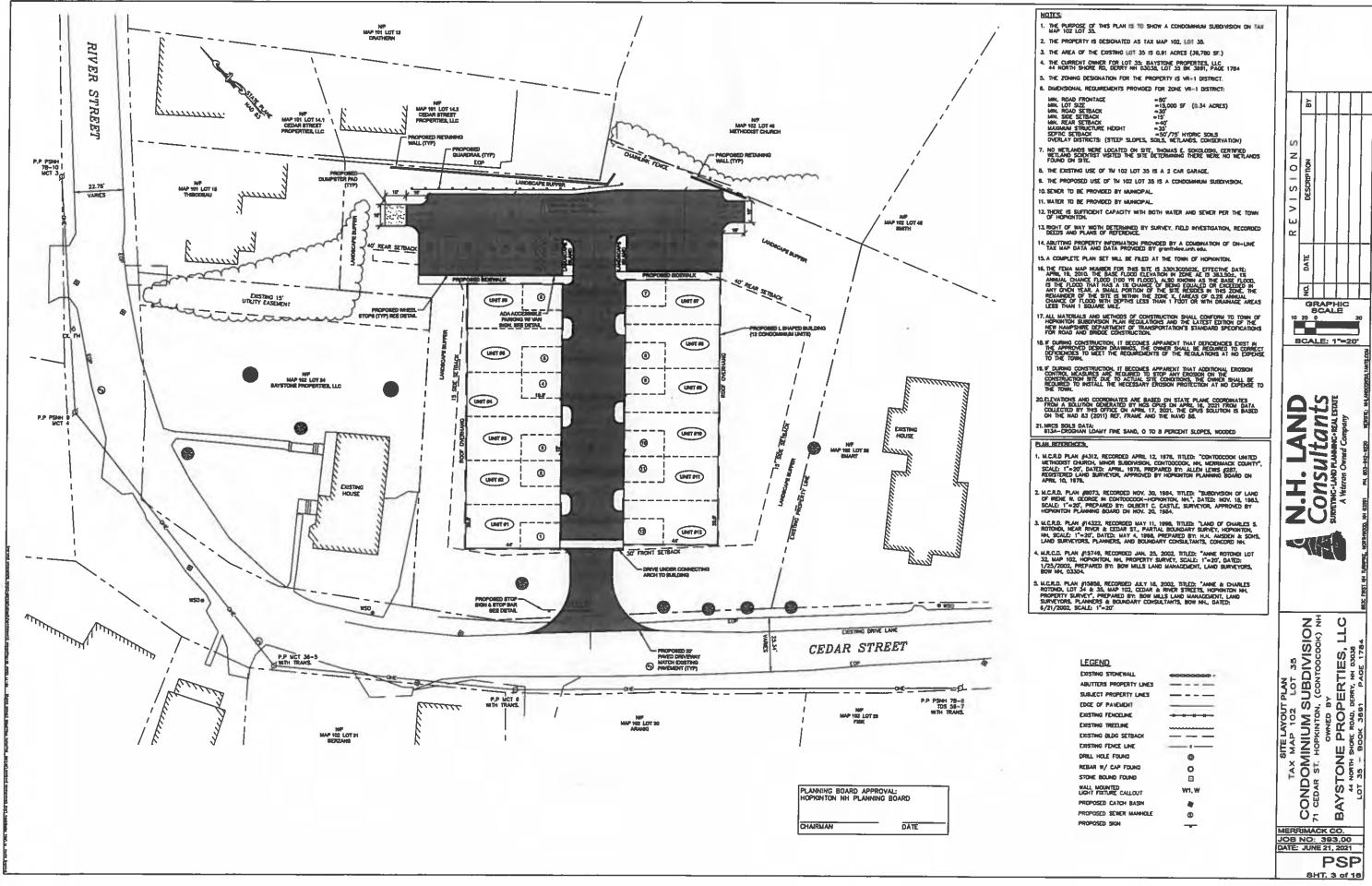
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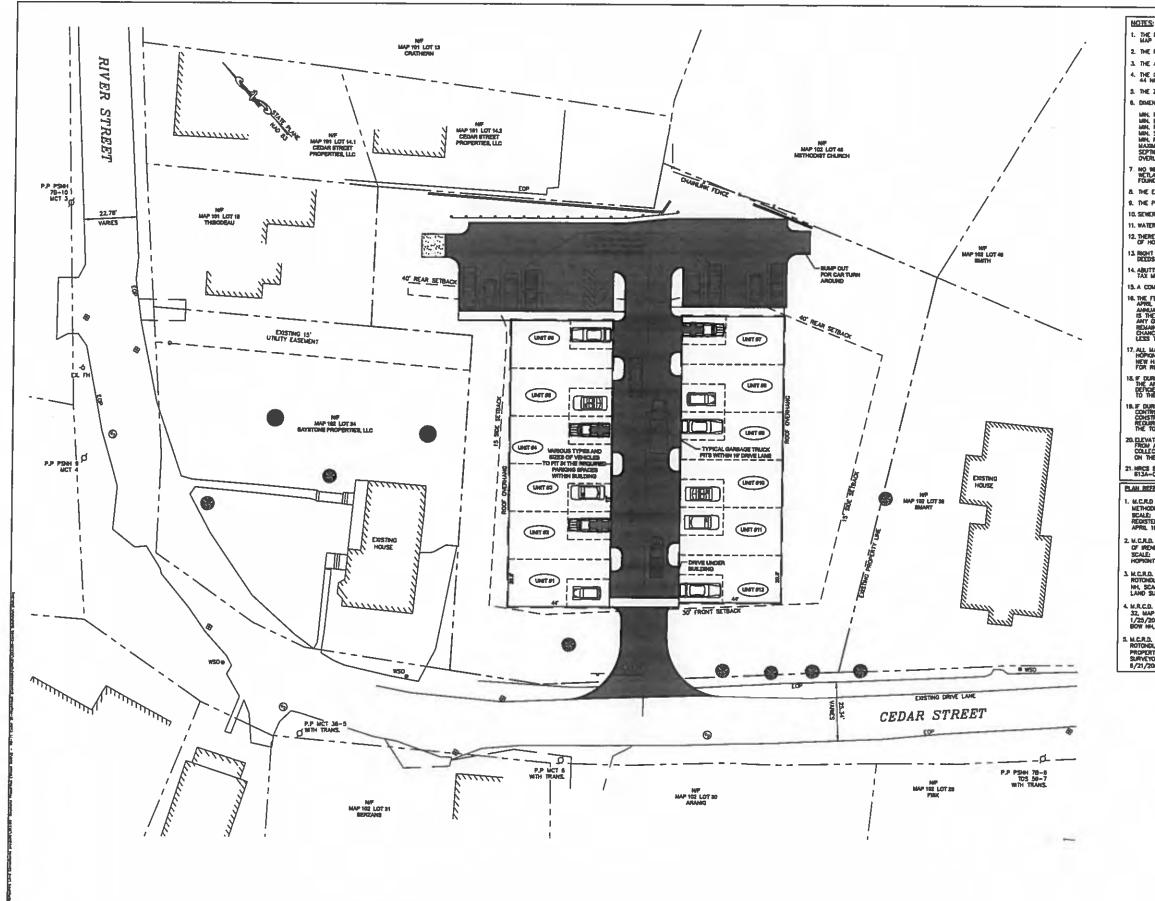
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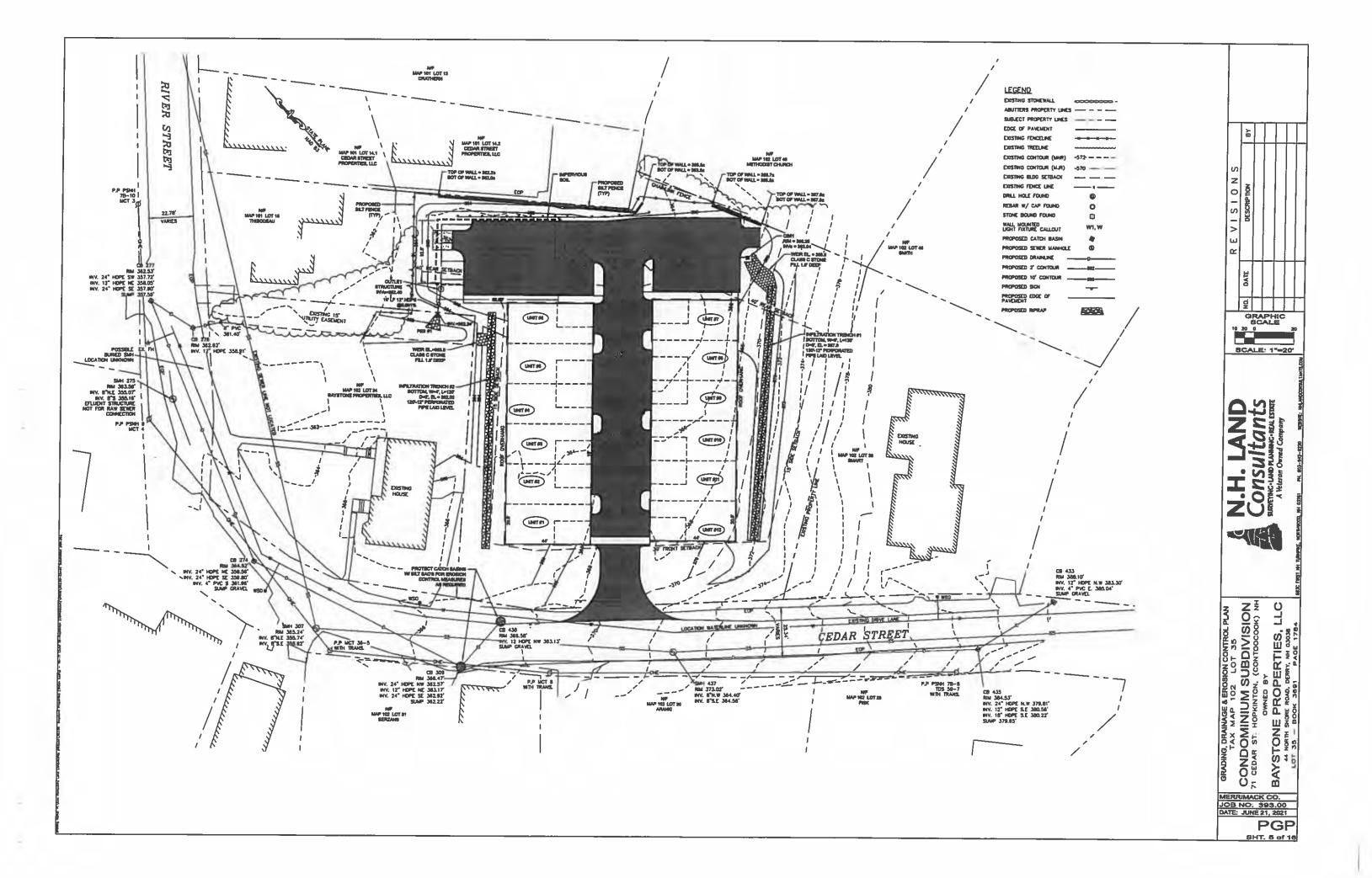


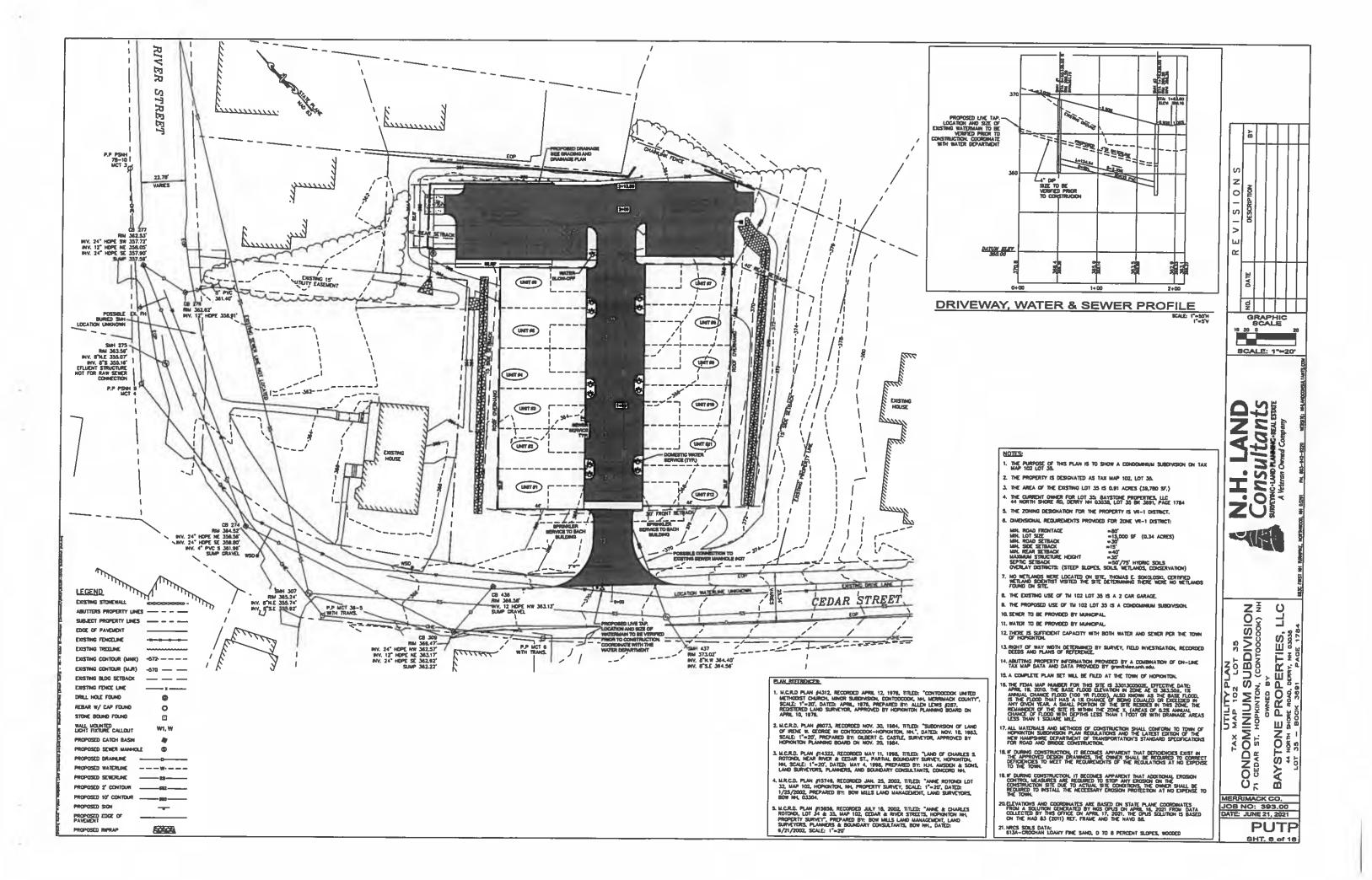
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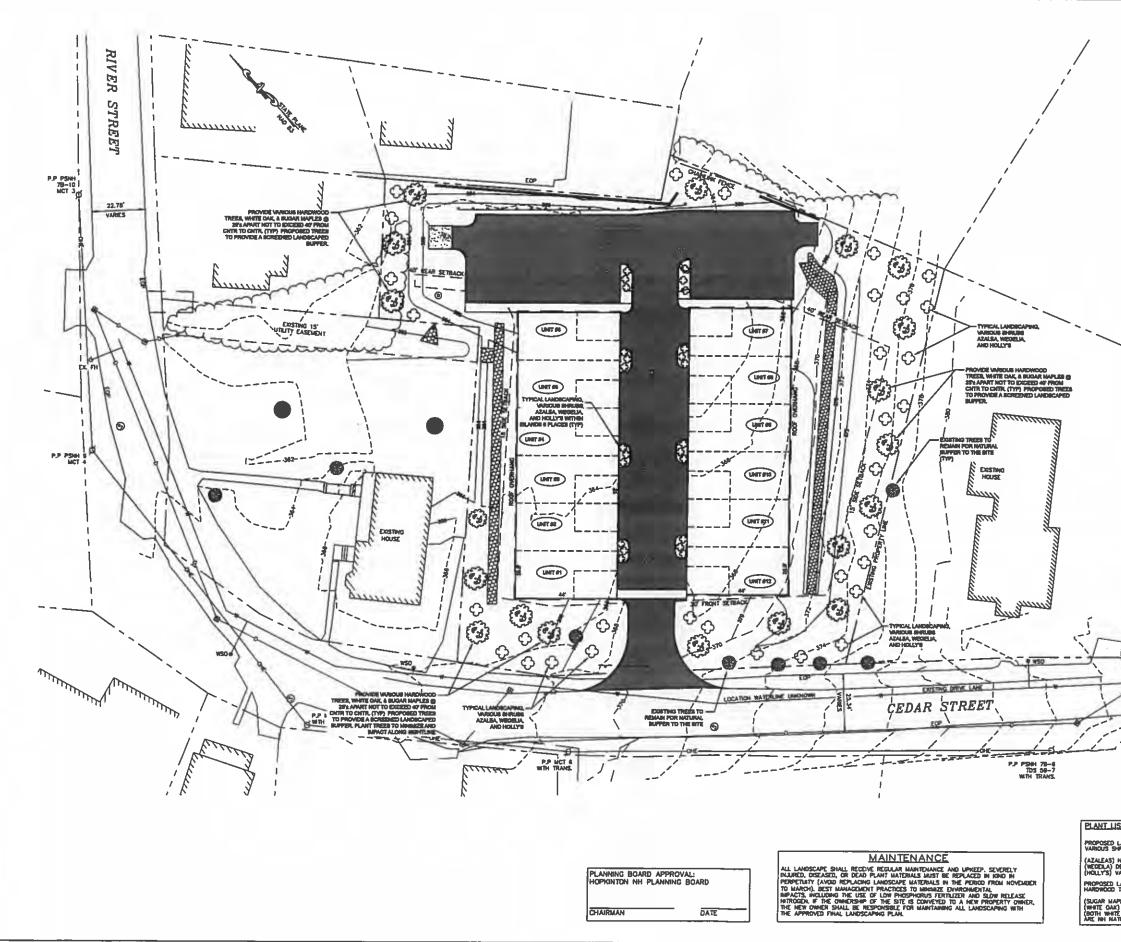




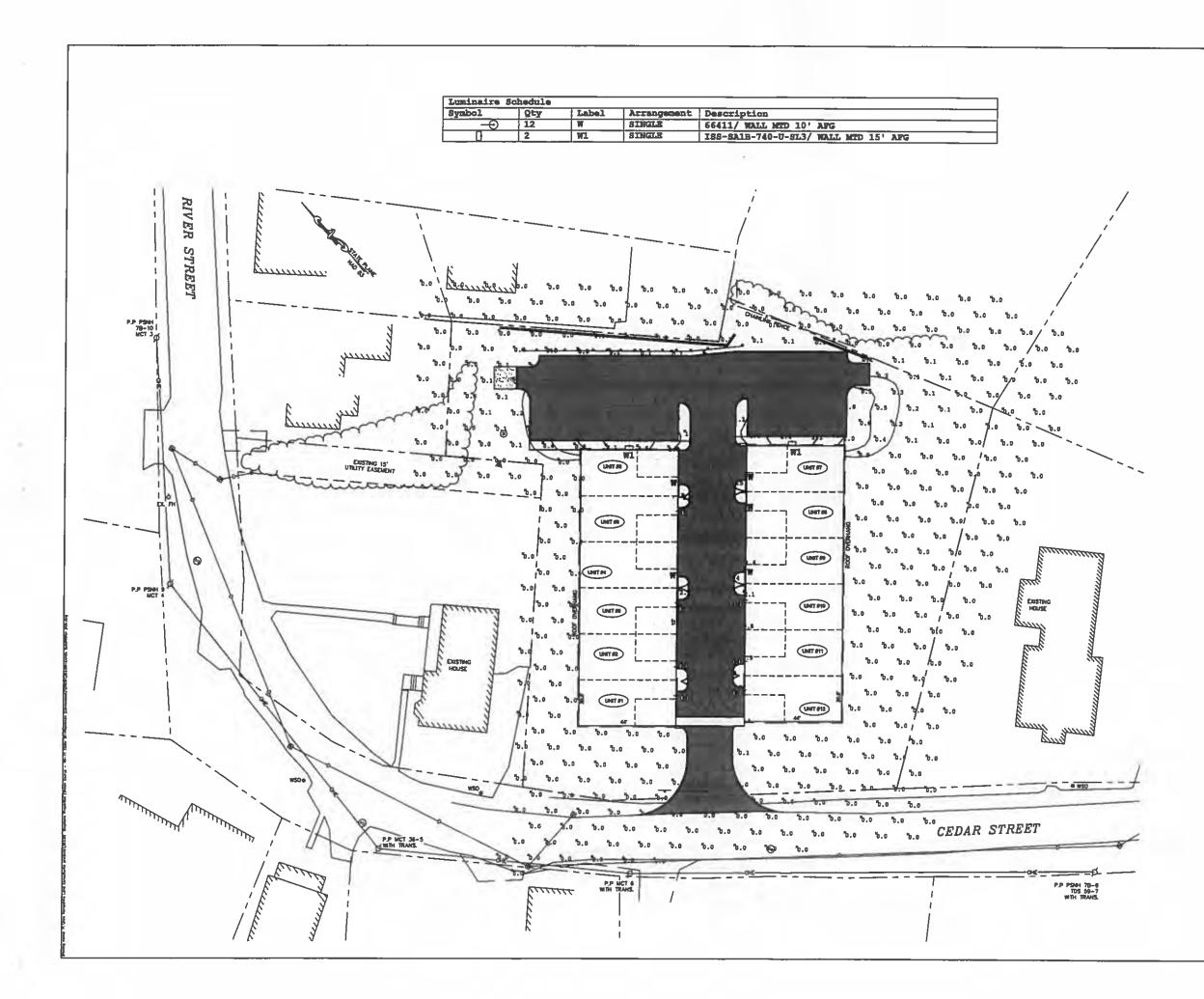
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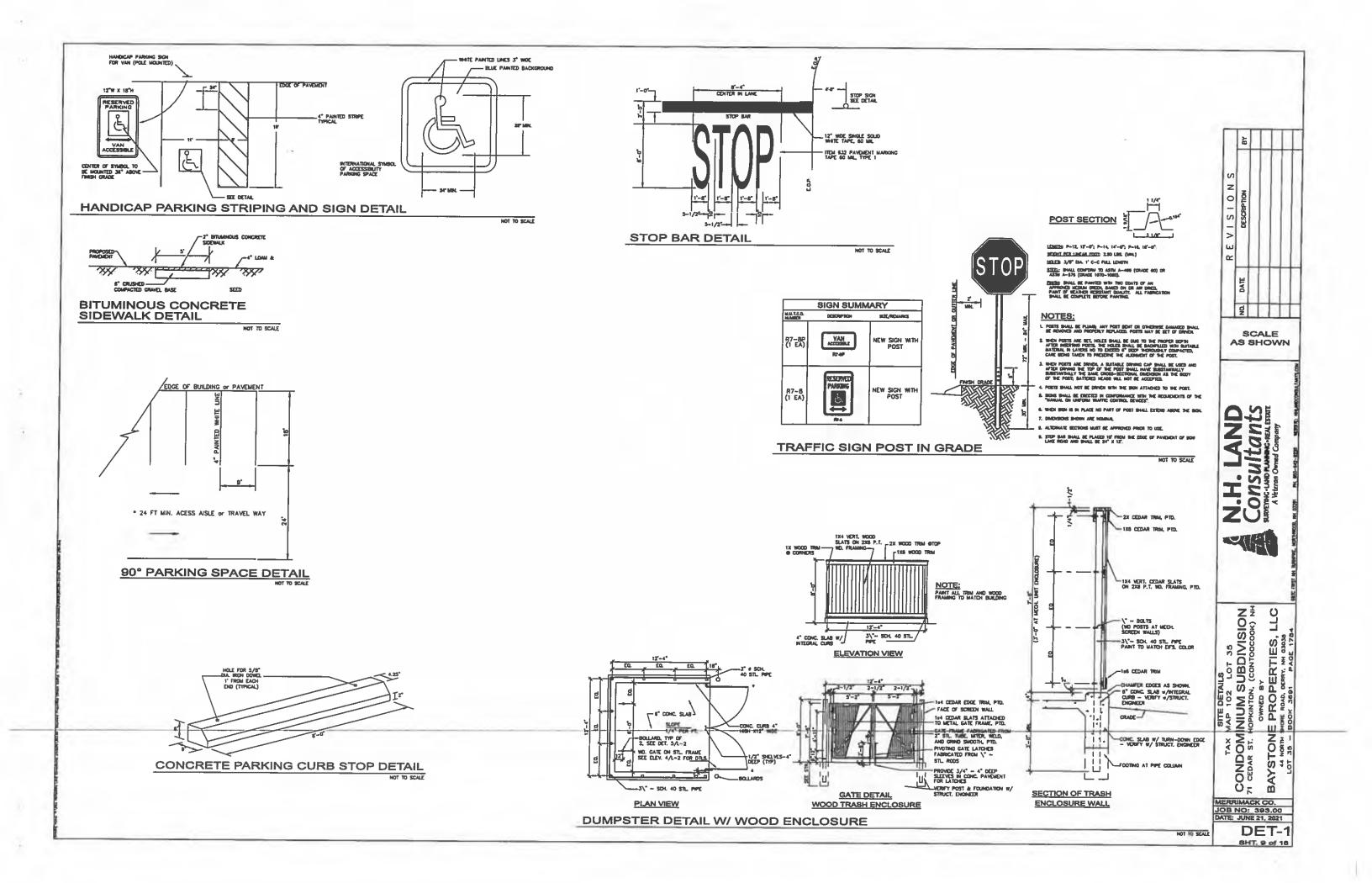




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LANDSCAPING TREES PLES) ) 2 DAK & SUGAR MAPLE			MERR JOB 1	О _К Ш Імаск со. 10: 393.00







### MANHOLE TESTING PER NHOES Env-Wg. 704.17

THE MANHOLE VACUUM TEST SHALL IN ACCORDANCE WITH ASTM C1244 AND CONFORM TO THE FOLLOWING: (1)

THE INITIAL VACUUM GAUGE TEST PRESSURE SHALL BE 10 INCHES HO: AND MEMIAUM ACCEPTABLE TEST HOLD TIME FOR A 1-INCH HG PRESSURE DROP TO 9 INCHES HO SHALL BE: A. HOT LESS THAN 2 MINUTES FOR MANHOLES LESS THAN 10 FEET DEEP IN DEPTH; B. MOT LESS THAN 2.5 MINUTES FOR MANHOLES 10 TO 15 FEET DEFP: AMD (8) TH

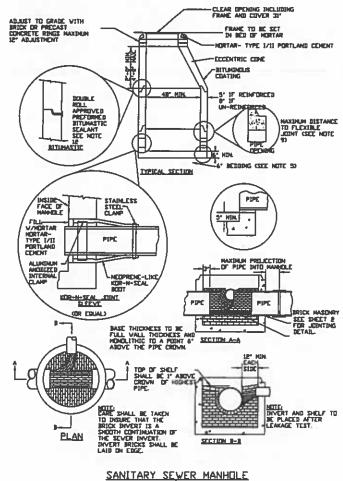
B. NOT LESS THAN Z.5 MINUTES FOR MANHOLES 10 TO 15 FEET DEEP; AND C. NOT LESS THAN 3 MINUTES FOR MANHOLES MORE THAN 15 FEET DEEP;

- THE MANHOLE SHALL BE REPARED AND RETESTED IF THE TEST HOLD THIES FAIL TO ACHIEVE THE ACCEPTANCE LIMITS SPECIFIED ABOVE. (2)
- FOLLOWING COMPLETION OF THE LEAKAGE TEST, THE FRAME AND COVER SHALL BE PLACED ON THE TOP OF THE MANNOLE OR SOME OTHER HEANS USED TO PREVENT ACCOUNTAL ENTRY BY UNALTHORIZED PERSONS, CHELDREN, OR AMMALS, UNTLI THE CONTRACTOR IS READY TO MAKE FRAM. ADJUSTMENT TO GRADE. (3) WHIMLIN SIZE PIPE FOR HOUSE SERVICE SHALL BE SIX INCHES.

### PIPE AND JOINT MATERIALS

- A. PLASTIC PIPE:
- 1. PVC SEWER PIPE AND FITTINGS SHALL CONFORM TO ASTM 02412 (SDR 35 MANMUM). METHODS OF SHEPPING AND STORAGE ON SITE SHALL BE SUCH AST D AVOID INJURY TO THE PIPE. DAMAGED PIPE SHALL BE REJECTED AND REMOVED FROM THE JOB SITE.
- ALL FITTINGS SHALL BE INJECTION MOLDED FITTINGS, FABRICATED FITTINGS ARE NOT ALLOWED EXCEPT AS PERMITTED BY THE TOWN ENGINEER. 2. .
- JOINTS FOR PVC PIPE SHALL BE OIL RESISTANT COMPRESSION RINGS OF ELASTOMERIC MATERIAL COMPORING TO ASTM D3312. MANUFACTMERT'S INSTRUCTIONS FOR MISTALLITION SHALL BE FOLLOWED. SOLVENT CEMENT JOINTS SHALL NOT BE PERMATED.
- DUCTLE IRON PIPE:
- DUCTLE IRON PIPE SHALL CONFORM TO ANWA (151/A21.50 & A21.51. PIPE SHALL HAVE EITHER THE RUBBER-RING TYPE, PUSH-ON JOINT, OR STANDARD MECHANICAL JOINT.

FORCE MAIN HOPE FORCE MAIN SEWER PIPE SHALL CONFORM WITH ASTM D3035. PVC FORCE MAIN PIPE SHALL CONFORM WITH ASTM D3241 OR ASTM D1785. FORCE MAIN QLEANOUT VALVES AND FITTINGS SHALL BE INSTALLED IN MANDLES MEETING THE REQURIEMENTS OF DVV-W0 704.12 TO 704.37.



NOT TO SCALE

### GRAVITY SEWER PIPE TESTING PER Env-Wo704.06 ALL NEW GRAVITY SEWERS SHALL BE TESTED FOR WATER TIGHTNESS BY THE USE OF LOW-PRESSURE AIR TESTS.

- LOW-PRESSURE AIR TESTING SHALL BE IN CONFORMANCE WITH (2) A. ASTM F1417 "STANDARD TEST METHOD FOR INSTALLATION ACCEPTANCE OF PLASTIC GRAVITY SEWER LINES USING
- ALL NEW GRAVITY SEWERS SHALL BE CLEANED AND VISUALLY INSPECTED USING A LAMP TEST AND BY INTRODUCING WATER TO DETERMINE THAT THERE IS NO STANDERG WATER IN THE SEWER; AND TRUE TO LIVE AND GRADE FOLLOWING INSTALLATION AND PRIOR USE.
- ALL PLASTIC SEWER PIPE SHALL VISUALLY INSPECTED AND DEFLECTION TESTED NOT LESS THAN 30 DAYS NOR MORE THAN 90 DAYS FOLLOWING THE INSTALLATION.
- THE MAXIMUM ALLOWABLE DEFLECTION OF FLEXIBLE SEVER PIPE SHALL BE 5 PERCENT OF AVERACE #SOLE DIAMETER A RICO BALL OR MANDREL WITH A DUMETER OF AT LEAST 0325 OF THE AVERACE INSIDE PIPE DIAMETER SHALL BE USED FOR TESTING PIPE DEFLECTION. THE DEFLECTION TEST SHALL BE CONDUCTED WITHOUT MECHANICAL PULLING DEVICES.

FORCE MAIN

(1)

(3)

(4)

(5)

FORCE MAINS SHALL BE TESTED IN ACCORDANCE WITH SECTION 5 OF AWWA COOD "INSTALLATION OF CAST IRON WATER MAINS AND THEIR APPURTENAMCES", STAMDARD IN EFFECT WHEN THE TEST IS CONDUCTOR, AT A PRESSURE EQUAL TO THE GREATER OF 150 PERCENT OF THE DESIGN OPERATING TOTAL DYNAMIC MEAD OR AT LEAST 100 PSI.

SEVER MANHOLE NOTES:

ALL YERK THENDELLE, CULLENS: 1. IT IS THE WITEHTIGN THAT THE MANNELL, INCLUDING ALL COMPONENT PARTS, MAVE ABEDIANT SPACE, STRENGTH AND LEADWRDDF GUALTHES CONSIDERED INCISSIANT FOR THE INFORCE SERVICE REDUREDON'S AND CONTIGRATIONS SHALL BE AS SHOWN IN THE DRAVING MANNULS MAY BE AM ASSEMBLY OF PRECASATY AND AND LEADWING MANNELS MAY BE AM ASSEMBLY OF PRECASATY AND AND LEADWING MANNELS MAY BE AM ASSEMBLY OF PRECASATY AND AND LEADWING MANNELS MAY BE SHALL BE DF SICH MATERIA, AND EMALITY AS TO VITATING LEADER DF B THE LIFE DF MICH MATERIA, AND EMALITY AS TO VITATING LEADER DF B THE LIFE DF THE STREAM AND EMALTY AS TO VITATING LEADER DF B THE LIFE DF THE STREAM AND EMALTY AS TO VITATING LEADER DF B THE LIFE DF THE STREAM AND EMALTY AS TO VITATING LEADER DF B THE LIFE DF THE STREAM AND EMALT AND EMALTY IN CONSENS FOR THE LIFE DF THE STREAM AND EMALT AND EMALINE AS TO VITATING LEADER DF B MAY AS ALLEN DF THE STREAM AND EMALT AND EMALT AND LEADER DF B MAY A LIFE DF THE STREAM AND EMALT AND EMALLY IN CONSENS AND A LIFE DF DECESS OF 251 YES.

LARCENT CONCRETE NAME LES SHALL CONFORM WITH ASTM CA78 PER DAV-VO TOALS GACD BARRELS AND CODE SICTIONS SHALL DE REDAFFRED CONCRET PER Env-VO 704126A PERCAST CONFORTE BARREL SECTIONS CONS AND BASES SHALL CONFORM TO ASTM CA78. ALL PRECAST SECTIONS CONS AND BASES SHALL CONFORM TO ASTM CA78. ALL PRECAST SECTIONS AND BASES SHALL HAVE THE BATE OF MMCFACTURER AND THE NAME TRAEDWARD OF THE MANFACTURER DATESSED OF DODLINEY WHERE ON THE SIZE VALL BASE SECTIONS SHALL READOLINE TO A PEDITA AT LEAST 6" ABOVE THE DREDSEND OF DICCHING PIPE PER Env-VQ 70412(4).

3. ALL SEVERS, NAMERLES AND FORCEMAINS SMALL BE TESTED FOR VATER TIGHTNESS BY USE OF EITHER VATER OR LOW PRESSURE AIR TESTS, LOW PRESSURE AIR TESTS SHALL CONFORM TO ASTM CRER. SERVICES TO BE TESTED AT SAME THE AS MAINS RETUR TO BUILDING COMPETION.

4. DIVERTS AND SHELVED HAMMOLES SHALL HAVE A BRICK FAVED SHELF AND INVERT CONSTRUCTD TO CONTONN TO THE SIZE OF PIPE AND FLOW. AT DWARES IN DIRECTIONS THE DIVERSISHED BE LAID DUT IN CLIPVES OF THE LONGEST RABBLE POSSIBLE TAMEENT TO THE CONTON LINE OF THE SIZVER PIPELS. SHELVES SHALL BE CONSTRUCTED TO THE CLOVED THE KIGHEST PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE THE KIGHEST PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM AND SLOPE TO BRAIN TOWARD THE FLOWARD THE RECOVER PIPE CREAM STATE SHALL CONSTRUCTION AND LODGELS WITH STATE CREAMEST OF MANEEL BRICK MASDING SHALL BONGTON WITH ASTRUCE LONGERLATIONT OF MANEEL BRICK MASDING SHALL CONSIST WITH ASTRUCE LONGERLATION TO THANGELE BRICK MASDING SHALL CONSIST WITH ASTRUCES PER DWA-VO 704.1200 DIVERT AND SHELF SHALL BE BRICK HASDART FOR DWALED TO THANGEL DWYERT AND SHELF SHALL BE BRICK HASDART FOR DWALED TO THANGEL

S. BEDDINGSCREENED CRUSHED STONE FREE FROM CLAY, LOAN, ORGANIC MATTER AND MEETING ASTH COD

6. FLEXIBLE JOINT: A FLEXIBLE JOINT SHALL BE PROVIDED VITHIN THE FOLLOWING DISTANCES FROM HANNELE JOINTS, ROP CI PIPE - ALL SIZES - VITHIN 48" PVC GREATER THAN 15" - VITHIN 48"

THER MEETING AT THE CLO MEETING IN A STEVE AND A STATEMENT OF A STATEMENT OF A STEVE ASSING JA DICH SCHEDN OF AS PASSING IN STEVE AND ASSING JA DICH SCHEDN OF A STATEMENT AND A STATEMENT AND

7. SHALLOV HANNOLE IN LIEU OF A CONC SECTION, VHON HANNOLE DEPTH IS LESS THAN & FEET, A REDEFORCE CONCRETE SLAB COVER HAY BE USED HAVING AN ECCENTRIC ENTRANCE OPENING AND LAPAILE OF SUPPORTING H-20 LIAMS.

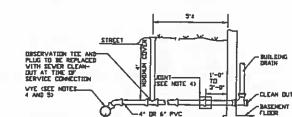
B. HORIZOMTAL JOINTS BETVEEN SEETIDAS OF PRECAST CONCRETE BARRELS SMALL & OF OVERLAPPING TYPE, VHICH SHALL BEPUID FOR VATER TIGHTNESS UPDA AN ELASTORERIE OR MASTIE-LINE SEALANT.

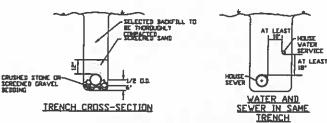
10. FOR BITUMASTIC TYPE JOINTS THE ANDUST OF SEALANT SHALL BE SUFFICIENT TO FELL AT LEAST 75 X OF THE JOINT CAVITY APPRIVED BITUMASTIC SEALANTS

TURNSTEL SEALANTS RAN - NES ROTT SEAL NG & BUIRLE ALL GASSETS AND SEALANTS SHALL BE INSTALLED IN ACCORDANCE VITH MANUFACTURERS VRITTEN INSTRUCTIONS.

12. MORTAR SHALL CONFORM WITH ENV-WO 704.13 MORTAR SHALL BE TYPE 1/11 PORTLAND CONFORM

12. UNLESS OTHERVISE NOTED ALL GRANLAR NATERIAL SHALL BE PLACED IN 12" LIFTS AND COMPACTED TO 95% OF THE MODIFIED PROFINE TEST. 13. STEPS ARE NOT ALLOVED

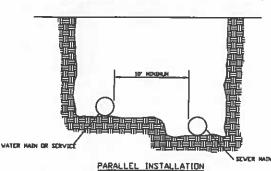


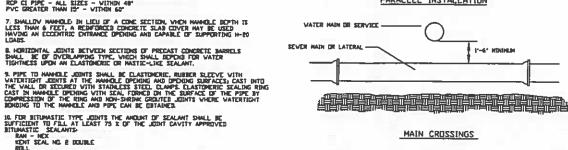


SANITARY SEVER SERVICE DETAIL NOT TO SCALE

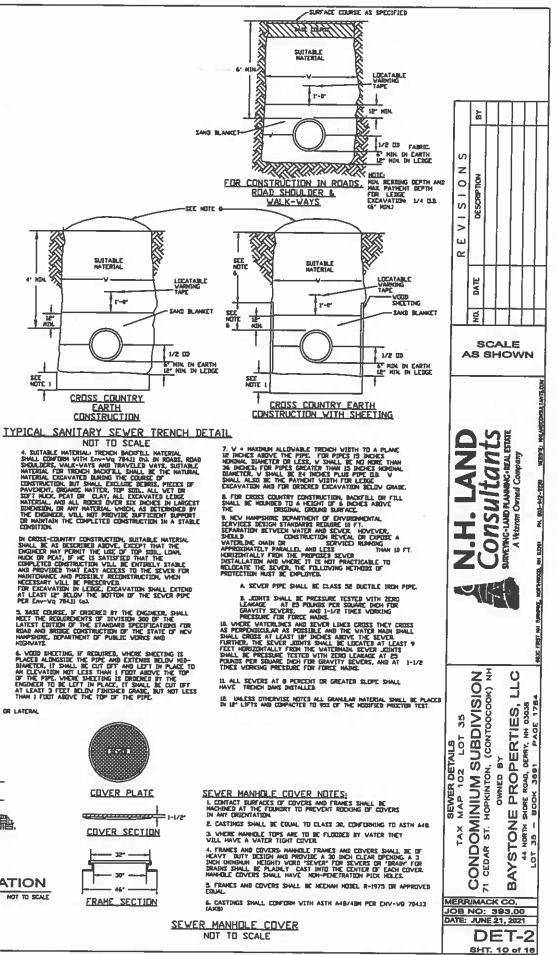
STATE IBLACK NUTTED 1. ORDERED EXCAVATION OF U BELOV GRADE ACTULL VITH SEE ALSO KOTE 7. BEDOUGG PCD Ever-Vog Fill FREE FROM ELAN NATTER AND MEETING STORE SIZE NO. 67.	BEDDING HATERIAL. 7041(ca) SAND 7. LOAN, DIRGANC ASTH C33-03
LOOX PASSING 90-100X PASSING 20- JIX PASSING 0 -10X PASSING 0 - JX PASSING	1 JNCH SCREEN 3/4 JNCH SCREEN 3/8 JNCH SCREEN 6 4 SIEVE 8 8 SIEVE
THE TRENE	HE ENGINEER TO STABILIZE H BASE, GRADED CRUSHED 1/2 DICH SHALL HE USED
NATURAL SAND CONFERENCE TO SPECIFICATIONS FOR CONCEPT	11(b) SAND SWALL CONSIST OF DERT I THE ASTH STANDARD E G'DHEI AGGREGATES, DESIGNATION
CJ3 AS FOLLOW PASSING 1002 PASSING 80-1002 PASSING 50-1002 PASSING 10-202 PASSING 10-302 PASSING	3/R DICH SCREEN 0 4 SIEVE 0 0 SIEVE 0 16 SIEVE
10 - 302 PASSING FINENESS HODULUS	# 50 SIEVE 8.3-3.1

3.) PLTER FABRIC SHALL BE INSTALLED ABOVE PIPE - MIRAFI 140H OR EDUAL



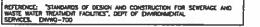


WATER MAIN / SEWER MAIN SEPARATION

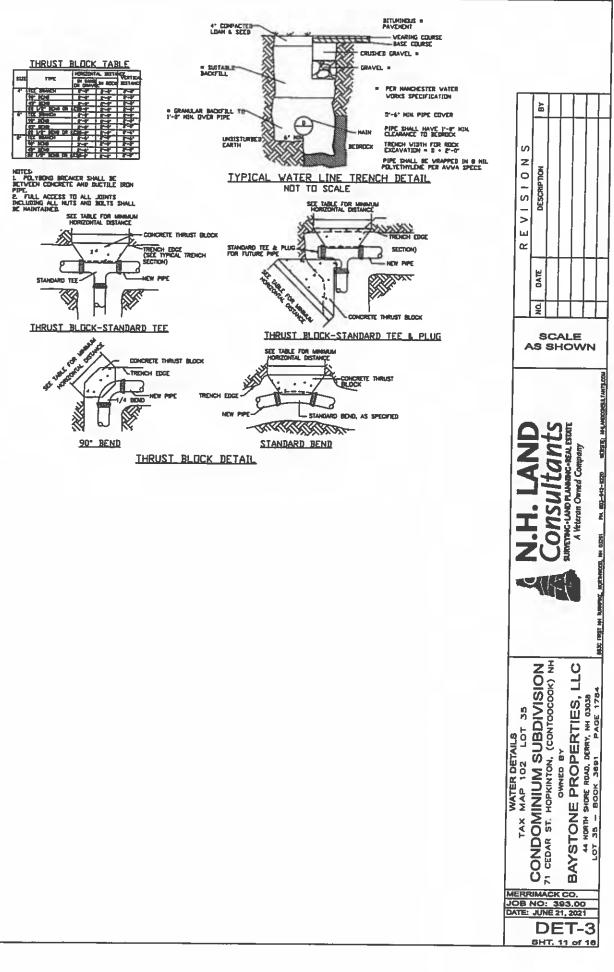




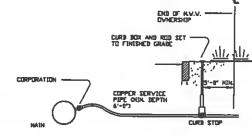
	- 32"	
1	- 30"	



STREET SEVER



D END OF N.V.V. -CURB BOX AND ROD SET all ML CURPURATIO COPPER SERVICE PIPE OUN. DEPTH 6'-0'7 CURB STOP



CORPORATIONS SHALL BE TAPPED BIRECTLY TO THE HAIN DI SIZES UP TO 1" # CINCLUSIVEX.

CORPORATIONS 1-1/2"# AND GREATER SHALL BE INSTALLED USING A TAPPING SADDLE AND SHELL CUTTER

4

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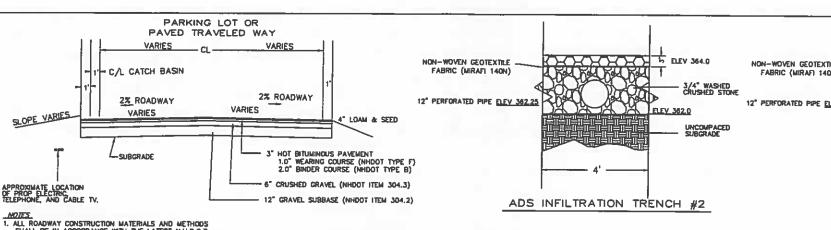
TYPICAL SERVICE CONNECTION

NOT TO SCALE

### GENERAL NOTES

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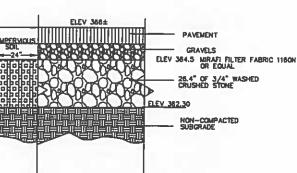
- ANNAUAL ACCEPTABLE STANDARDS FOR ALL CONSTRUCTION MATERIALS AND METHODS SVALL BE IN ACCERDANCE WITH THE NEW HAMPSHIE OF TRANSPORTATION (HODDT), STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST REVEND, (AND ALL SUBSECLENT ANENDMENTS) AND THE TOWN OF (F) SOM REGULATIONS, DRAINAGE DESION IS BASED ON THE "STORMWATER MANAGEMENT AND RECORD AND SEDMENT CONTROL HANDBOOK VELIME I
- ALL ELEVATIONS AND LOCATIONS OF DISTING UTULTY AND DRAINAGE STRUCTURES SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR PROFIT TO UTULZATION OF DESIGN ELEVATIONS ON THIS PLAN.
- BACIDILL OF TRENCHES AND ALL PAVED AREAS SHALL BE COMPACTED IN ACCORDANCE WITH NH DOT-STANDARD SPECIFICATIONS-SECTION 304.
- THE CONTRACTOR BHALL TAKE ALL NECESSARY MEASURES AND SHALL PROVIDE ALL NECESSARY CONTINUOUS BARRENS OF SUFFICENT TYPE, SZE AND STRENGTH TO PREVENT ACCESS TO ALL OPPL EXCANATIONS AT THE COMPLETION OF EACH DAYS WORK.
- ALL ELEVATIONS ARE BASED ON U.S.G.S. DATUM
- The contractor shall be aware of HS responsibility to contact "dig safe" at 11 SC. BEDFORD STREET, BURDROTON, MA (1-888-344-7233) at least 72 working house phore to the start of any decayation.
- SHORING AND STABILIZING OF TRENCH SIDEWALLS DURING EXCAVATION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- ALL WORK ADJACENT TO EXISTING OF AR STREET SHALL BE PERFORMED IN WITH THE STREET OPDIMING REQUIREMENTS OF THE TOWN OF HOPMITCH AND INH DOT STANDARD SPECIFICATIONS.
- ALL CALVERTS, DRAMAGE STRUCTURES AND ROAD CONSTRUCTION SHALL BE SUBJECT TO PARTIAL AND THAL INSPECTION FINDER TO ACCEPTANCE BY THE TOWN, THE CONTRACTOR IS RESPONSELE FOR SCHEDULING AND COORDINATING INSPECTION BY THE TOWN ENGINEER.
- 10. UTILITY PLANS SHALL BE SUBMITTED TO THE TOWN ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
- 11. THE CONTRACTOR SHALL PROVIDE A MINIMUM OF 4" TOPSOL AND SEE OVER ALL DISTURBED UNPAVED AREAS LINLESS OTHERWISE SPECIFIED.
- 12. DND SECTIONS, (FLARED DNDS) SHALL COMPLY WITH MHOOT STANDARD SPECIFICATIONS, MORMAY DESIGN MARAAL, PLATES 5 & 6, 07 STANDARD 11 DATED 1978 MO ALL SUBSECUDIT AMEMBENTS.
- 13. ALL DRIVEWAY GRADING IS SUBJECT TO DEPARTMENT OF PUBLIC WORKS REVIEW PRIOR TO DRIVERAY CONSTRUCTION ON INDIVIDUAL LOTS. DRIVEWAY CLEVERTS, LOCATED OUTSDE OF THE TOWNS RIGHT OF WAY, MAY BE NECESSARY DEPENDING ON THE ACTUAL PROPOSED LOT DEVELOPMENT.
- 14. ALL PAVEMENT MARKERS SHOWN CONFORM TO THE LATEST EDITION OF THE NEW HAMPSHIEL DEPARTMENT OF TRANSPORTATION STANDARDS PLANS FOR ROAD AND BRODE CONSTRUCTION.



- MOZES. 1. ALL ROADWAY CONSTRUCTION MATERIALS AND METHODS SHALL BE IN ACCORDANCE WITH THE LATEST N.H.D.O.T. SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. 2010 INCLUDING SUBSEQUENT AMENDMENTS AND EDITIONS.
- 2. PROVIDE 4" (MIN.) COMPACTED LOAM AND SEED ON ALL SIDE SLOPES AND DRAINAGE SWALES UNLESS OTHERWISE NOTED.
- 3. ALL LEDGE AND ROCK SHALL BE REMOVED TO 6" BELOW SUBGRADE.

4. ROADWAY UNDERDRAIN SHALL BE PROVIDED IN ALL CUT SECTIONS (AT SIDE WITH CUT) AND WHERE SEASONAL HIGH WATER IS WITHIN FOUR (4) FEET OF FINISHED GRADE IN ALL OTHER AREAS. UNDERDRAIN SHALL HAVE A MINIMUM OF FOUR (4) FEET OF COVER.

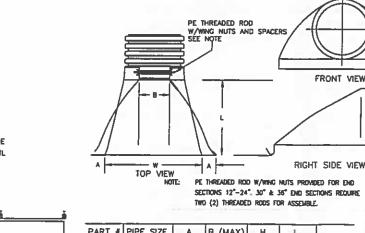
TYPICAL ROADWAY SECTION TO SCALE



ADS INFILTRATION SYSTEM DETAIL

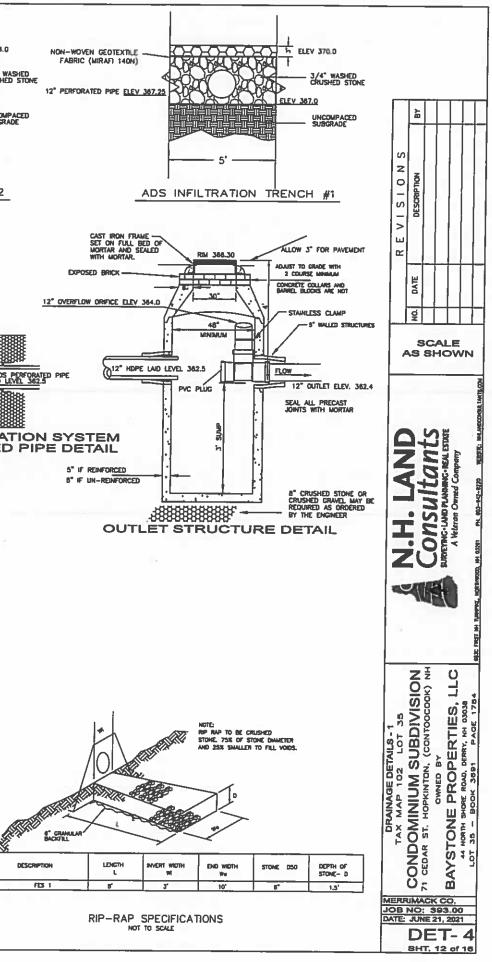
### IMPERVIOUS SOIL

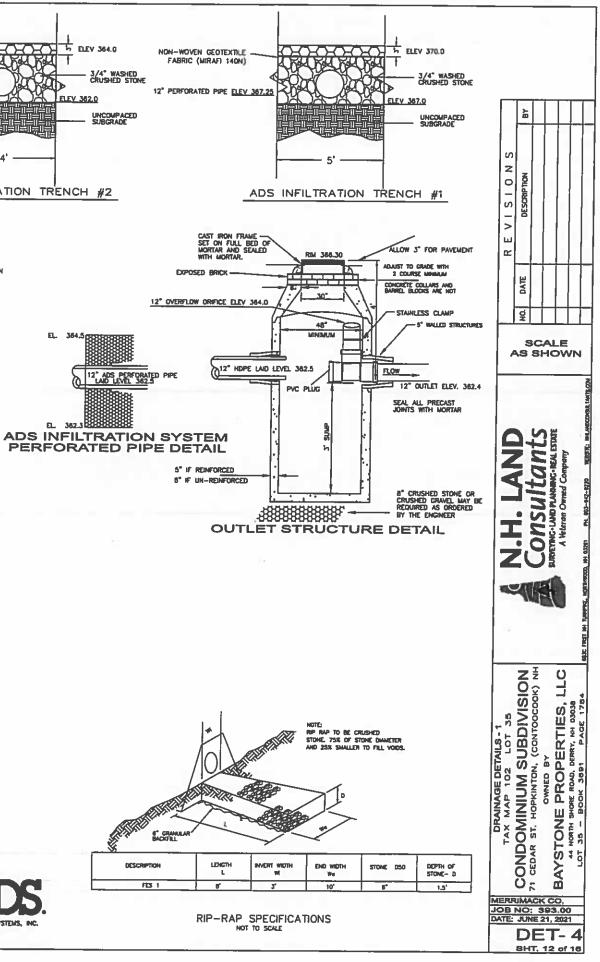
- NOTES 1. IMPERIOUS CORE AND POIND LINER SCIL SAMPLES AND SERVE ANALYSIS TO BE SUBSITIED FOR APPROVAL PRIOR TO CONSTRUCTION. APPROVALS SOLS TO MEET THE FOLLOWING ORTIDAL SOL SHALL HAVE NO DREAME MATTER OR PROZEN MATERIAL AND NO STORES LARDER THAN 2/3 OF THE MAXIMUM LIFT SOLE. STORES AND/OR THAN 2/3 OF THE MAXIMUM LIFT SOLE. STORES AND/OR THE FOLLOWING GRADA CONDUCTS SHALL NOT EXCEED 3 MORES. FILL MATERIAL SHALL HAVE THE FOLLOWING GRADA
  - PEVE SIE: R PASSING #100 #100 80 - 80 40 - 80 25 - 45
- THE CONTRACTOR BHALL USE CARE NOT TO OVER EXCAVATE AND DISTURB THE EXISTING SOL AT THE BERM AND DUTLET PIPE AREAS.
- IMPERVICUS SCIL SHALL BE INSTALLED WITHIN THE LIMITS SHOWN ALONG THE BETM AREA. THE SCIL SHALL BE KEYTED 12" WITH THE BOTTOM OF THE POND AND SHALL BE PLACED IN LIFTS NOT DISCEEDING IF AND COMPACTED TO A MINIMAM 95% OF THE WET WEDRY ADDRESS AND THE STRING (ASTIN 1557)
- At the impervals dam area. The dam shall be reved with the dottom and sides of the tribunation  $\mathcal{F}$  . The impervals sol shall be fraced in Links and Lord the data of  $\mathcal{F}$  . The impervals sol shall be fraced in the wet wident as determined TESTING (ASTM 1557)

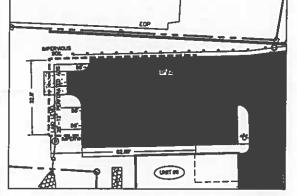


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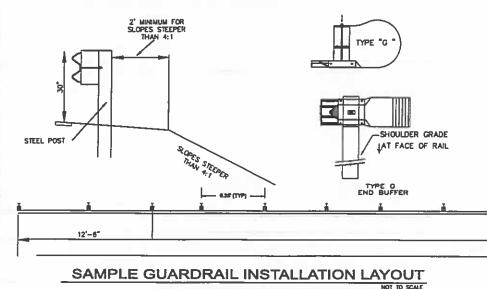
PART	PIPE SIZE	A	B (MAX)	H	L	
210NP					25.00 IN	
510NP			10.00 IN			
810NP	18 N	7.50 IN	15.00 IN	6.50 IN	32.00 IN	
2410NP	24 IN	7.50 IN	15.00 IN	6.50 IN	36.00 IN	
3015NP	30 IN	7.50 IN	12.00 IN	8.60 IN	58.00 IN	 ADVANCED DRAINAGE SYSTEMS, INC.
5615NP	36 IN	7.50 IN	25.00 IN	8.60 IN	58.00 IN	

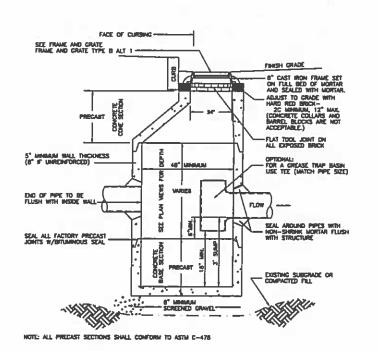




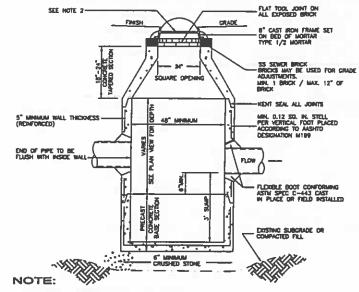


ADS INFILTRATION SYSTEM PLAN VIEW











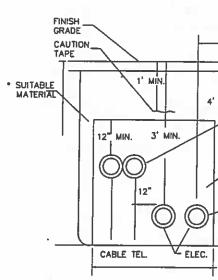
2. USE INDOT TYPE & ALT-1 BICYCLE SAFE IN ALL PAVED AREAS, DITCH GRATES IN ALL SWALES

3. POLYETHYLENE INSERT LINERS ARE TO BE INSTALLED ON ALL NEW CATCH BASINS

### CATCH BASIN

SINGLE PIPE SYSTEM

NOT TO SCALE

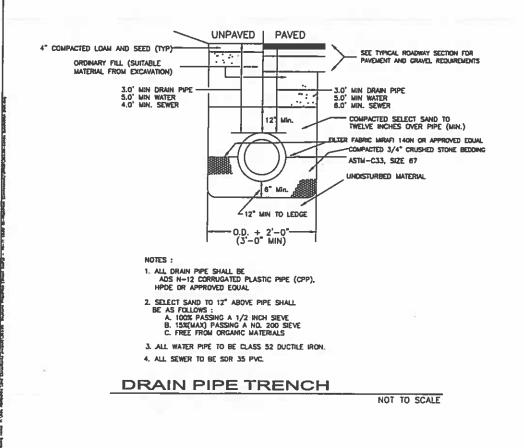


SUITABLE MATERIAL: IN ROADS, ROAD SHOULDERS, WALKWAYS AND TRAVELED WAY FILL SHALL BE THE NATURAL MATERIAL EXCAVATED DURING THE COURSE OF CON-PIECES OF PAVEMENT, ORGANIC MATTER, TOP SOL, ALL WET OR SOFT MUCK, PEA MATERIAL, AND ALL ROCKS OVER SIX INCHES IN THE LARGEST DIMENSION, OR ANY THE TOWN ENGINEERS, WILL NOT PROVIDE SUFFICIENT SUPPORT OR MAINTAIN THE CONDITION. SUITABLE MATERIAL SHALL BE PLACED IN 6" LIFTS AND THOROUGHLY

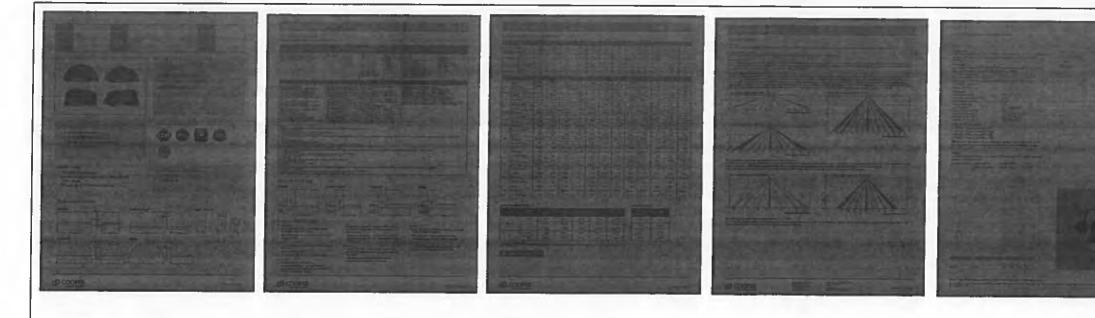
IN CROSS-COUNTRY CONSTRUCTION, SUITABLE MATERIAL SHALL BE DESCRIBED AS ENCINEERS MAY PERMIT THE USE OF TOP SOIL LOAM, OR PEAT. IF SATISFIED THE ENTIRELY STABLE AND PROVIDED THAT THE EASY ACCESS TO THE STRUCTURES FI RECONSTRUCTION, WHEN RECESSARY WILL BE PRESERVED SUITABLE MATERIAL SHA THOROUGHLY COMPACTED.

NOTES: 1. UTILITIES SHALL BE INSTALLED ACCORDING TO THE RESPECTIVE UTILITY SPECIFICATIONS. 2. ALL ABOVE GRADE UTILITIES MUST BE PLACED OUT OF THE R.O.W. AND WITH THE ROADWAY DRAINAGE SYSTEM. PLACEMENT OF TRANSFORMERS CANN OF R.O.W. AND PROPERTY CORNER MONUMENTS.

### UNDERGROUND UTILITIES TR

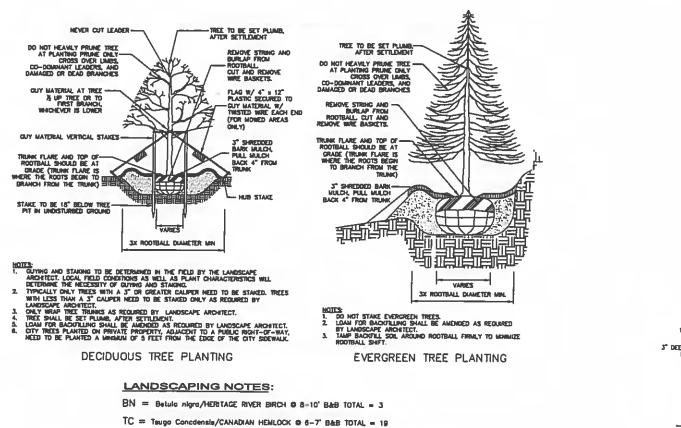


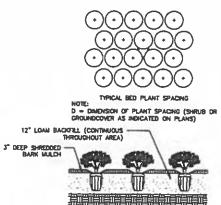
# TO BODE OF PROPERTY	
	8
L	A
DIAMETER VARIES, CONTACT RESPECTIVE UTILITY COMPANY FOR REQUIRED SIZE	ν
N.H.D.O.T. SCREENED SAND 6" BELOW & ON THE SIDES OF THE PIPE	S I O N DESCRIPTION
12" ABOVE THE PIPE (MIN.)	
FROM RISER POLES TO END OF SADDLE HILL DR. SINGLE 3" TO REMAINING CUL-DE-SACS	ж >
NOTH AS REQUIRED BY RESPECTIVE UTILITY	OATE
S, SUITABLE MATERIAL FOR TRENCH BACK	QN III
AND THAT, BALL EXCAVATED LEDGE MATERIAL, WHICH, AS DETERMINED BY COMPLETED CONSTRUCTION IN A STABLE COMPACTED.	SCALE AS SHOWN
ABOVE, EXCEPT THAT THE TOWN COMPLETED CONSTRUCTION WILL BE DR MAINTENANCE AND POSSIBLY LL BE PLACED IN 12° LIFTS AND	ND nts ex. crue eny sere execution
COMPANY STANDARDS AND	
IOT CONFLICT WITH THE INSTALLATION	
ENCH	H. LA ISUIT
NOT TO SCALE	
	ZOT
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	He (SAL) 378
	DRAINAGE DETALS - 2 TAX MAP 102 LOT 35 NDOMINIUM SUBDIVISI AR ST. HOPKINTON, (CONTOOCOOM OWNED BY STONE PROPERTIES, L 44 NORTH SHORE RAUL, DEMTY, HH 0303
	DRAINAGE DETAILS - 2 TAX MAP 102 LOT 35 OMINIUM SUBDIV ST. HOPKINTON, (CONTOO OWNED BY ONE PROPERTIES ONE PROPERTIES
	DRAINAGE DETAILS - 2 VX MAP 102 LOT 3 MINIUM SUBDI MINIUM SUBDI OWNED BY OWNED BY ONE PROPERTIE
	AGE DETA (P 102 IIUM SU PKINTON, ( OWNED BY OWNED BY CON 3561
	DRAINAGE DET TAX MAP 102 NDOMINIUM SI EDAR ST. HOPKINTON. OWNED BY YSTONE PROPE
	DRAINAGE DETAILS - 2 TAX MAP 102 LOT 35 CONDOMINIUM SUBDIVISION 71 CEDAR ST. HOPKINTON, (CONTOOCOOK) NH OWNED BY BAYSTONE PROPERTIES, LLC 44 NORTH SUGE RAAD, DERNY, HI 03038 LOT 33 - BOOK 3561 PAGE 1784
	MERRIMACK CO. JOB NO: 393.00



Luminaire Sc	hedule			
Symbol	Qty	Label	Arrangement	Description
Ð	12	W	SINGLE	66411/ WALL MID 10' AFG
	2	W1	SINGLE	ISS-SAIB-740-U-SL3/ WALL MTD 15' AFG

PR = Pims resinoso/RED PINE @ 7-8' B&B TOTAL = 10





TYPICAL PERENNIAL DETAIL NOT TO SCALE

DO NOT HEAVELY PRUNE TREE AT PLANTING PRUNE ONLY CROSS OVER LIMES CO-DOMINANT LEADERS, AND DAMAGED OR DEAD BRANCHES

GUY MATERIAL AT TREE -) UP TREE OR TO FIRST BRANCH, WHICHEVER IS LOWER

GUY MATERIAL VERTICAL STAKES

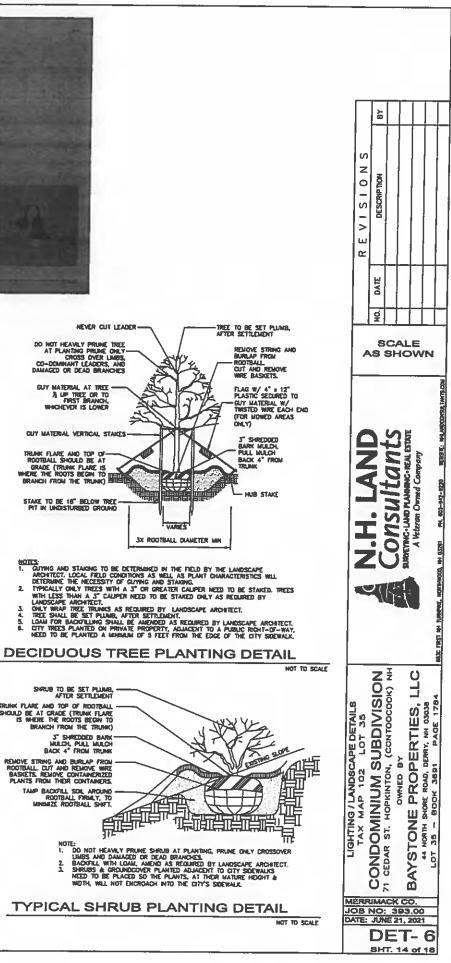
TRUNK FLARE AND TOP OF-ROOTBALL SHOULD BE AT GRADE (TRUNK FLARE IS WHERE THE ROOTS BECAN TO BRANNE FROM THE TRUNK BRANCH FROM THE TRUN

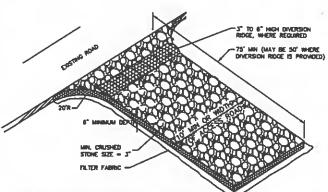
STAKE TO BE 16" BELOW TREE PIT IN UNDISTURBED GROUND

	1.45 5
1.	GUYING AND STAKING
	ARCHITECT, LOCAL FIEL
	DETERMINE THE NECES
2	TYPICALLY ONLY TREES
	WITH LESS THAN A 3"
	LANDSCAPE ARCHITECT
3.	ONLY WRAP TREE TRU
4.	TREE SHALL BE SET P
5.	LOAM FOR BACKFILLING
6.	CITY TREES PLANTED
	NEED TO BE PLANTED

SHRUB TO BE SET PLIME, AFTER SETTLEMENT TRUNK FLARE AND TOP OF ROOTBALL SHOULD BE AT GRADE (TRUNK FLARE IS WHERE THE ROOTS BEGIN TO BRANCH FROM THE TRUNK) 3" SHREDDED BARK MULCH, PULL MULCH BACK 4" FROM TRUNK REMOVE STRING AND BURLAP FROM ROOTBALL CUT AND REMOVE WIRE BASKETS, REMOVE CONTAINERIZED PLANTS FROM THER CONTAINERS.

TAMP BACKFILL SOIL AROUND ROOTBALL FIRMLY, TO MINIMIZE ROOTBALL SHIFT.





- MANTEMANTS I. THE DET SAALL BE MANTANED IN A CONDITION THAT WILL PREVENT TRACKING OF SEDMENT CHILD PUBLIC I. THOTTI-OF-TAX, INFOLTING CONTROL PAD BECOMES RESTLETING THE STORE SHALL BE REMOVED ALDRO WITH THE COLLECTED SOL MATERIAL RECRADED ON SIT, AND STABULED. THE DITRAMES SHALL THEN BE RECEMPTRICITED.
- 2. THE CONTRACTOR SHALL SHEEP THE PAVEMENT AT EXITS WHENEVER SOL MATERIALS ARE TRACKED ONTO THE ADJACENT PAVEMENT OR TRAVELED WAY.
- 1. WHEN WHEL WASHING IS RECURED, IT SHALL BE CONDUCTED ON AN AREA STABILIZED WITH ADDREASE, WHICH DRUMS WITO AN APPROVED SEDMENT-TRAPHING DEVICE. ALL SEDMENT SHALL BE PREVENTED FROM DIFEDING STORM DRAMES, DITCHES, OR WATERWAYS.
- CONSTRUCTION SPECIFICATIONS 4. OR Y CONSTRUCTION TRAFFIC LEANING THE SITE IS REQUIRED TO USE THE TOUROMARY STABLIED DOT. CONSIDER PROVINGING AS DEPARATE, UNPROTECTED, DUTIANCE FOR TRAFFIC DIFERING THE SITE, THES WILL INCREASE THE LONGENTY OF THE STABLIED DOT BY DIMINIATING HEAVY LOADS DIFERING THE SITE AND REDUCING THE TOTAL TRAFFIC OVER THE DEVICE.
- THE DATE AND SHALL BE MANTAHED IN A CONDITION THAT WILL PREVENT TRACISING DR FLOWING DR SEDWENT ONTO PUBLIC ROATS-OF-WAY, THIS MAY REQUEE PUBLICS TOP DRESSING WITH ADDITIONAL STORE OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND UREPARE MAYOR MAINTEDIANCE DR ANY MELAURES USED TO TRAP STORE-OF CONDITIONS DUANCH, AND USED ANY MAYOR DUANCE DR ANY MELAURES DUANCE AND USED STORE STORE OF CONDITIONS DUANCE DR ANY MELAURES DISTORE DR ANY MELAURES DR
- 8. STONE FOR A TEMPORARY CONSTRUCTION EXIT SHALL BE 3 INCH STONE, RECLAMED STONE, OR RECYCLED CONCRETE EDUVALENT.
- 7. THE MINIBALM LENGTH OF THE PAD SHALL BE 75 FEET, EXCEPT THAT THE MINIBALM LENGTH MAY BE REDUCED TO 30 FEET IF A 3-NICH TO 8-NICH NGH BERM IS INSTALLED AT THE DITRANCE OF THE PROJECT SITE.
- IL THE THOMESS OF THE STONE FOR THE STABILIZED ENTRANCE SHALL NOT BE LESS THAN & INCHES.
- S. THE WOTH OF THE ENTRANCE SHALL NOT BE LESS THAN THE FULL WOTH OF THE COT OR 10 FEET, WHICH EVER IS GREATER. 10. GEOTEXTLE FILTER CLOTH SHALL BE PLACED OVER THE ENTIRE AREA PROR TO PLACING THE STONE.
- 11. ALL SURFACE WATER THAT IS FLOWING TO OR EVENTED TOWARD THE CONSTRUCTION DUT SHALL BE PAPED BENEATH THE DIFFLANCE IF PAPING IS MARACTICAL, A BEINM WITH \$21 SLOPES THAT CAN BE CROSSED BY VORCES MAY BE SUBSTITUTED FOR THE PAPE.

### TEMPORARY CONSTRUCTION EXIT NOT TO SCALE

### CONSTRUCTION SEQUENCES:

MITE: - ALL FRISTIN CONTRES TO BE DISPECTED WITHLY NO ATTER EVERY SY OF BANKAL. MAXIMUM AREA TO BE DISTURBED AT ONE TIME IS IS ACRES.

PROR TO CONSTRUCTION INSTALL FABRIC SELIXION FERCING AS SHOWN ON PLAN CONSTRUCT TEMPONIARY STABLIED DYNAMICE, AND INSTALL OTHER APPROPRIATE SEDIMORT AND EXOSON CONTROL CONSTRUCT TEMPONIARY STABLIED DYNAMICE, AND INSTALL OTHER APPROPRIATE SEDIMORT AND EXOSON CONTROL

- CONSIGNALE LABORMENT SUBMILLED LETINGEL, AND PEORLE LITER APPARTMENT SUBMERT AND DECAME LABORE. 2. CLF AND CLEAR ALL RECEARDS AND SUBMERT FROM LES ELEVES, POORS, AND SIME AND RECK. NAZE DESTING BUILDINGS ALL WATCHAL TO BE DEPOSED OF IN ACCORDINCE WITH LICON, STATE AND FEDERAL RECLARICIES.
- J. COMPLETE TEMPONINY SEDIMENT BIGHES AT POIND LOCATORIS. BASING AND SIMILES MUST BE STANLIZED PRIOR TO DREETING RANGET TO THOSE CONSTRUCT BEINES AND SIMILES TO DREET STOMMARTER TO BASING. BEDIMENT BASING LOCATOD WITHIN PROPOSED BIFLITUTION AREAS ARE TO BE CONSTRUCTED & MIRINUM OF 1" ABOVE THE TIME, BASIN FLOOR FLEWATOR
- AN ARA IS CONSIDERED STARLE F: A) BACK CONSECTIONES TARLE IF: B) A MARAN IS CONSECTIONES THAT EED IN AREAS TO BE PARTY. B) A MARAN OF SEX VECTATIO CONTINUE BEDIN ISTANLESSING. C) A MARAN OF SEX VECTATIO CONTINUE AND AND A SUCH AS STORE, ON HAM-RAP HAS BEDIN INSTALLED, ON D) EMISSION CONTING REMARKED HAVE BEDIN MERCENT AS STORE, ON HAM-RAP HAS BEDIN INSTALLED, ON
- 5. CUT AND CLEAR ALL VEDETATION AND STUMPS FROM AREAS TO BE DISTURBED FOR THE CONSTRUCTION OF THE
- CONSTRUCT, OUT, AND FILL SLOPES. ALL CUT AND FILL SLOPES TO BE SUMRED MANDANELY AFTER CONSTRUCTION. ALL SLOPES GEALTE THAN 3-1 TO BE STABLED WITH AFE MATTING. ALL CUT AND FILL SLOPES THAN LE SEDIED AND COALD WITHIN 72 FOLDER OF ADDREVED FINES CAUCH.
- CONSTRUCT STORM DRAWAR, AND OTHER LANDERGRAVING UTLIERS. ALL SHALLS TO BE PROTECTED WITH TRANSMAY DISSIGN COMPILE, MOSARES SHORM, ALL CACH MASH OPDINGS TO BE PROTECTED WITH BLOCK AND DWICE, MILT STANDON TRUTTS AN SHORM, SEDAND TRAVE ANALYSIS MASH SHOLD BE USED STARL UPPD.
- BECH TOP SOLUCE, SEEDING AND MARCHATELY AFTER COMPLETION OF DIBMNORPHIC, TELEPONARY Disson common, / dychoon commence swall be implemented where required to previow disson of Dissonkading. Any filoson occurring swall be rowned marchately upon discovery. 10. FRESH CRUDING & PANING, ALL ROADINITS AND PANKING LOTS SHALL BE STABLIZED WITHIN 72 HOURS OF ACHIEVING FRESH CRUDES.
- 11. ALL PARED AREAS TO BE COMPLETED BY OCTOBER 15. ALL LANDSCAPED AREAS TO BE STABLIZED BY OCTOBER 1500, WITH NAY WILCH AND SETU.
- 12. COMPLET, PERMINENT SEEDING AND MALCHING OF ALL DISTURBED ANDAS, ALL TEMPORARY (FIOSION CONTROL MEASURES TO REMAIN IN PLACE LIMIT, A FULL VETERATIVE COVER INSI BEDI ISTABLISHED ON ALL DISTURBED ANDAS.
- 13. SLT FIDICES AND HAT BALE BARRENS TO BE REMOVED DACE THE STAR STARLING.

- MANITEMANCE RECURRENENTS 1. TEMPORARY SEEDING SHALL BE INSPECTED WEDLY AND AFTER ANY RAMPALL EXCEEDING & INCH IN 24 HOURS ON ARTIVE CONSTRUCTION STEEL TEMPORARY SEEDING SHALL ALSO BE INSPECTED JUST PRIOR TO SEPTEMBER 13, TO ASSERTAM INFERTER ADDITIONAL SEEDING IS RECURRED TO PROVIDE STABILIZATION OVER THE WRITEM PERIOD.
- 2. BASED ON INSPECTION, AREAS SHALL BE RESERVED TO ACHEVE FULL STABLIZATION OF EXPOSED SOLS. IF IT IS TOO LATE IN THE PLANTING SEASON TO APPLY ADDITIONAL SEED, THEN OTHER TEMPORARY STABLIZATION MEASURES SHALL BE IMPLEMENTED
- 3. AT A MINIMUM, BOX OF THE SOIL SURFACE SHALL BE COVERED BY VEDETATION
- 4. IF MAY EVIDENCE OF EROSION OR SEDIMENTATION IS APPARENT, REPARES SHALL BE MADE AND AREAS SHALL BE RESEDED, WITH OTHER TEMPORARY MEASURES (I.C., MALCH) USED TO PROVIDE EROSION PROTECTION DURING THE PERIOD OF VICETATION ESTABLISHMENT.

- SPECIFICATIONS STEL PREPARATION: 3. INSTALL NEEDED EROSION AND SEDIMENT CONTROL MEASURES SUCH AS SETATION BARMERS, DIVERSIONS, AND SEDIMENT TRAPS.
- 6. GRADE AS NEEDED FOR THE ACCESS OF EQUIPMENT FOR SEEDED PREPARATION, SEEDING, MULCH APPLICATION, AND MULCH ANCHORING. 7. RUNOFT SHALL BE DIVERTED FROM THE SEEDED AREA.
- B. ON SLOPES 4:1 OR STEEPER, THE FINAL PREPARATION SHALL INCLIDE CREATING HORIZONTAL GROOVES PERPENDICULAR TO THE DIRECTION OF THE SLOPE TO CATCH SEED AND REDUCE RUNOFF.
- STORED PREPARATION. R. STORES AND TRASH SHALL BE REMOVED SO AS NOT TO INTERFERE WITH THE SEEDING AREA.
- 10. WHERE THE SOL HAS BEEN COMPACTED BY CONSTRUCTION OPERATIONS, LOOSEN SOL TO A DEPTH OF 2 WCHES BEFORE APPLYING FERTILIZER, LINE AND SEED.
- 11, IF APPLICABLE, FERTILIZER AND ORGANIC SOIL AMENDMENTS SHALL BE APPLIED DURING THE GROWING
- ADD. APPLY LARSTONE AND FORMULER ACCORDING TO SUL TEST RECOMMEDIATIONS IF SOL, TESTING IS MAY A LARSTONE AND FORMULER ACCORDING TO SUL TEST RECOMMEDIATIONS. IF SOL, TESTING IS NOT FEASIBLE ON SMALL OR VARABLE STES, OR WHORE THING IS CONTACL, FERTURIZER MAY BE APPLED AT THE RATE OF 1000 POLINGS PER ACRE OR 13.8 POLINGS POR 1.000 SQUARE FETT OF LOW PHOSPHATE FORTULER. (N-P203-103) OR ESUFVALENT APPLY LIMESTORE (CONVALENT TO SO PORDENT CALCUM PLUS MACRESIAN ORDE) AT A RATE OF 3 TONS PER ACRE (130 LB, PER 1,000 SQUARE FET). FORTILIZER SHALL BE RESTRICTED TO A LOW PHOSPHATE, SLOW RELEASES INTRODOM FORTULER WHCH APPLED TO AREAS BETWERD 25 FET AND 25 FET FROM SAMERA EXAMPLE BODY. NO FERTILIZER CREET LANSTONE SHALL BE APPLED WITHIN 25 FET OF A SURFACE WHER BODY. THESE UNITATIONS ARE RECOMPENDING FOR ANY WATER BODY PROTECTED BY THE COMPREMENSIVE SHORELAND PROTECTION ACT.

STEDIO: 12. SELECT SEED FROM RECOMMENDATIONS IN TABLE 4-1.

- 13. APPLY SEED UNFORMLY BY HAND, CYCLONE SEEDER, DRLL, CULTPACKER TYPE SEEDER OR HYDROSEEDER (SLURRY INCLIDING SEED AND FERTILIZER), NORMAL SEEDING DEFTH IS FROM % TO & MCH. HYDROSEEDING THAT INCLUSES MULCH MAY BE LEFT ON SOL SUNFACE. SEEDING RATES MUST BE INCREASED 10 % WHCH HYDROSEEDING.
- 14. TEMPORARY SEEDING SHALL TYPICALLY OCCUR PRIOR TO SEPTEMBER 1574
- 15. AREAS SEEDED BETWEEN MAY 15TH AND AUGUST 15TH SHALL BE COVERED WITH HAY OR STRAW MULCH, ACCORDING TO THE "TEMPORARY AND PENMARENT MULCHING" PRACTICE.
- 18. VEGETATED GROWTH CONDING AT LEAST 63% OF THE DISTURBED AREA SHALL BE ACHEVED PRICE TO DOETBER 137N, IF THIS CONDITION IS NOT ACHEVED, BIPLEMENT OTHER TEMPORARY STABULZATION MEASURES FOR OVERNMENT PROTECTOR.

### TABLE 4-1. SETUNG RECOMMENDATIONS FOR TEMPORARY VEGETATION SPECIES PER ACRE BUSHELS PER 1,000 FT2 REMARKS WINTER RYE 2 BU. OR 112 LBS. 2.5 LBS. BEST FOR FALL SEEDING SEED FROM AUGUST 15 TO SEPTEMBER 15 FOR BEST COVER. SEED TO A DEPTH 2.5 BL, OR BO LBS, 2 LBS, BED NO LATER THAN MAY 15 FOR SUMMER PROTECTION, SEED TO A DEPTH OF 1 HOL. OATS 40 LBS. 1 LB. GROWS GLACK, WEITS OF SHORT DURATION. USE WHENE APPEARANCES ARE INFORTANT, SEED LARLY SPIROR AND/OR DETINED AUGUST 15 AND SEPTEMBER 15. COVER THE SEED WITH NO MORE THAN 0.25 NCH OF SOL. ANNULAL RYECTASS PEREDINIAL RYECRASS

30 LBS. 0.7 LB. DODD COVER WHCH IS LONGER LASTING THAN ANNULA. RYEGRASS. SEED BITMEIN ANNULA AND LIME I AND/OR BETNEEN ANDUST IS AND SEPTEMBER IS. MALCHING MILL ALLOW SEED TO THE THEORY AND THE GROWM SEASON. SEED TO A DETTIN OF APPROXIMATELY O.B. NOL.

TEMPORARY VEGETATION

- 14, REMOVE ACCURRINATIONS OF SEDMENT FIRM DIMANCE STRUCTURES, SEDMENT INSING AND DEEP SUMPS ALL AVEAS TO BE LONGED & WATED AS NECESSARY UPON COMPLETION OF PROJECT.
- 13. THE MAXIMUM AND AT A ALLONED TO BE DISTURBED & INSTANLIZED AT ONE TIME IS 1.5 ACHES. ALL ANCAS SHALL BE STABLIZED WITHIN 45 DAYS FROM MITAL DISTURBANCE.

IS. WHER CONSTRUCTION HOTES:

- INTER CONSTITUCTION NOTE: A) DURING WITTER CONDITIONS, THE MAXIMUM ALLOWING DISTURDED AND A DURING WITTER CONDITIONS, THE MAXIMUM ALLOWING DISTURDED AND CONTROL THAT AND A DURING WITCH CONTROL THAT AND A DURING WITCH A DURING AND A DURING WITCH AND A DURING WITCH AND A DURING WITCH A DURING WITCH AND A DURING WITCH A DURING WITC



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- CONSIDERATIONS 1. THIS PRACTICE IS INTERDED FOR USE IN AREAS OF CONSENTRATED FLOW, BUT MUST NOT BE USED IN STREAM CHANNELS (INTERNET PERSIMIAL OR INTERNITTENT).
- 2. THE CHECK DAM MAY BE LEFT IN PLACE PERMANENTLY TO AVOID UNHECESSARY DISTURBANCE OF THE SOL ON REMOVAL, BUT ONLY # THE PROJECT DESIGN HAS ACCOUNTED FOR THEIR HYDRAULC PERFORMANCE AND CONSTRUCTION PLANS CALL FOR THEM TO BE RETAINED.
- S. IF IT IS NECESSARY TO REMOVE A STORE CHECK DAM FROM A GRASSLINED CHANNEL THAT WILL BE MOMED, CARE SHALL BE TAKEN TO ENSURE THAT ALL STORES ARE REMOVED. THIS INCLUDES STORE THAT MAY MASHED DOMNSTREAM.

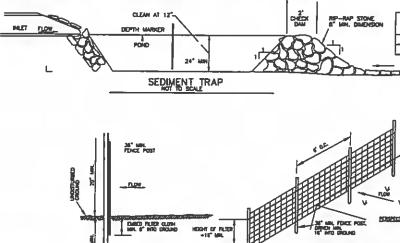
- & EROSION CAUSED BY HIGH FLOWS AROUND THE EDGES OF THE DAM MUST BE CORRECTED IMMEDIATELY. 7. IF EVIDENCE OF SILTATION IN THE WATER IS APPARENT DOWNSTREAM FROM THE CHECK DAM, THE CHECK DAM SHALL BE INSPECTED AND ADJUSTED IMMEDIATELY.
- 8. CHECK DAMS SHALL BE CHECKED FOR SEDMENT ACCUMULATION AFTER EACH SIGNFICANT RARFALL SEDMENT SHALL BE REMOVED WHEN IT REACHES ONE HALF OF THE ORDINAL HEIGHT OR BEFORE.
- STORICATIONS B. CHECK DAWS SHALL BE INSTALLED BEFORE RUNOFF IS DIRECTED TO THE SWALL OF DRAMAGE DITCH.
- 10. THE MAIDINM CONTRIBUTING DRAMAGE AREA TO THE DAM SHALL BE LESS THAN ONE ACRE.
- 11. THE MAXIMUM HEIGHT OF THE DAM SHALL BE 2 FEET.
- 12. THE CENTER OF THE DAM SHALL BE AT LEAST & INCHES LONER THAN THE DUTER EDGES.
- 13. THE MAXIMUM SPACING BETWEEN THE DAMS SHALL BE SUCH THAT THE TOE OF THE UPSTREAM DAM IS AT THE SAME ELEVATION AS THE OVERFLOW ELEVATION OF THE DOWNSTREAM DAM.
- 14. STONE CHECK DAMS SHALL BE CONSTRUCTED OF A WELL-GRADED ANGULAR 2-MCH TO 3-MCH STONE. 3/4-MCH STONE ON THE UPDRADIENT FACE IS RECOMMENDED FOR BETTER FILTERING.
- 12. THE GRAVEL FILTER SHALL B 15. IF PROVOED BY DESIGN AND CONSTRUCTION PLANS, LEAVE THE DAM IN PLACE PERMANENTLY. 13 THE GRAVEL SHALL BE PLACE THE TOP OF THE BLOCK BARD
- 18. TEMPORARY STRUCTURES SHALL BE REMOVED ONCE THE SHALE OR DITCH HAS BEEN STABILED. BI TEMPORARY DITCHES AND SWALES, CHECK DAMS SHALL BE REMOVED AND THE DITCH FILLED IN WHEN IT IS NO LOADER MEETERS. BI PERMANENT STRUCTURES, CHECK DAMS SHALL BE REMOVED WHEN A PERMANENT LINING HAS BEEN ESTABLISHED, IS THE FEMALINEST LINING IS VECETARION. THEN THE CHECK DAM SHALL BE REMANED UNTE, THE CHECK DAMS MALENT IS HOLD IS VECETARION. THEN THE CHECK DAM SHALL BE REMANED UNTE, THE CHECK DAMS MALENT OF PROTECT THE DITCH OR SWALE. THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND TO PROTECT THE DITCH ON SWALE. THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALENED IS MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALENED IS MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALENDE IMMEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND IS MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND INCOMENDATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND INCOMENDATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND THE MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND THE MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND THE MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALEND THE MEDIATION AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND MALENT DAM HAST DATE THE THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND THE DISTORT AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND THE DISTORT AND THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND THE CHECK DAMED THE THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND THE CHECK DAMED THE THE AREA BENEATH THE CHECK DAM HAST DE SEDED AND THE CHECK DAMED THE THE THE THE THE THE AREA DAMENENT DAMENENT

TEMPORARY STONE CHECK DAMS

NOT TO SCALE

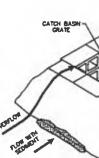
FINCE STETION

SEEVENT TRAPS AND SEEVENT BASING VELL NE DISPETETED FOR BEPTH D' SEEVENT, AT A KINDAM SEEVENT VOLLO NE RENOVED and trap restored to its driednal size when seevent neaders 1/2 of the original size



- - 1. WHEN THE SECTIONS OF 5 NOVES, FOLDED AND S

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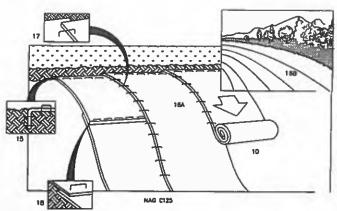
# MANIFLAMET RECRETEDATE 4. GEOR DANS SHALL BE RESPECTED AFTER EACH RAWFALL AND AT LEAST DALLY DURING PROLONGED RAWFALL AND RECESSARY REPARTS SHALL BE MADE MMEDIATELY.

- 2" - 3" STONE

- 5. INSPECTIONS SHALL VERIFY THAT THE CENTER OF THE DAM IS LOWER THAN THE EDGES.

CONCRETE BLOCK PLACED WITH HOLES PARALIEL TO THE GROUND, TO ALLOW WATER TO FLOW ENTO THE				
CATCH BASH				
WIRE SCHEDN TO BE PLACE BETWEEN THE STONE AND				
GAU BLOCK TO THEY CAN THE CAN BE AND A BLOCK TO THEY CAN BE AND A BLOCK TO THE CAN BE AND THE CAN BE AND THE CAN BE AND A BLOCK TO THE CAN BLOCK TO THE CAN BE AND A BLOCK TO	7			_⊢
(STONE RELIVEY FOR GRAPHIC CLARTY DULY, STORE HUST BE IN PLACE WHEN CONSTRUCTED)				
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La contraction of the second sec	S	ESCRIP		
MARTENANCE RECURRENENTS 1. RUET BARRERS SHALL BE INSPECTED BEFORE AND AFTER EACH RAIN EVOLT AND REPARED AS NEEDED.	>			
2. SCOMENT SHALL BE RELIVED AND THE STORM DRAIN SEDMENT BARRER RESTORED TO ITS ORGINAL DIMENSIONS WHEN THE SEDMENT HAS ACCOMMANDED TO 1/2 THE DESIGN DOPTH OF THE BARRER, RELIVED SEDMENT SHALL BE OPPOSITE IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERDOR.	ы И			
3. THE BARRERS SHALL BE REMOVED AND THE AREA STABILIZED WHEN THE CONTROLING DRAMAGE AREA HAS BEEN PROPERLY STABILIZED.			$\left  \right $	$\left  \right $
4. ALL CATCH BASHS AND STORN DRAM HELTS MUST BE CLEANED AT THE DND OF CONSTRUCTION AND AFTER THE SITE HAS BEEN FULLY STABILIZED.		DATE		
SPECIFICATIONS 1. THE MAXIMAN CONTRIBUTING DRAINAGE AREA TO THE TRAP SHALL BE LESS THAN ONE AGRE.		ğ	$\left  \cdot \right $	
THE BALET PROTECTION DEVICE SHALL BE CONSTRUCTED AL A MANNER THAT WILL FACULTATE CLEAN-OUT AND DEPOSAL OF TRANSPORT SEMILATION AND INTRACE INTERFERENCE WITH CONSTRUCTION ACTIVITIES.     ANY RESULTANT, PONDING OF STORING TER MUST NOT CAUSE EXCESSIVE MODIFIENCE OF DAMAGE TO	H		<u> </u>	
ADMODIT APEAD AT A STORTMENT OF MUST NOT CAUSE EXCLUSIVE MODIVEMENCE OF DAMAGE TO ADMODIT AREAS OF STRUCTURES.     B. THE BLOCKS SHALL BE PLACED LENGTHMESE IN A SINGLE ROW AROUND THE PERMETER OF THE INLET.	4		ALE HOW	
8. THE BLOCK ENDS SHALL ABUT ONE ANOTHER.		_		
10. THE HOLDH OF THE BARRED CAN BE VARED, DEPENDING ON DESCH NEEDS, BY STADIONG COMBINATIONS OF 4-INCH, B-INCH AND 12-INCH NEDE BLODIS, THE BARRED OF BLODIS AND GRAVEL FILTER SHALL BE A MINBAUM OF 12 MOVES INCH AND NO MOTE THAN 24 MOVES MODIL				MIRCON
11. A HADDWARE CAUTH OR WARE MEEN SHALL BE PLACED OVER THE OPENINGS OF THE CONCRETE BLOCKS AND EXTEND AT LEAST 12 INCHES AROUND THE OPENING TO PREVENT AGGREGATE FROM BEIND TRANSPORTED THROADER THE OPENINGS IN THE BLOCKS HAROWARE CLUTH OR COMPARABLE WEE MESH				TIMIT TO COOL IN STREET
WITH 1/2-MOH OPDNINGS SHALL BE USED. 12. THE GRAVEL FILTER SHALL BE CLEAN COARSE ADGREGATE.		75	] #	
12. THE GRAVEL SHALL BE PLACED AGAINST THE WIRE AND ALONG THE DUTSIDE EDGES OF THE BLOCKS TO THE DLOCK BANDLER.		2 3	E.U. B	
14. F THE STOKE FILTER BECOMES CLOOGED WITH SEDMENT SO THAT IT NO LONGER ADEQUATELY PERFORMS ITS FUNCTION, THE STOKE MUST BE PULLED AWAY FROM THE BLOCKS, CLEAKED AND REPLACED.		12	A Company	8
MANUEACTINETD STDMENT AARBEIS. 13. MAAUARATURED STDMENT AARBEIS. 13. MAAUARATURED STDMENT AARBEIS ARE NOW AVALABLE THAT EDLAD BE FUNCTIONALLY EDLIVALENT TO THE AARBEIS LISTED ABOVE. THESE MEASURES ARE ACCEPTABLE AS LONG AS THEY ARE INSTALLED, USED, AND MANTAMENT AS STOPPED BY THE VERDER OF MANTACATINET LON BEST,	-	1 È		021-51-60
AND BLANKTANED AS SPECIFIC BY THE EXCHANGE AND ALL AND ALL AS LONG AS THEY AND BREALLED, USED, DITEMBRY THE STORM DRAWN STRELL (S SUCH FROMENTS FALL TO FLORING THE REDARD) DITEMBRY THE STORM DRAWN STRELL (S SUCH FROMENTS FALL TO FLORING THE REDARD) TRAPPING FLORETRON, THEY SHALL BE REMOVED AND REPLACED WITH AN EDITEMPT ANTEMANTE BARRIER.			Veteron	8
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- CONSDERATIONS 1. DURING THE CROWING SEASON (APRIL 15 SEPTEMBER 15) USE MATS OR MILEO AND NETTING ON SLOPES 15% DR GREATER AND ANY DISTURBED SOIL WITHIN 100 FEET OF LAKES, STREAMS AND CONETLANDS.
- 2. DURING THE LATE FALL AND WINTER (SEPTEMBER 15 APRIL 15) LISE HEAVY GRADE MATS ON ALL AREAS MOTED ADOVE PLUS USE LIGHTER GRADE MATS OR MALCH AND NETTING ON SLOPES GREATER THAN BS. THERE MAY BE CASES WHERE MATS WILL BE NEEDED ON SLOPES FLATTER THAN BS, DEPENDING ON SITE CONDITIONS AND THE LEDNITH OF THE SLOPE.
- 3. INSTALL MATS AND STAPLE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- MANTENANCE RECEIRENENTS 4. ALL BLANKET AND MATS SHOULD BE INSPECTED WERLY DURING THE CONSTRUCTION MERIOD, AND AFTER ANY RAINFALL EVENT EXCEEDING & INCH IN A 24-HOUR MERIOD.
- 3. ANY FALLINE SHOULD BE REPARED BANEDIATELY, IF WASHOUT OF THE SLOPE, DESPLACEMENT OF THE MAY OR DAMAGE TO THE MAY OCCURE, THE AFFECTED SLOPE SHALL BE REPARED AND RESERVED, AND THE AFFECTED AREA OF MAY SHALL BE RE-RESTALLED ON REPLACED.

- SPECIFICATIONS STE PREPARATION: 8. GRADE AND SHAPE AREA OF INSTALLATION.
- 7. REMOVE ALL ROCKS, CLODS, TRASH, VEGETATIVE OR OTHER DESTRUCTIONS SO THAT THE INSTALLED BLANKETS WILL HAVE DIRECT CONTACT WITH THE SOL I PREPARE SEEDBED BY LOOSENING 2-3 INCHES OF TOPSOIL ABOVE FINAL GRADE.
- R. INCORPORATE AMENOMENTS, SUCH AS LINE AND FERTULZER, INTO SOIL ACCORDING TO SOIL TEST AND THE SEEDING PLAN.

# STERNO. 10. SEED AREA BEFORE BLANKET INSTALLATION FOR EROSION CONTROL AND RE-VEGETATION. SEEDING AFTER MAT INSTALLATION, IS OFTEN SPECIFED FOR TWE REMOVED EXPLICATION, WHEN SEEDING PROOF TO BLANKET INSTALLATION, ALL DIECK SLOTS AND OTHER AREAS DISTURBED DURING INSTALLATION MUST BE RESERVED.

II. WHERE SOIL FILLING IS SPECIFIED, SEED THE MATTING AND THE ENTIRE DISTURBED AREA AFTER INSTALLATION AND PROR TO FILLING THE WAT WITH SOIL

- IL BLANKETS SHALL BE INSTALLED AND ANCHORED PER THE MANUFACTUREN'S SPECIFICATIONS.
- 11 DISURE COMPLETE CONTACT OF THE PROTECTION MATTING WITH THE SOL.

INSTALLATION ON SLOPES. 14. BLANKETS SHALL BE INSTALLED ON SLOPES FOR THE MANUFACTUREN'S SPECIFICATIONS. IF THE MANUFACTUREN'S INSTRUCTIONS DIFFER FROM THOSE LISTED BOLOW, THE MANUFACTUREN'S INSTRUCTIONS SHOULD BE FOLLOWED.

15. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN 6" DEEP X 6" WOE TRENCH. BACKFRL AND COMPACT THE TRENCH AFTER STAPLING.

18. ROLL THE BLANKETS (A.) DOWN OR (B.) HORIZONTALLY ACROSS THE SLOPE

17. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 8" OVERLAP.

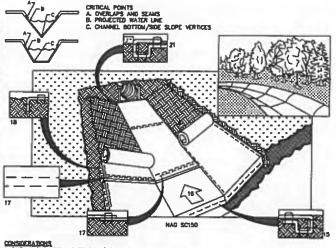
8. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS DID OVER DID (S-WHELE STULE) WITH APPROXIMATELY 6" CHERLAP. STAPLE THROUGH DYERLAPPED AREA, APPROXIMATELY 12" APART.

### TEMPORARY EROSION CONTROL BLANKET ON SLOPES NOT TO SCALE

### TEMPORARY EROSION CONTROL BLANKETS NHFG WILDLIFE FRIENDLY REQUIREMENTS

CONSDERATIONS 1. THE ELMINATION OF PLASTIC OR " BIODEGRADABLE PLASTIC" CROSION CONTROL NETTING IS RECLIRED AS THESE ARE KNOWN SOURCE OF ENTRAPMENT AND WORKALITY TO PROTECTED SNAKES AND TURTLESS 2. SEVERAL MULDIFE FRENDLY OPTIONS SUCH AS WOVEN ORGANIC MATERIAL (E.G., COCO MATTING) OR THE USE OF EROSION CONTROL BEREN DIKAY

3. ACCEPTABLE MATERIALS INCLUDE NORTH AMERICAN CREEN C1228N OR EAST COAST EROSON CONTRO BLANKET ECC-28 BOTH ARE BIODEGRADABLE WITH A COCONUT FIBER MATRIX AND JUTE NETTING.



CONSIDERATIONS 1. DURING THE GROWING SEASON (APRIL 15 - SEPTEMBER 15) USE MATS OR MULCH AND NETTING ON THE BASE OF GRASSED WATERWAYS.

- 2. DURING THE LATE FALL AND WINTER (SEPTEMBER 15 APRIL 15) USE HEAVY GRADE MATS ON SOE SLOPES OF GRASSED WATERWAYS.
- 3. INSTALL MATS AND STAPLE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- MANTENANCE RECURRENTLYS 4. All BLANGE AND MATS SHOULD BE INSPECTED WEDLY DURING THE CONSTRUCTION PERIOD, AND AFTER ANY RAMFALL EVENT EXCEEDING M INCH IN A 24-HOLR PERIOD.
- 3. ANY FAILINE SHOULD BE REPARED INMEDIATELY. IF WASHOUT OF THE SLOPE, DISPLACEMENT OF THE MAT, OR DAMAGE TO THE MAT DOCUME, THE AFFECTED SLOPE SHALL BE REPARED AND RESELVED, AND THE AFFECTED WARE OF MAT SHALL BE RE-INSTALLED OR REPLACED.

# Specifications Site preparation: 8. grade and Shape area of Installation.

- 7. REMOVE ALL ROCKS, CLODS, TRASH, VEGETATIVE OR OTHER DESTRUCTIONS SO THAT THE INSTALLED BLANKETS WILL HAVE DIRECT CONTACT WITH THE SCIL.
- 8. PREPARE SEEDEED BY LOOSENING 2-3 INCHES OF TOPSOIL ABOVE FINAL GRADE.
- 8. INCORPORATE AMENDMENTS, SUCH AS LINE AND FERTILIZER, INTO SOIL ACCORDING TO SOIL TEST AND THE SEEDING PLAN.

STEINED. IL SEED AREA BEFORE BLANKET INSTALLATION FOR ENOSION CONTROL AND RE-VEDETATION, SEEDING AFTER MAT INSTALLATION IS OFTEN SPECIFIED FOR THE REMPORTED. BLANKET INSTALLATION, ALL CHECK SLOTS AND OTHER ANEAS DISTURBED DURING INSTALLATION MALST BE RESELDED.

11. WHERE SOIL FILING IS SPECIFIED, SEED THE MATTING AND THE ENTIRE DISTURBED AREA AFTER INSTALLATION AND PRIOR TO FILING THE MAT WITH SOIL.

ESTALLING AND ANCHORNG BLANKETS. 12. BLANKETS SHALL BE INSTALLED AND ANCHORED PER THE MANUFACTURER'S SPECIFICATIONS. 13. DISURE COMPLETE CONTACT OF THE PROTECTION MATTING WITH THE SOL.

- RETAILATION IN CHANNELS. 14. RUANETS SHALL BE INSTALLED IN CHANNELS PER THE MANUFACTUREN'S SPECIFICATIONS. IF THE MANUFACTUREN'S HISTRUCTIONS DIFFER FROM THOSE LISTED BELOW, THE MANUFACTUREN'S INSTRUCTIONS SHOULD BE FOLLOWED.
- 15. BEGIN AT THE DUTLET OF THE CHANNEL BY ANCHORING THE BLANKET IN A 6" DEEP X 6" WOE TRENCH. BACKFUL AND COMPACT THE TRENCH AFTER STAPLING.
- 16. ROLL CENTER BLANKET IN DIRECTION OF THE INLET END OF THE CHANNEL
- 17. PLACE BLANKETS DND OVER DND (SHINGLE STYLE) WITH A 6° OVERLAP, USE A DOUBLE ROW OF STACGERED STAPLES 4° APART TO SECURE BLANKETS.

18. FULL LENGTH EDGE OF BLANKETS AT TOP OF SIDE SLOPES MUST BE ANCHORED IN 6" DEEP X 6" MOE TRENCH, BACKTRLL AND COMPACT THE TRENCH AFTER STAPLING,

IB. BLANKETS ON SDE SLOPES MUST BE OVERLAPPED 4" OVER THE CENTER BLANKET AND STAPLED. 20. IN HIGH FLOW CHANNEL APPLICATIONS. A STAPLE CHECK SLOT IS RECOMMENDED AT 30 TO 40 FOOT INTERVALS. USE A ROW OF STAPLES 4" APART OVER ENTRY WOTH OF THE CHANNEL PLACE A SECOND ROW 4" BELOW THE FREST ROW IN A STACCERED PARTNERN.

21. THE TERMINAL END OF THE BLANKETS MUST BE ANCHORED IN A 5° DEEP X 5" WIDE TRENCH, BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.

### TEMPORARY EROSION CONTROL BLANKET FOR CHANNELS

NOT TO SCALE

### TEMPORARY & PERMANENT MULCHING

- CONSIGNATIONS I. NITHIN GO RETS OF STREAMS, NETLANDS AND H LAKE NATURENEDS, TEMPORARY MULCH SHOULD BE APPLIED WITHIN 7 BAYS OF EXPOSING SOL, OR PRIOR TO ANY STORM ENDIT.
- 2. AREAS THAT HAVE BEEN TEMPORABLY OR PERMANENTLY SEEDED SHOULD BE MILEDED BANEDATELY FOLLOWING SEEDING.
- A AREAS THAT CANNOT BE SEEDED WITHIN THE GROWING SEASON SHOULD BE MULCHED FOR OVER-WINTER PROTECTION, THE AREA SHOULD BE SEEDED AT THE BEGINNING OF THE HEAT ORDINING SEASON.
- L MALCH ANCHORING SHOULD BE USED ON SLOPES WITH GRADIENTS GREATER THAN SS IN LATE FALL (PAST SEPTEMBER 15), AND EVER-INVER (SEPTEMBER 15 + MAY 15).
- S. PERMARENT MALCH CAN BE LISED IN CONJUNCTION WITH THEE, SHALB, WHE, AND GROUND COVER PLANTINGS.
- 7. ENGINE CONTROL MIX MULTIN USED FOR TEMPORARY STABILIZATION SHOULD BE LEFT IN PLACE, VERETATION ADDS STABILITY AND SHOULD BE PROMOTED.

- 10. IF THE MULCH HEEDS TO BE REMOVED, SPREAD IT OUT WITH THE LANDSCAPE.
- BY CONCATORS CONTINUE 1. APPLY MALCH PROP TO A STORM EVENT. THIS IS APPLICABLE IN EXTREMELY SENSITIVE AREAS SUCH AS WITHIN 100 PETT OF LARSE, PONDE, INVEST. STREAMS, AND WITH MOUNT IT WILL BE RECESSION TO CLOBELY MONTOR WEATHER PREDICTORS TO MAKE ADDREAM TO WARRING OF SENSITIVE ASSOCIATE WARRING AREAS SUCH AS WITHIN 100 IN APPLY MEDICTORS TO MAKE ADDREAM TO A STORM AND ADDREAM TO THE PERCON FROM CRICINAL RELEXANCE OF SENSITIVE AND ADDREAM TO A STORM EVENT. THIS IS APPLICABLE IN EXTERNAL Y SENSITIVE AREAS SUCH AS WITHIN 100 PETT OF LARSE, PONDE, INCREMENT, AND WE THE ADDREAM TO ADD
- PROJECTIONES IN THREE RELEASED WITHIN THE FOLLOWING SPECIFIED THREE FUTHORS FINDING CHIRINAL, BOLL, DUPORUME: WITHIN TOO FOLT OF INVERT AND STREAMS, WELLANDS, AND IN LAND, MON POIND WATCHENDES, THE THREE PERSON SPOLED BE NO OFFICIENT AND STREAMS, WELLANDS, AND IN LAND, MON POIND WATCHENDES, THE THREE PERSON SPOLED BE NO OFFICIENT AND STREAMS, WELLANDS, AND IN LAND, MON POIND WATCHENDES, THE THEATHER PERSONS, THE THE PERSON OF TAKE, RECTANT OF STREAMS, THE LINGTH OF THE WATCHE WATCHEND HISTORIAN, MEAS, THE THE PERSON OF TAKE, RECTANT OF SOLADWATCE, PHONEMENT TO SOLATION WITH STIE CONTINUE (AND AND ALL OF ENGINEENT, BEASEN OF TAKE RECTANCE OF AND AND AND THE STREAMS). THE POTEMPAL MPACT OF ENGINE IN ADJACENT ANEAR, DIMENT STATE OF LOCAL MESTRECTIONS MAY ALSO APPLY.
- 13. THE DEDICE OF MATURALS FOR MALCHING SHOULD BE BASED ON SITE CONDITIONS, SOLS, BLOPE, FLOW CONDITIONS, AND
- HAY OR STRAT MECHTS. 14. ORGANIS MICLIONS HAY AND STRAT SHOLED BE AR-DRED, FREE OF UNDESIGNAL SEEDS AND COARSE MATTERNS
- 15. APPLICATION BATE SHOLED BE 2 BALES (70-80 POLINES) PER 1000 SOUARE FEET OR 1.5 TO 2 TONS (90-100 BALES) PER ACRE TO CONCR 75 TO 80 X OF THE GROUND SURFACE.
- 15 HAY OF STRAW MULCH SHOLD BE AND/ORED TO PREVENT DISPLACEMENT BY WHO OF FLOWING WATER, USING ONE OF THE FOLLOWING METHODS:
- A DIAMMET REALMARK AND AN ANALYSIS IN PREVENT GOVERLING FOR MALE AND ALL AND A
- 17. WEDI HALDH IS APPLED TO PROVIDE PROTECTION OVER WHITER (PAST THE GROWING SEASON), IT SHOLD BE APPLED TO A DEPH OF FORM INFORMS (100-200 PROMOS OF MAY OR STRAW PER 1000 SEAME FUTL OR DOBLE STANDARD APPLCATION ARTS, SEEDING CAMPOT GEDERALLY HE DIFFETTED TO ORDER IP THOLED IP NOS DEPH OF MALEAN AND WIL AS SUBTIONED, IF VICETATION IS DESIRED, THE MALON WILL MEED TO BE REMOVED IN THE SPRINTING AND THE AREA SEEDID AND MALADED.

### NOTE CHIES OF BASIC 15. NOCE CHIES OF CREAR BASIC SHOULD BE AFFLED TO A THEOREES OF 2 TO 8 INCIDE.

- 18 WOOD DWPS OR GROUND BANK SHOULD BE APPLIED AT A RATE OF 10 TO 20 TOKS PER ACRE OR 480 TO 820 POUNDS
- DATEMPEN CONTROL MAR 20 ADDRESS CONTROL MAR CAN BE MANUFACTURED ON OR OFF THE PROJECT BIT. IT MART CONSIST PREMATE 20 ADDRESS CONTROL MAR THE POINT OF CONSTANTON, AND MAY PROJECT BATE, STRAN RESOLUTION BATE, OR ACCEPTARE LANGE ACTURED PROJECTS WOOD AND BATH OFFS, ORDERD CONSTRUCTION DOWNS OF REPROCESSED WOOD PREDUCTS WILL NOT BE ACCEPTABLE AS THE ORDANG COMPONENT OF THE MIX.

- 21. COMPOSITION OF THE UNDERGN CONTINGL MIX SHOLD BE AS FOLLOWS DIGGION CONTINGL MEL SHOLD CONTINGL MIX USE L-GRADED MIXTURE OF PARTICLE SERES AND MAY CONTAIN ACCHS LESS THAN * IN DAMATCE. BROBEN CONTINGL MEX MIST ME FREE OF FULLES. ANY DAAT CONTAINANTS, AND MATCHAL TEXES TO PLANT GROWTH, THE MEX COMPOSITION SHOLD MELT THE FRELOWING STANDARDS NK GRANE MATTER CONTON THOUGH BE UNTERIOR SHOLD MELT THE FRELOWING STANDARDS NK GRANE MATTER CONTON THOUGH BE UNTERIOR STAND SS. PARTICLE SEE BY WORD'T BYOLD BE UNTERIOR AS AND ELEMENTS. TO TOSS FASSING A ST-MONT BYOLD BE UNTERIOR AND CLOKELTD. THE MEX PASSING A T-MONT SOURCE, AND ELEMENTS. THE MEX PASSING A T-MONT SOURCE, AND ELEMENTS. THE MEX PASSING A CLOSE TO SEE THEORY AND ELEMENTS. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CALL DEAL SAME. THE MEX PASSING A CLOSE THE CLOSE AND ELEMENTS. THE MEX PASSING A CLOSE THE CLOSE AND ELEMENTS. THE MEX PASSING A CLOSE THE CLOSE AND ELEMENTS. THE MEX PASSING A CLOSE AND ELEMENTS. THE M
- BOLIDLE SALTS CONTENT ENOLED BE < 4.0 H

## 22. THE BANNET MUST BE PLACED ALONG A RELATIVELY LEVEL CONTOUR. IT MAY BE HECKSWARY TO GUT TALL GRASSES OF BOCOV NETTATION TO AVOD CREATING WIDE AND BROCES THAT WOULD EMAILS FREES TO WASH UNDER THE BANNET THEOLOW THE GRASS BLUEDE OF FLANT STUDY.

23. THE BARRER MUST BE A MINIMUM OF 12" HOH, AS MEASURED ON THE UPHILL SDE OF THE BARRER, AND A MIN TWO PETT HOP.

- C. CONTROL GROWTH

WINTER CONSTRUCTION NOTES

- ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXCHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCT.. 15TH, OR WHICH ARE DISTURBED AFTER OCT. 15TH, SHALL BE STABRIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR DN FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.
- 2. ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCT. 15TH, OR WHICH ARE DISTURBED AFTER OCT. 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.
- 3. AFTER OCTOBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.

- KIND OF SEED

KENTUCKY BLUEGRASS

CORDER NO. REAL PROPERTY IN

PERENNIAL RYEGRASS

BIRDEFOOT TREFOR

SEEDBED PREPARATION

2. ESTABLISHING A STAND

3. MULCH

A.

Α.

FIELD CONDITIONS

KIND OF SEED

ALSINE CLOVER

REDTOP

- CREEPING FEBCUE PERENNIAL RYEGRAS

- 8. WHERE PERMANENT MARCH IS USED IN COMMENCION WITH ORMANENTAL PLANTINGS, INSPECT PERCENCULLY THROUGHOUTHE YEAR TO DETERMINE IF MARCH IS MAINTAINING EXPERANCE OF THE SOL SUMFACE, REPAIR AS NEEDED.

R. FUTMANENT MALENED AREAS SHOULD BE BESPECTED AT LEAST AMMANLY, AND AFTER EACH LARGE RAMFAL (2.5 BODGE of note in a 34-moor formot, any fictared reparts bread be have balenatity, where (doesn's control not nas here note, fract and to the of the balenation to handahing the recommended brooses. When he hallen is decomposed, closed with sedwidt, drood or metytering, in wastie reparated on Reparate

### **GN-4: VEGETATION STABILIZATION NOTES**

ALL VEGETATION STABILIZATION SHALL BE IN ACCORDANCE WITH USDA NRCS "VEGETATING NEW HAMPSHIRE SAND and GRAVEL PITS", IN ADDITION TOO "BEST MANAGEMENT PRACTICES FOR ROLTINE ROADWAY MAINTENANCE ACTIVITIES IN NEW HAMPSHIRE", LATEST EDITIONS.

PARK SEED TYPE 15 SHALL NORMALLY BE USED ON LOAM AREAS. THIS SEED NUKTURE SHALL CONFORM TO TABLE 1 UNLESS AMENDED BY THE PROJECT ENGINEER TO SUIT ACTUAL FIELD CONDITIONS.

TA	BLE 1	
MINIMUM	MINIMUM	POUNDS/ACRE
PURITY (%)	GERMINATION (%)	
96	85	40
96	90	50
	. 85	25
95	80	6
	TO	TAL 120

SLOPE SEED TYPE 44 SHALL NORMALLY BE USED FOR ALL SLOPE WORK, and SHALL CONFORM TO TABLE 2 UNLESS AMENDED BY THE DESIGN ENGINEER TO BUIT ACTUAL

TABLE 2

PURITY (%)	GERMINATION (%)	POUNDE	ACRE
50	\$5	- 55	
98	90	30	
95	80	5	
87	60	6	
08		5	
	TO TO	TAL 50	

SEEDING SEASON:

A. ALLAREAS TO BE SEEDED SHALL BE A REASONABLY FIRM, BUT FRIABLE.

SURFACE and SEEPAGE WATER SHOULD BE DRAINED OR DIVERTED FROM THE SITE TO PREVENT DROWNING OR WINTER KILLING.

C. THE SEEDBED SHOULD BE LEFT IN A REASONABLY FIRM and SMOOTH CONDITION, FOLLOWING SEEDING OPERATIONS.

ALLAREAS TO BE SEEDED SHALL MEET THE SPECIFIED GRADES, AS SPECIFIED ON THE APPROVED PLAN.

ALL VEGETATION BHALL BE INSPECTED ANNUALLY FOR UNMEALTHY or DEAD AREAS. ANY and ALL SUCH AREAS ARE TO BE REPAIRED or REPLACED IN KIND.

A LIME AND FERTILIZER SHOULD BE APPLIED PRIOR TO OR AT THE TIME OF SEEDING AND INCORPORATED INTO THE SOL, KINDS AND AMOUNTS OF LIME AND FERTILIZER SHOULD BE BASED ON EVALUATION OF SOIL TESTS, WHEN A SOL TEST IS NOT AVAILABLE, THE FOLLOWING MINIMUM AMOUNTS SHOLE D BE APPLIED.

- ARRICULTURAL LIMESTONE: 2 TONS PER ACRE OR 1.09 LBS, PER SQ. FT. - NITROGEN (0): 50 LBS, PER ACRE OR 1.1 LBS, PER 1000 SQ. FT. - PHOSPHATE (PAD): 100 LBS, PER ACRE OR 2.2 LBS, PER 1000 SQ. FT. - POTABH (IGO): 100 LBS, PER ACRE OR 2.2 LBS, PER 1000 SQ. FT.

NOTE THE IS THE EDUNALENT OF 500 LBS. PER ACRE OF 10-20-30 FERTILIZER OR 1,000 LBS. PER

SEED SHOULD BE SPREAD UNIFORMLY BY THE METHOD MOST APPROPRIATE FOR THE SITE. METHODS INCLUDE BROADCASTING, DRILLING, AND HYDROSEEDING. WHERE BROADCASTING IS USED, COVER SEED WITH 0.25 INCH O SOR, OR LESS, BY CULTIPACIONG OR RAIGING.

HAY, STRAW, OR OTHER MULCH, WHEN NEEDED, SHOULD BE APPLIED MMEDIATELY AFTER

MULCH WILL BE HELD IN PLACE USING TECHNIQUES FROM THE "BEST MANAGEMENT PRACTICE FOR MULCHING", AS SHOWN IN, "STORMWATER MANAGEMENT AND SEDMENTATION CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE

4. MAINTENANCE TO ESTABLISH A STAND

PLANTED AREAS SHOULD BE PROTECTED FROM DAMAGE BY FIRE, GRAZING, TRAFFIC, AND DENSE WEED GROWTH.

8. FERTILIZATION WILL BE PERFORMED ANNUALLY IN ACCORDANCE WITH NOTE 2A.

IN WATERWAYS, CHANNELS, OR SWALES WHERE UNPORM PLOW CONDITIONS ARE ANTICEPATED, OCCASIONAL MOWING or TRIMMING WILL BE PERFORMED ANNUALLY TO

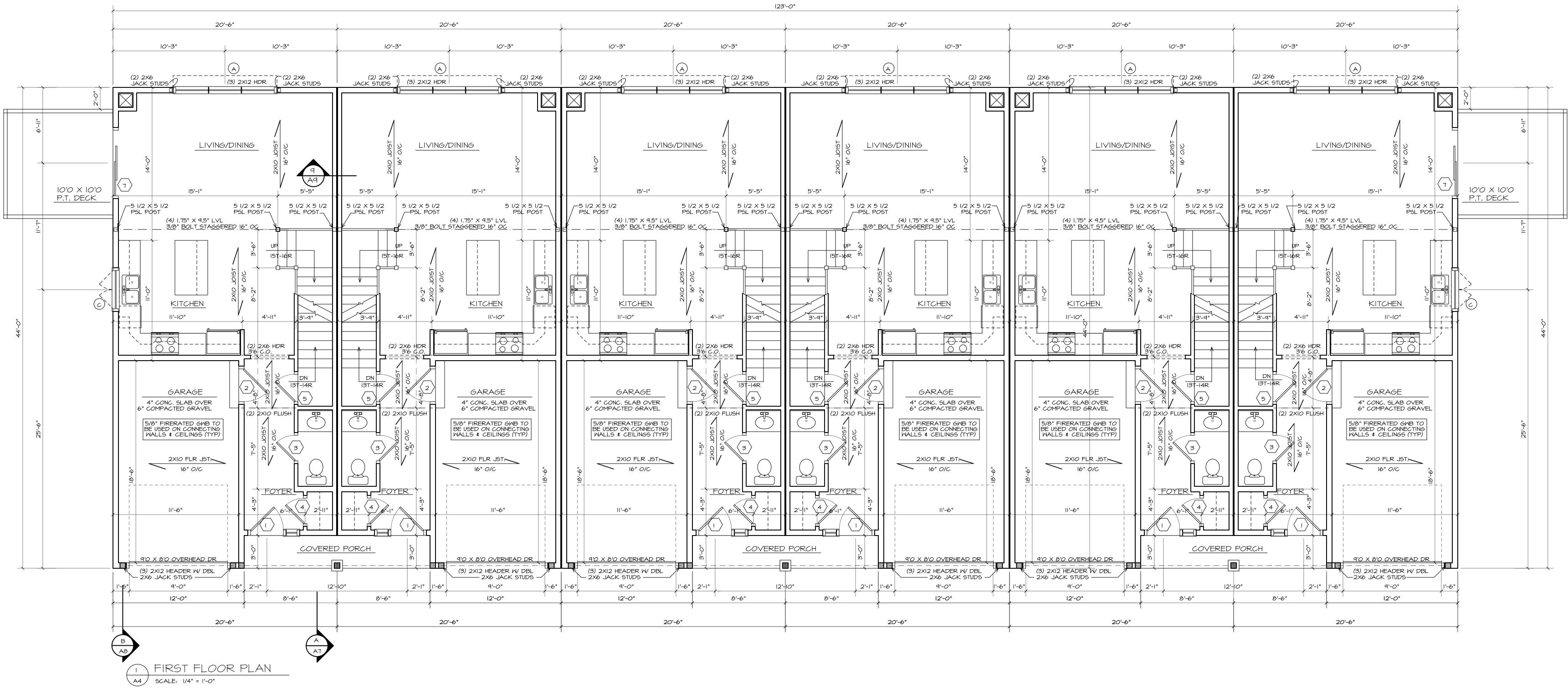
ALL VEGETATION BHOULD BE INSPECTED REGULARLY and AFTER EVERY MAJOR RAIN EVENT (> 5724 kr). DAMAGED AREAS SHOULD BE REPAIRED AND RE-VEGETATED MARENATELY.

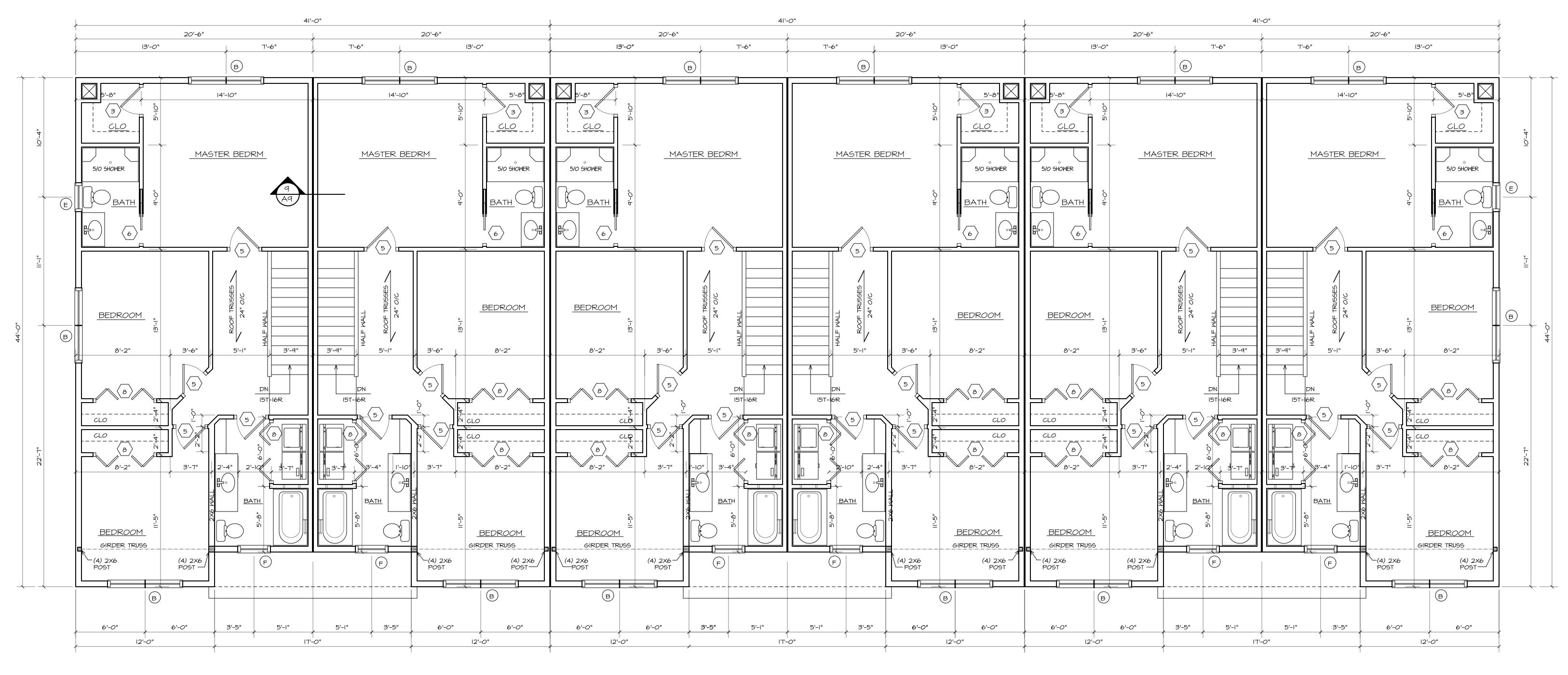






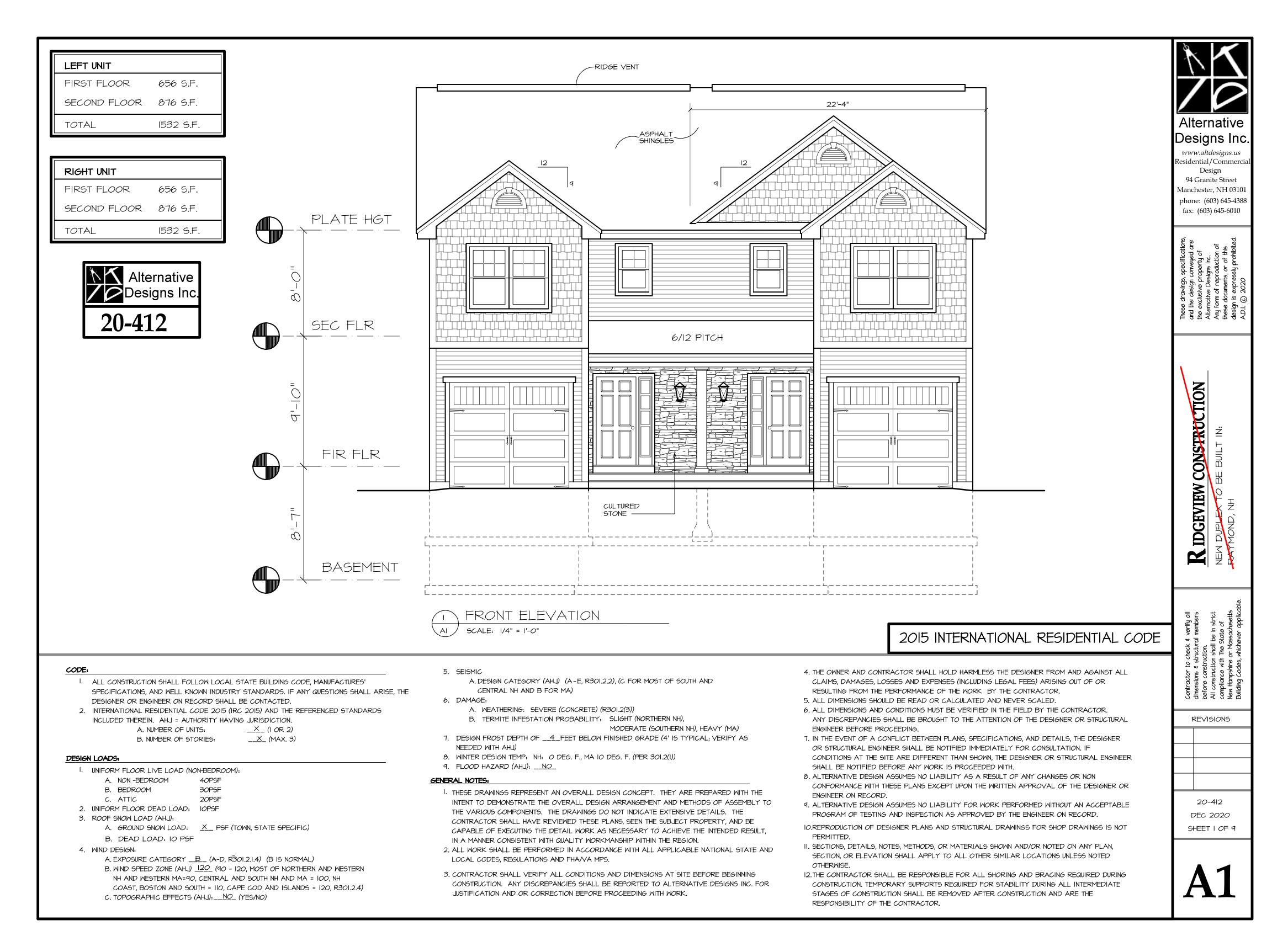
	RIGHT UNIT		LEFT UNIT	
Alternative Designs Inc.	FIRST FLOOR	656 S.F.	FIRST FLOOR	656 S.F.
	SECOND FLOOR	876 S.F.	SECOND FLOOR	876 S.F.
-256	TOTAL	1532 S.F.	TOTAL	1532 S.F.





SECOND FLOOR PLAN

A5 SCALE: 1/4" = 1'-0"



### FOUNDATIONS:

- I. FOUNDATIONS CONSIST OF CONTINUOUS FOOTINGS ASSUMED TO BEAR ON COMPACTED STRUCTURAL FILL PLACED ON UNDISTURBED NATURAL SOIL HAVING AN ASSUMED ALLOWABLE BEARING PRESSURE OF 2,500 PSF (TO BE VERIFIED BY BUILDER). IF THE SOIL AT BEARING DEPTH IS DISTURBED OR THE ACTUAL ALLOWABLE BEARING PRESSURE IS LESS THAN 2,500 PSF, THEN A QUALIFIED GEOTECHNICAL ENGINEER SHALL BE CONSULTED.
- 2. UNLESS OTHERWISE NOTED, FOOTINGS SHALL BE CENTERED UNDER SUPPORTED MEMBERS. 3. THE BOTTOM PERIMETER FOUNDATIONS SHALL BE DESIGN FROST DEPTH BELOW FINISHED
- GRADE. 4. THE BOTTOM 3 INCHES OF FOOTING EXCAVATIONS SHALL BE FINISHED BY HAND SHOVEL.
- 5. FINISH EXTERIOR GRADE SHALL BE AT LEAST &" BELOW TOP OF FOUNDATION WALL.
- 6. PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES OF WALLS TO THE GRADES INDICATED. 7. UNBRACED/UNBALANCED FOUNDATION WALLS: MAXIMUM UNBALANCED FILL: 24"
- WITHOUT DESIGN/ENGINEER INPUT/APPROVAL. (EXAMPLE GARAGE SLAB ON GRADE WHERE BACKFILL WILL BE MORE THAN 24" BELOW TOP OF SLAB) (SEE R404.1.3) (ENGINEER DESIGN REQUIRED WHEN >48")
- 8. WE RECOMMEND THAT WALKOUT AND KNEEWALL STYLE BASEMENTS BE REVIEWED. (IE. WHENEVER PERIMETER FOUNDATION WALLS ARE NOT FULL HEIGHT).
- PROVIDE FORMWORK FOR ALL FOOTINGS, WALLS, AND PIERS. EARTH FORMED FOUNDATIONS ARE NOT ALLOWED.
- IO.SUB -SOIL SHALL HAVE 3/4 " MAXIMUM AGGREGATE WITHIN 12" OF SLAB ON GRADE II. ANCHOR BOLTS: 1/2" X 9" (MIN. 7" EMBEDMENT) @ 4' OC AND BETWEEN 6-12" OF EACH
- END. (R403.1.6)
- 12.DAMP PROOFING ALWAYS REQUIRED BELOW GRADE WHEN INTERIOR SPACE IS CREATED (PER R406)
- 13.WATERPROOFING REQUIRED WHEN INTERIOR SPACE CREATED AND HIGH WATER TABLE OR OTHER CONDITIONS. (PER R406)

### CONCRETE:

- I. CONCRETE SHALL BE A MIX DESIGNED FOR ULTIMATE STRENGTH IN ACCORDANCE WITH ACI 211.1 TO ACHIEVE THE DESIRED COMPRESSIVE STRENGTH. STANDARD MINIMUM 3,000 PSI FOR FOOTINGS AND INTERIOR FLOOR, 3,500 PSI FOR WALLS AND GARAGE SLAB. (R402.2)
- 2. CONCRETE SHALL NOT BE CAST IN WATER OR ON FROZEN GROUND. CONCRETE SHALL NOT BE EXPOSED TO WATER (I.E. RAIN) DURING SETTING PERIOD.
- 3. CONCRETE FLOORS SHALL BE PLACED OVER MIN. 4" THICK POROUS LAYER (SUCH AS CRUSHED STONE) WITH DRAINAGE AND APPROVED VAPOR BARRIER. (R405.2.2)
- 4. TOP OF FOUNDATION WALLS AND SLABS SHALL BE SMOOTH AND LEVEL.
- 5. NO PIPE GREATER THAN 4" DIAMETER WITH APPROPRIATE SLEEVE SHALL PASS THROUGH CONCRETE WITHOUT ENGINEER APPROVAL. PIPE SLEEVES SHALL BE PROVIDED AND SPACED A MINIMUM THREE DIAMETERS APART.
- 6. KEYS SHALL BE 2"X4", WITH BEVELED SIDES, UNLESS OTHERWISE NOTED
- 7. CONSTRUCTION JOINTS SHALL BE FORMED WITH A KEY, AND REINFORCING SHALL BE LAPPED TO DEVELOP THE FULL TENSION CAPACITY OF THE (SMALLER) BAR.
- 8. EXPOSED CONCRETE SHALL BE RUBBED IMMEDIATELY AFTER REMOVAL OF FORMS AND SNAP TIES REMOVED TO FLUSH.
- 9. OPENINGS IN CONCRETE WALLS SHALL BE LOCATED, SIZED, AND REINFORCED (WITH THE EXCEPTION OF SMALL OPENINGS AND/OR SLEEVES OF A SIZE THAT WILL NOT DISPLACE OR INTERRUPT THE CONTINUITY OF THE REINFORCING) AS SHOWN ON RESPECTIVE DETAILS. ANY ALTERATIONS REQUIRE APPROVAL OF THE STRUCTURAL ENGINEER.
- IO.DO NOT BACKFILL FOUNDATION WALLS UNTIL THE CONCRETE HAS BEEN IN PLACE FOR SEVEN (7) DAYS AND ATTAINED 75% OF ITS DESIGN COMPRESSIVE STRENGTH, AND FLOOR DIAPHRAGMS ARE IN PLACE. (R404.1.7)

### REINFORCING STEEL:

- REINFORCING STEEL SHALL BE NEW STEEL BAR, FREE FROM LOOSE RUST AND SCALE, AND CONFORMING TO ASTM A615, GR 60.
- 2. STANDARD MINIMUM FOUNDATION FOOTING: 16" WIDE X 8" HIGH WITH NO REINFORCING.
- 3. STANDARD MINIMUM VERTICAL FOUNDATION WALL REINFORCING FOR COMMON CONDITIONS:

WALL HEIGHT	MAX. BACKFILL	WALL THICKNESS	HORIZONTAL REINFORCING (R404,1.2)	VERTICAL * REINFORCING
8'	7' 7'	8"	I #4 WITHIN I2" OF TOP AND I #4 AT MID -HEIGHT	#6 @ 36" OC *
۹'	8'	10"	I #4 WITHIN 12" OF TOP AND #4 BARS AT THIRD HEIGHTS	#6 @ 30" OC *
10'	9'	10"	I #4 WITHIN 12" OF TOP AND #4 BARS AT THIRD HEIGHTS	#6 @ 30" OC **

TABLE ABOVE ASSUMES BEST SOIL CLASS GW, GP, SW AND SP.

- * AT & AND 9' WALLS, VERTICAL REINFORCING NOT REQUIRED IF 75% DESIGN COMPRESSIVE STRENGTH AND 7 DAYS BEFORE BACKFILL IS ATTAINED
- ** AT IO' WALLS, ADDITIONAL ENGINEERING REQUIRED IF BACKFILLED BEFORE 75% DESIGN COMPRESSIVE STRENGTH IS ATTAINED
- 4. FLATWORK: WELDED WIRE FABRIC (WWF 6"X6" X NO. 10) RECOMMENDED IN ALL FLATWORK. IT SHALL CONFORM TO ASTM A185, LAP TWO SQUARES AT JOINTS AND TIE AT 3'-O" O.C. FURNISH WWF IN FLAT SHEETS.
- 5. PLAN CONTROL JOINTS AT 10-12' OC BOTH DIRECTIONS. WWF MUST NOT CROSS CONTROL JOINTS.
- 6. DECOUPLE FLATWORK FROM WALLS.
- 7. WELDED WIRE FABRIC SHALL BE SUPPORTED ON CONCRETE BRICKS SP. AT 24" OC EACH DIRECTION ON GRADE. WELDED WIRE FABRIC SHALL BE SUPPORTED ON ELEVATED DECK WITH CONTINUOUS BOLSTERS LOCATED OVER JOISTS AND BEAMS.
- 8. CLEAR CONCRETE COVER OVER BARS SHALL BE IN ACCORDANCE WITH ACI 318.
- 9. ACCESSORIES SHALL HAVE UPTURNED LEGS AND BE PLASTIG DIPPED AFTER FABRICATION. ACCESSORIES FOR REINFORCING SHALL BE IN ACCORDANCE WITH THE MOST CURRENT ACI EDITION.
- IO.LAP REINFORCING TO DEVELOP THE FULL TENSION CAPACITY OF THE (SMALLER) BAR.

- II. NO BARS SHALL BE CUT OR OMITTED IN THE FIELD BECAUSE OF SLEEVES, DUCT OPENINGS, OR RECESSES. BARS MAY BE MOVED ASIDE WITHOUT CHANGE IN LEVEL WITH THE PRIOR APPROVAL OF STRUCTURAL ENGINEER.
- 12. ANCHOR BOLT MATERIAL SHALL CONFORM TO ASTM A36, A307, OR BETTER, AND MEET IRC 2015 CODE.

### WOOD:

- I. WORK SHALL BE IN ACCORDANCE WITH THE AMERICAN WOOD COUNCIL, ANSI/AF&PA, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 2012 (NDS)" INCLUDING "DESIGN VALUES FOR WOOD CONSTRUCTION", NATIONAL FOREST PROTECTION ASSOCIATION.
- 2. ALL LUMBER SHALL BE NEW AND STRAIGHT AS DESCRIBED IN "STANDARD GRADING RULES FOR NORTHEASTERN LUMBER" BY NORTHEASTERN LUMBER MANUFACTURERS ASSOCIATION. 3. NEW WOOD FOR STRUCTURAL USE SHALL HAVE A MOISTURE CONTENT AS SPECIFIED IN THE
- "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."
- 4. FRAMING FOR WALLS AND JOISTS SHALL BE SPRUCEPINE -FIR NO. I/NO. 2 OR BETTER. UNLESS NOTED OTHERWISE, DIMENSIONAL LUMBER REPRESENTS NOMINAL SIZES.
- 5. SHEATHING PANELS SHALL BE MARKED WITH THE AMERICAN PLYWOOD ASSOCIATION (APA) TRADEMARK AND SHALL MEET THE LATEST US PRODUCT STANDARD PS I OR APA PRP - 108 PERFORMANCE STANDARDS.
- 6. ALL WALL SHEATHING PANELS SHALL BE NOMINAL 1/2" THICK APA RATED , UNLESS OTHERWISE NOTED, FASTEN WITH 8D COMMON NAIL SPACED AT 6" OC AT PANEL PERIMETER SUPPORTED EDGES AND 12" OC AT INTERIOR INTERMEDIATE SUPPORTS (FIELD). 1 -3/8" MIN. FASTENER PENETRATION. LAY WALL WITH REQUIREMENTS OF IRC 604.
- 1. ALL ROOF SHEATHING PANELS SHALL BE 5/5" THICK UNLESS NOTED OTHERWISE, C -D EXTERIOR GRADE, APA RATED EXPOSURE I MEETING DOC PSI OR PS2, FASTEN WITH 8D COMMON NAILS SPACED AT 6" OC AT PANEL PERIMETER SUPPORTED EDGES AND 6" OC AT INTERIOR INTERMEDIATE SUPPORTS (FIELD). 1 3/8" MIN. FASTENER PENETRATION. LAY ROOF SHEATHING WITH LONG DIMENSION PERPENDICULAR TO SUPPORT MEMBERS.
- 8. WOOD TO STEEL AND WOOD TO WOOD BOLTED CONNECTORS SHALL BE MADE WITH ASTM A307 BOLTS WITH FLAT WASHERS. BOLT HOLES IN WOOD SHALL BE 1/32" LARGER THAN THE BOLT. WOOD NAILERS SHALL BE FASTENED WITH 3/8" DIA. BOLTS STAGGERED AT 20" OC UNLESS OTHERWISE NOTED.
- 9. FASTENING SCHEDULE (SEE ALSO R602.3(1):
  - I. PLATE TO STUD, DIRECT: 2 16D
  - II. STUD TO PLATE, TOENAIL: 4 8D
- IO. WOOD IN CONTACT WITH SOIL, MOISTURE, WEATHER, CONCRETE, OR MASONRY SHALL BE PRESSURE TREATED SOUTHERN PINE NO. 2, OR BETTER AND APPROVED FOR THE APPLICATION.
- II. BRACING: THE PERMANENT LATERAL BRACING SYSTEM INCLUDES PLYWOOD, WALL AND ROOF SHEATHING WITH FASTENING AND LAYOUT AS DEFINED BY: SECTION 602. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING AS REQUIRED TO LATERALLY SUPPORT THE STRUCTURE DURING CONSTRUCTION.
- 12. ENGINEERED LUMBER (LVL, ETC.) SHALL MATCH MANUFACTURER AND SERIES LISTED OR APPROVED EQUIVALENT. PROVIDE LATERAL SUPPORT AT ALL BEARING POINTS AND ALONG COMPRESSION EDGES AT INTERVALS OF 24" OC, OR CLOSER.
- 13. MINIMUM SECTION WIDTH = 1-3/4", 3-1/2", 5-1/4" AND 7" MEMBERS MAY BE COMBINATIONS OF I-3/4"MEMBERS. FOLLOW MANUFACTURER'S GUIDELINES FOR MULTIPLE MEMBER CONNECTIONS AND FOR SIDE LOADED BEAMS.
- 14. WOOD CONSTRUCTION CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE CO., INC., OR APPROVED EQUAL, AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, INCLUDING FASTENERS.
- 15. ALL FLUSH FRAMING TO HAVE APPROPRIATELY SIZED METAL JOIST HANGERS.
- 16. LATERAL RESTRAINT REQUIRED AT ENDS OF FLOOR FRAMING SOLID BLOCK OF SAME MATERIAL (R502.7)
- 17. BRIDGING OR CONT. IX3 BRACE NAILED TO UNDERSIDE OF FLOOR FRAMING REQUIRED AT 8' INTERVALS (R502.7.1)
- 18. HEADERS: DEFAULT (MAX. 48" SPAN UNLESS POINT LOAD FROM ABOVE OR LATERAL BRACING REQUIREMENTS. SEE R.502.5) :
  - I. INTERIOR: (2) 2X8
- 2. EXTERIOR: (2) 2XIO (WITH 2-1/2" RIGID FOAM INSULATION). HEADERS: DEFAULT (MAX. 72" SPAN) 3-2X12 FOR 2 FLOORS CEILING AND ROOF
- 19. WIND BRACING: PROVIDE DIAGONAL WIND BRACING AT ALL OUTSIDE CORNERS. AT CORNERS WITH LESS THAN 48" OF PANEL WALL, USE ALTERNATE BRACING PANELS IN ACCORDANCE WITH R602.10.6.2. (GENERAL REFERENCE: R602)
- 20. RAFTER/CEILIMG JOIST HEEL CONNECTIONS (VAULTED CLGS @ 1/3) TABLE R802.5.1(9)

### PRE -ENGINEERED WOOD TRUSSES:

- I. ALL PRE-ENGINEERED WOOD TRUSSES SHALL CONFORM TO ANSI/TPII -2002 "NATIONAL DESIGN STANDARDS FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION."
- 2. THE MANUFACTURER OF THE PRE -ENGINEERED TRUSSES SHALL BE A TRUSS PLATE INSTITUTE (TPI) CERTIFIED PLANT, PROOF OF CERTIFICATION SHALL BE SUBMITTED TO THE DESIGNER/ENGINEER PRIOR TO FABRICATION OF THE WOOD TRUSSES.
- 3. THE CONTRACTOR SHALL ENSURE PROPER HANDLING, BRACING, AND LATERAL RESTRAINT IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ALL TEMPORARY AND PERMANENT TRUSS BRACING (INDIVIDUAL AND OVERALL) SHALL BE DESIGNED BY THE TRUSS MANUFACTURER AND INSTALLED BY THE CONTRACTOR. ALL PERMANENT TRUSS BRACING/LATERAL RESTRAINT REQUIREMENTS AND LOCATIONS SHALL BE DETAILED AND SUBMITTED PRIOR TO CONSTRUCTION TO THE ENGINEER OF RECORD BY THE TRUSS MANUFACTURER. ALTERNATIVELY, THE TRUSS DESIGNER MAY DESIGN ALL TRUSSES SUCH THAT NO PERMANENT LATERAL RESTRAINT IS REQUIRED.

4. ALL ROOF TRUSSES SHALL BE DESIGN FOR THE FOLLOWING UNIFORM LOADS WITH 51/2"OR 31/2"MAX BEARING. COORDINATE TRUSS BEARING WITH BEARING WALL FRAMING WIDTH: A. SNOW LIVE LOAD: GROUND SNOW LOAD X 0.7=_XX_ PSF

- B. BOTTOM CHORD LIVE LOAD (ATTIC): 20 PSF
- C. TOP CHORD DEAD LOAD: 10 PSF
- D. BOTTOM CHORD DEAD LOAD: IO PSF
- 5. TRUSS SHALL BE DESIGNED FOR AN UNBALANCED UNIFORM SNOW LOADING AS WELL AS ANY DRIFTED VALLEY SNOW LOADING CONDITIONS, AND WIND LOADING AS SPECIFIED IN THE PRO, IFCT BUILDING CODE.
- 6. PRE-ENGINEERED ROOF TRUSSES TO BE APPROVED BY THE STRUCTURAL ENGINEER. TRUSS SHOP DRAWINGS SHALL BE DESIGNED, STAMPED, AND SUBMITTED BY A LICENSED PROFESSIONAL ENGINEER QUALIFIED TO PERFORM THE WORK IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMITTAL SHALL INCLUDE ALL LOADING COMBINATIONS, A FULL REPORT FOR EACH TRUSS, AND TEMPORARY AND PERMANENT LATERAL TRUSS RESTRAINT LAYOUT AND DETAILS.
- 7. THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL VENTS, STACKS, RISERS, DRAINS, ETC. BEFORE TRUSSES ARE FIXED IN PLACE.
- 8. ALL TRUSSES SHALL HAVE HURRICANE CLIPS INSTALLED AT EACH END OF EACH TRUSS IN ORDER TO PREVENT LIFT.
- 9. ALL TRUSS TO TRUSS CONNECTION DESIGNS ARE RESPONSIBILITY OF THE TRUSS MANUFACTURER. IO. ALL TEMPORARY AND PERMANENT TRUSS BRACING (INDIVIDUAL AND OVERALL) IS THE
- RESPONSIBILITY OF THE TRUSS DESIGNER. BRACING AND LATERAL TRUSS RESTRAINT (INCLUDING DETAILS) SHALL BE SHOWN ON TRUSS DESIGN DRAWINGS AND TRUSS ERECTION DRAWINGS.

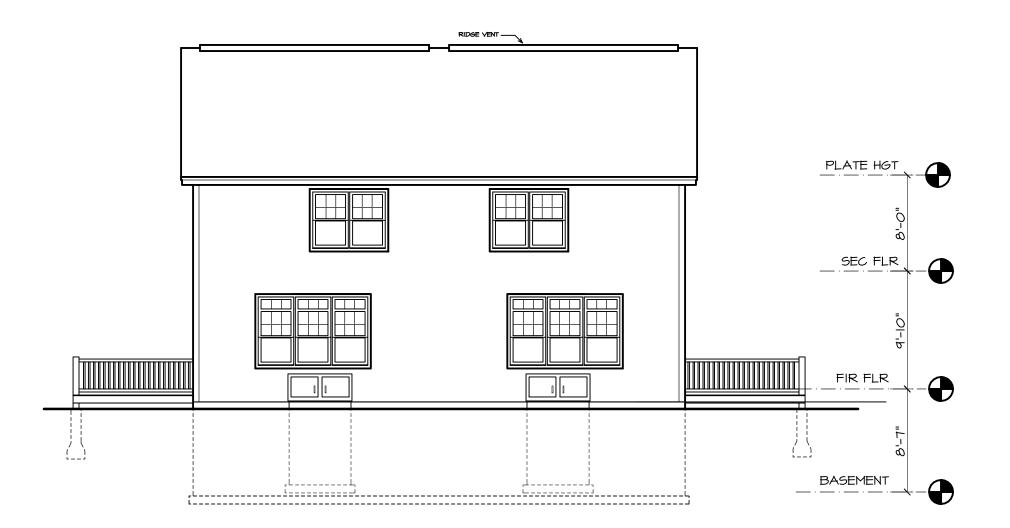
### MASONRY:

- I. CONCRETE MASONRY UNITS (CMU) SHALL BE NOMINAL THICKNESS UNLESS NOTED OTHERWISE. 2. MASONRY CONSTRUCTION SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY
- STRUCTURES (ACI 530/ASCE 5/TMS 402) 3. SPECIFIED MASONRY COMPRESSIVE STRENGTH, F'M = 1500PSI.
- 4. HOLLOW LOAD BEARING CMU SHALL HAVE THE FOLLOWING PROPERTIES: ASTM C90, TYPE I, GRADE N-I (NORMAL WEIGHT) WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI ACCORDING TO ASTM CI40, OVEN DRY WEIGHT OVER 125PCF AND MAXIMUM MOISTURE ABSORPTION OF I3PCF.
- 5. MORTAR SHALL BE ASTM C2TO, TYPE S WITH 28 DAY COMPRESSIVE STRENGTH OF 2000PSI. MIX MORTAR MATERIALS TO PRODUCE MORTAR CUBES HAVING A 2000PSI COMPRESSIVE STRENGTH WHEN TESTED IN ACCORDANCE WITH COMPRESSIVE STRENGTH TEST ASTM CT80.
- 6. GROUT SHALL BE ASTM C476, FINE GROUT WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000PSI.
- 7. VERTICAL AND HORIZONTAL DEFORMED REINFORCEMENT SHALL BE ASTM A615 GR 60 AND HORIZONTAL JOINT REINFORCEMENT SHALL BE ASTM A82, GALVANIZED ACCORDING TO ASTM A641 CLASS I AS SPECIFIED.
- 8. PRISM TESTS ACCORDING TO ASTM E446 ARE REQUIRED PRIOR TO WORK.
- 9. GROUT CMU SOLID AT EXPANSION ANCHOR LOCATIONS.
- IO. CORES AND BOND BEAMS WITH REINFORCING SHALL BE FILLED SOLIDLY WITH GROUT. FILLING SUCH CORES AND BOND BEAMS WITH MORTAR IS STRICTLY PROHIBITED. IN ADDITION, CARE SHALL BE EXERCISED IN KEEPING CORES FREE FROM MORTAR DROPPINGS.
- II. MINIMUM REINFORCING REQUIREMENTS FOR REINFORCED CMU WALLS SHALL CONFORM TO THE SCHEDULE SHOWN ON THE CONTRACT DRAWINGS AND THE APPLICABLE BUILDING CODE REQUIREMENTS.
- 12. GROUT SHALL BE PLACED USING LOW OR HIGH LIFT GROUTING PROCEDURES CONFORMING TO ACI/ASCE. TERMINATE GROUT POURS I-I/2" BELOW TOP COURSE OF PLACEMENT. REINFORCING SHALL BE SPLICED A MINIMUM OF 40 BAR DIAMETERS.
- 13. VERTICAL REINFORCING SHALL BE SECURELY HELD IN PROPER ALIGNMENT AND POSITION DURING GROUTING OPERATIONS BY USING "REBAR POSITIONERS," AS MANUFACTURED BY WIRE BOND OR APPROVED EQUIVALENT. THE PRODUCT, IN ADDITION, SHALL ALLOW FOR GUIDING THE SPLICED REINFORCING DROPPED FROM THE TOP OF THE LIFT.
- 14. MASONRY SHALL BE BRACED DURING CONSTRUCTION. BRACE SPACING SHALL NOT EXCEED TEN TIMES THE WALL THICKNESS BUT NOT LESS THAN THE PROCEDURES LISTED UNDER NOMA-TEK 72
- 15. PROVIDE FULL HEIGHT VERTICAL REINFORCEMENT AT EACH SIDE OF CONTROL JOINTS, WINDOWS, DOORS, AND WALL OPENINGS, AT ALL ENDS OF WALLS AND CORNERS. REINFORCING SHALL BE GROUTED SOLID AND MATCH THE DIAMETER OF THE TYPICAL WALL REINFORCING.

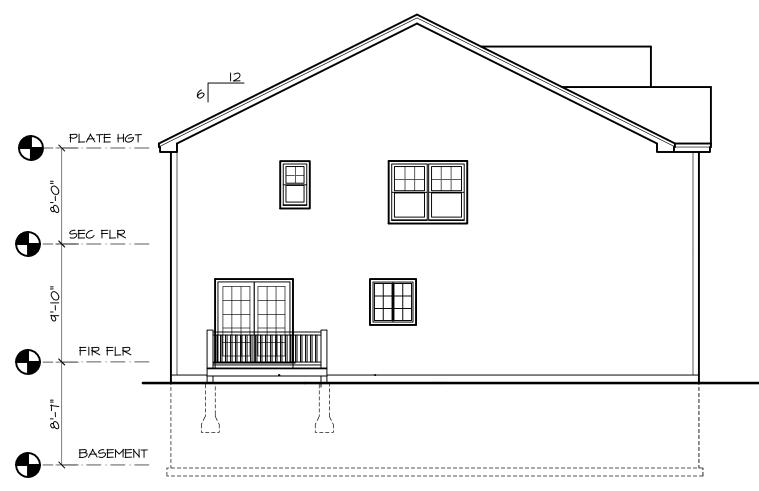
### FIRE RESISTANT CONSTRUCTION

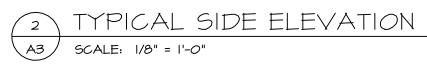
- I. FOLLOW SECTION 302. A FEW COMMON CRITICAL LOCATIONS FOLLOW:
  - A. GARAGE/RESIDENCE OR GARAGE/ATTIC SEPARATION 5/8" TYPE X GYPSM DRYWALL AT GARAGE SIDE WHEN ADJACENT TO LIVING SPACE. 5/8" TYPE X DRYWALL REQUIRED AT CEILING WHEN LIVING SPACE ABOVE. (TABLE R302.6)
  - B. ENCLOSED ACCESSIBLE SPACE UNDER STAIRS REQUIRES MIN. 1/2" GYPSUM (R302.7) C. FIREBLOCKING IS REQUIRED TO ISOLATE EACH FLOOR LEVEL. 2X BLOCKING AND "
  - GYPSUM AND FIBERGLASS/MINERAL WOOL IF SECURE ARE ALL ACCEPTABLE (R302.11.1)
- 2. DUPLEX/2 FAMILY STANDARD SEPARATION IS 5/8" TYPE X BOTH SIDES. (R302.3)

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Contractor to check ≰ verify all dimensions ≰ structural members before construction. All construction shall be in strict compliance with The State of New Hampshire or Massachusetts Building Codes, whichever applicable.
Contractor to check & verify all dimensions & structural members before construction. All construction shall be in strict compliance with The State of New Hampshire or Massachusetts Building Codes, whichever applic
20-412 DEC 2020 SHEET 2 OF 9
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	WINDOW SCHEDULE				
MARK	QTY	MODEL NUMBER	RSO	NOTES	
A		(3)244DH3049		MULLED DH W/ TRANSOM	
В		(2)244DH3O49		MULLION	
С		CN235		DBL. CSMT	
D		(3)244DH3049		MULLED DBL HUNG	
E		244DH2036		DBL. HUNG	
F		244DH2436		DBL. HUNG	

NOTES:

I. RSO TO BE DETERMINED BY WINDOW MANUFACTURER.

2. BEDROOM WINDOWS TO MEET EGRESS

3. IN ACCORDANCE WITH I.R.C. (2015)-R312.2, WHERE THE OPENING OF AN OPERABLE WINDOW IS MORE THAN 72" ABOVE THE EXT. FINISHED GRADE OR EXT. DECK BELOW, THE LOWEST PART OF THE CLEAR OPENING IS TO BE A MIN. OF 24" ABV. THE FIN. FLR.

4. WINDOWS ARE BASED ON ANDERSEN 200 SERIES TILT-WASH MODEL NUMBERS

		Ľ	OOR SCHEDULE	
MARK	QTY	SIZE	R50	NOTES
I		3'0 X 6'8		EXT. DOOR W/ SINGLE SDLT
2		3'0 X 6'8		FIRE RATED
3		2'4 X 6'8		INTERIOR
4		3"O X 6'8		BIFOLD
5		2'6 X 6'8		INTERIOR
6		2'4 × 6'8		POCKET DR
Г		6'0 X 6'8		SLIDING PATIO DR
8		5'0 X 6'8		BIFOLD
9		2'0 X 6'6		INTERIOR
10		3'0 X 6'8		9-LITE
		2'8 X 6'8		STL INSUL.

RSO TO BE DETERMINED BY DOOR MANUFACTURER.

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Manchester, NH 03101

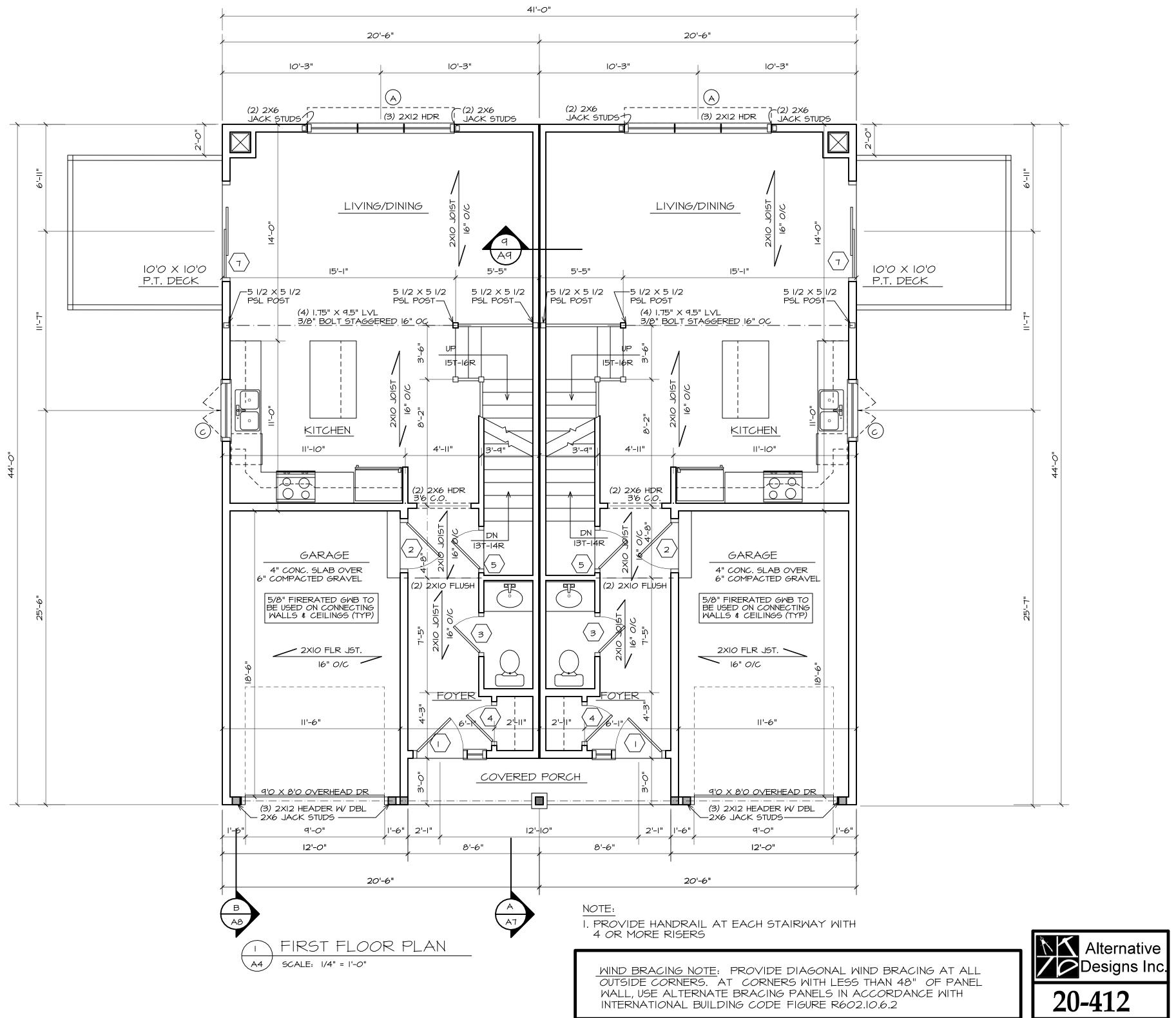
phone: (603) 645-4388

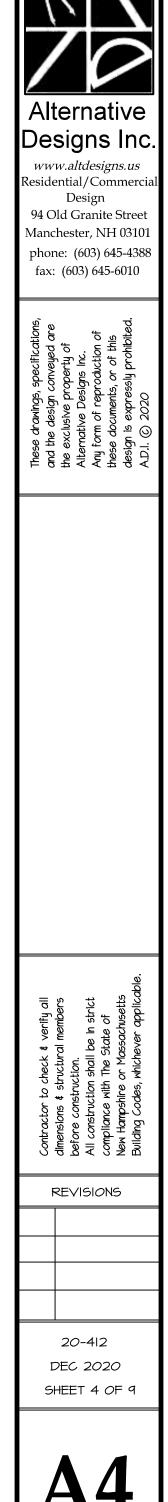
fax: (603) 645-6010

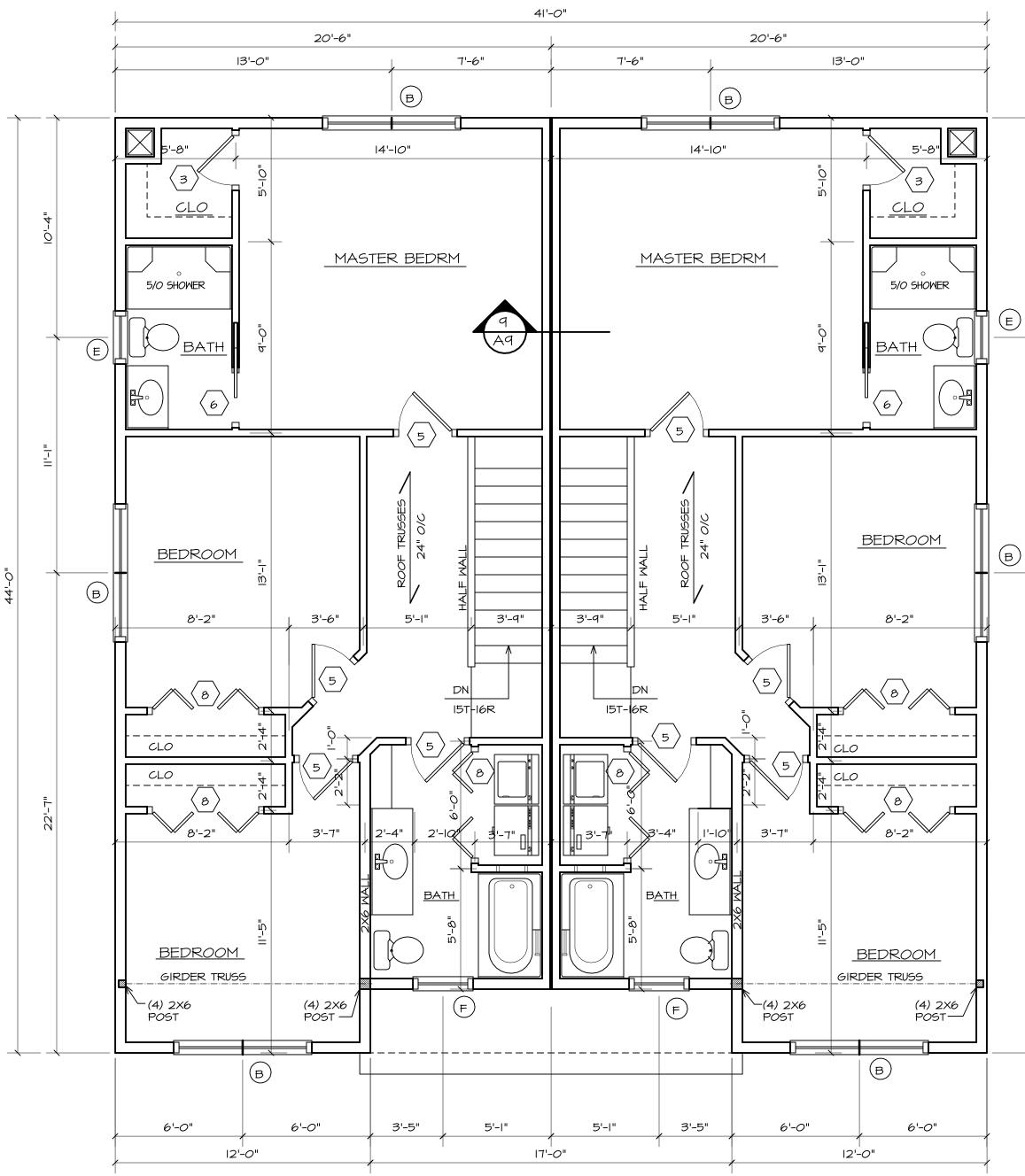
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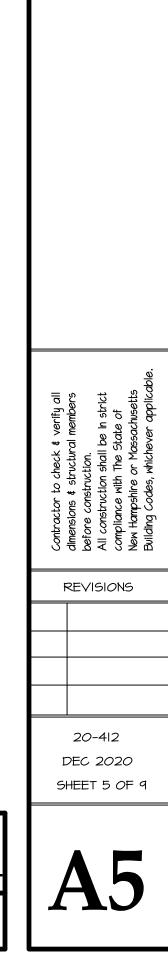






SECOND FLOOR PLAN SCALE: 1/4" = 1'-0" _A5_

NOTE: USE TRIPLE 2X6 TOP PLATE ON SECOND FLOOR BEARING WALLS FOR ROOF TRUSSES



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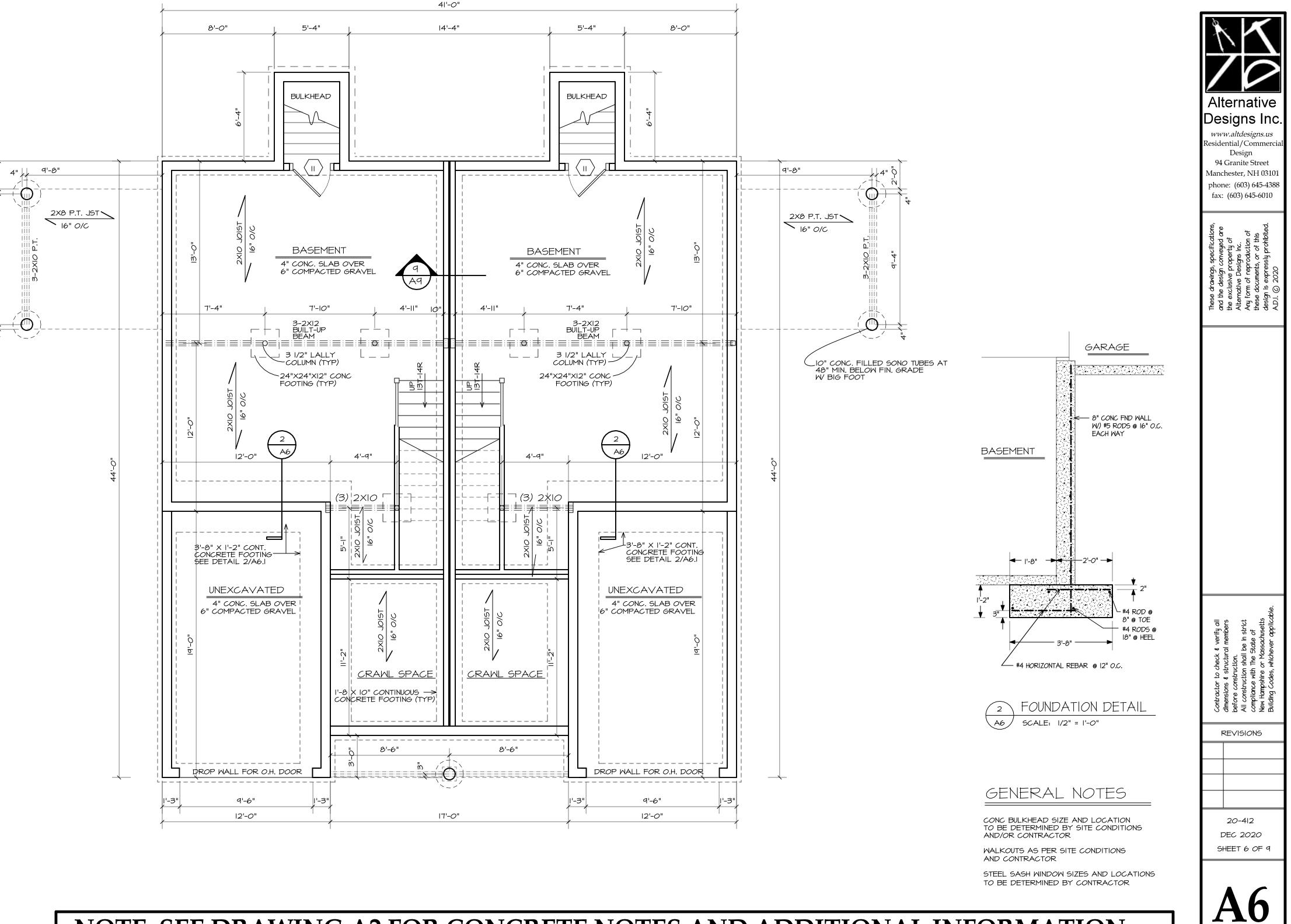
22'-T"

I. PROVIDE HANDRAIL AT EACH STAIRWAY WITH 4 OR MORE RISERS

WIND BRACING NOTE: PROVIDE DIAGONAL WIND BRACING AT ALL OUTSIDE CORNERS. AT CORNERS WITH LESS THAN 48" OF PANEL WALL, USE ALTERNATE BRACING PANELS IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE FIGURE R602.10.6.2

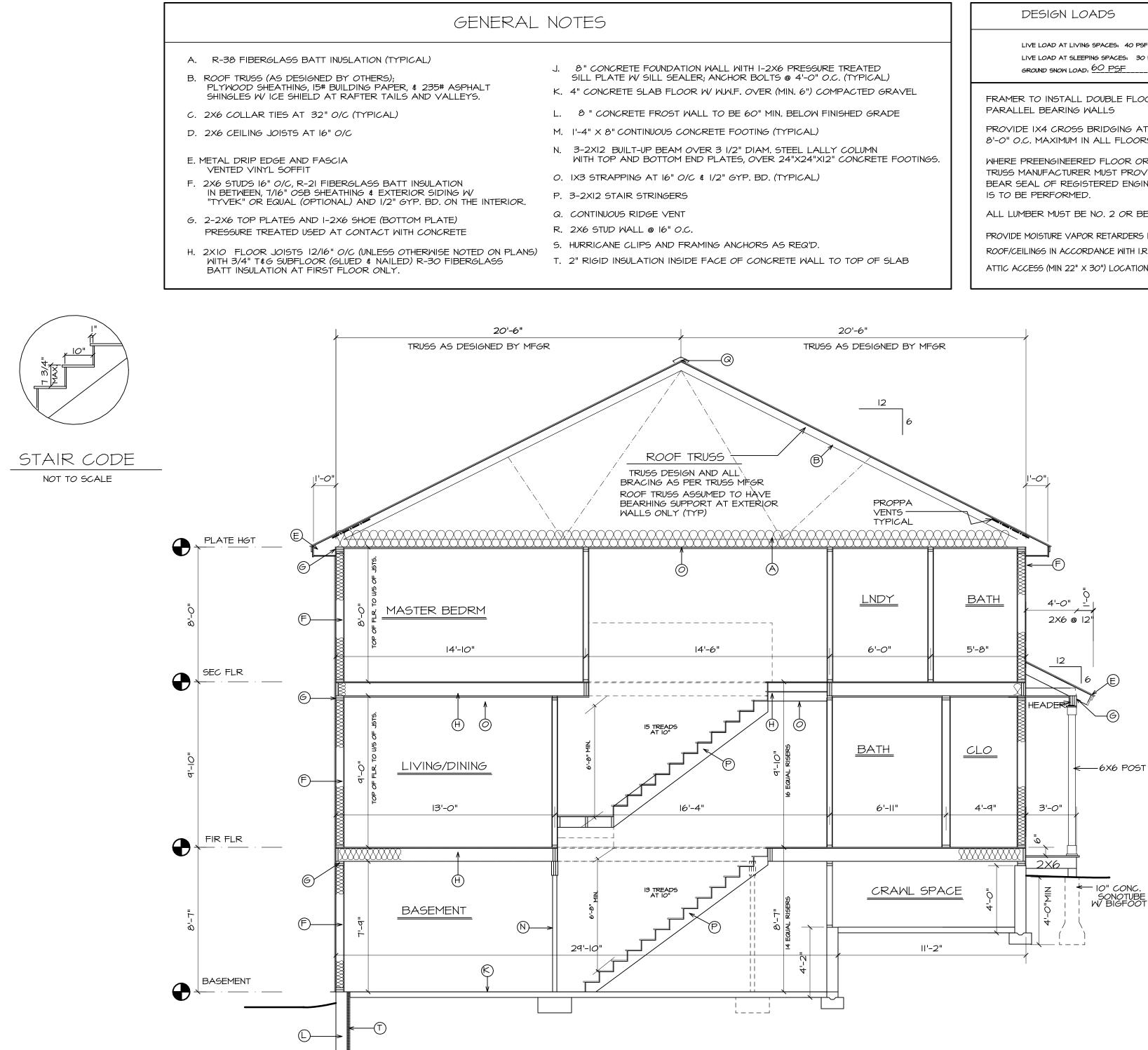


# NOTE: SEE DRAWING A2 FOR CONCRETE NOTES AND ADDITIONAL INFORMATION



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Ab



M-

LIVE LOAD AT LIVING SPACES: 40 PSF LIVE LOAD AT SLEEPING SPACES: 30 PSF

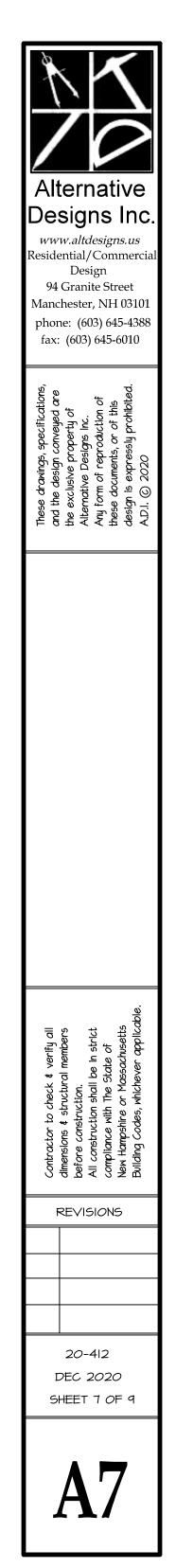
FRAMER TO INSTALL DOUBLE FLOOR JOISTS UNDER ALL

PROVIDE IX4 CROSS BRIDGING AT MID POINT OF SPAN OR 8'-0" O.C. MAXIMUM IN ALL FLOORS.

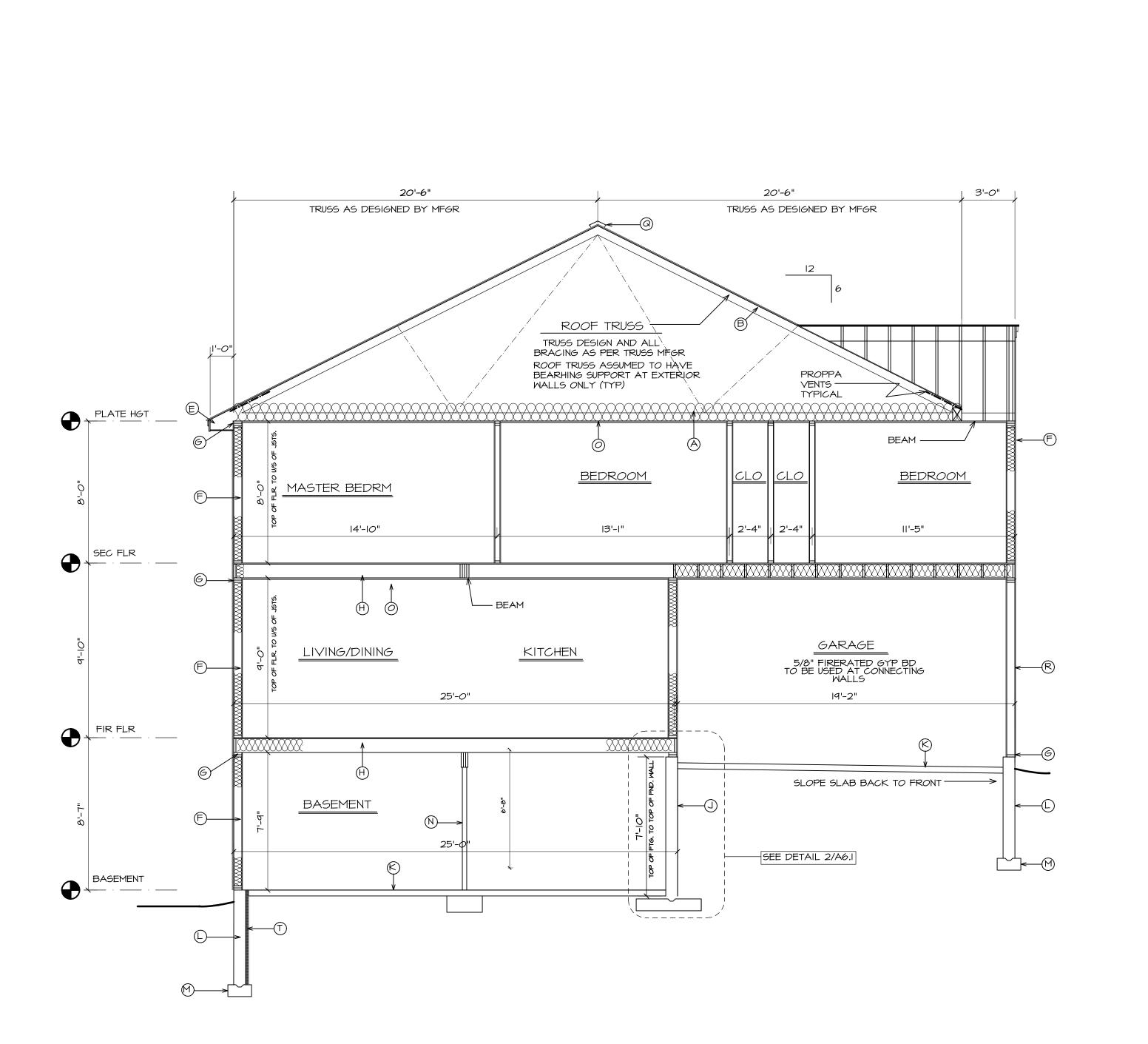
WHERE PREENGINEERED FLOOR OR ROOF TRUSSES ARE USED, TRUSS MANUFACTURER MUST PROVIDE SHOP DRAWINGS WHICH BEAR SEAL OF REGISTERED ENGINEER IN STATE IN WHICH WORK

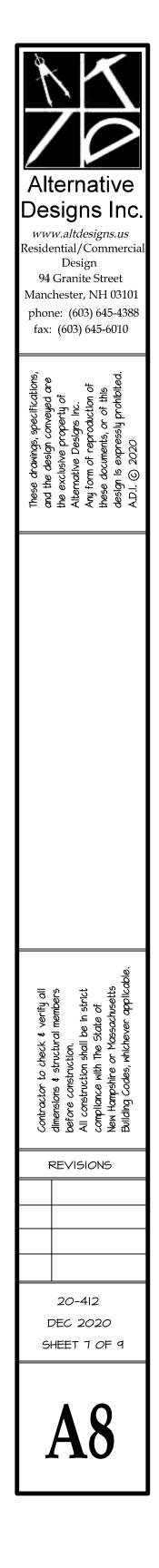
ALL LUMBER MUST BE NO. 2 OR BETTER, SPRUCE - PINE - FIR.

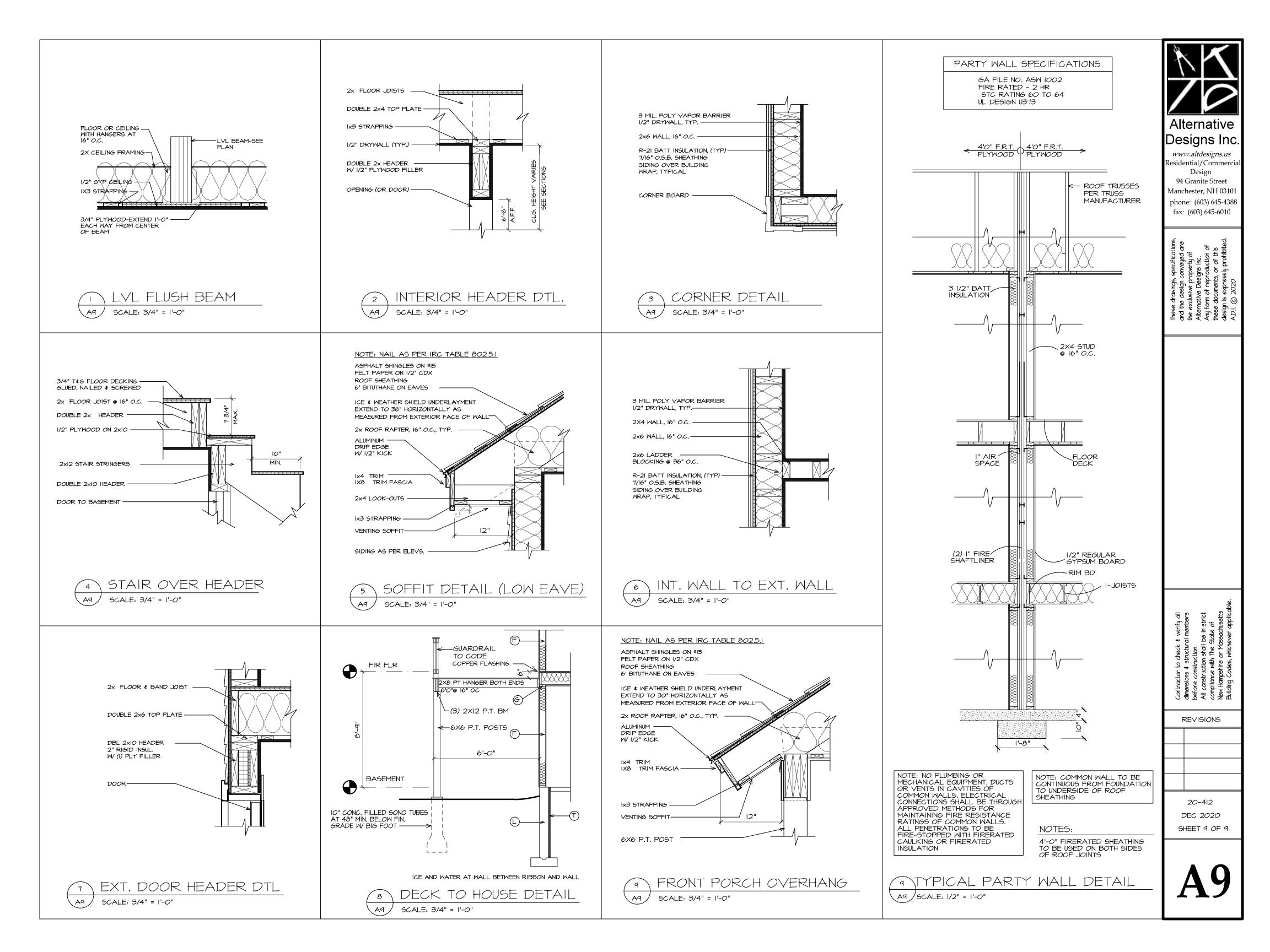
PROVIDE MOISTURE VAPOR RETARDERS IN ALL FRAMED WALLS, FLOORS AND ROOF/CEILINGS IN ACCORDANCE WITH I.R.C. SECTIONS R-506.2.3 AND RT02.7 ATTIC ACCESS (MIN 22" X 30") LOCATION TO BE DETERMINED BY CONTRACTOR

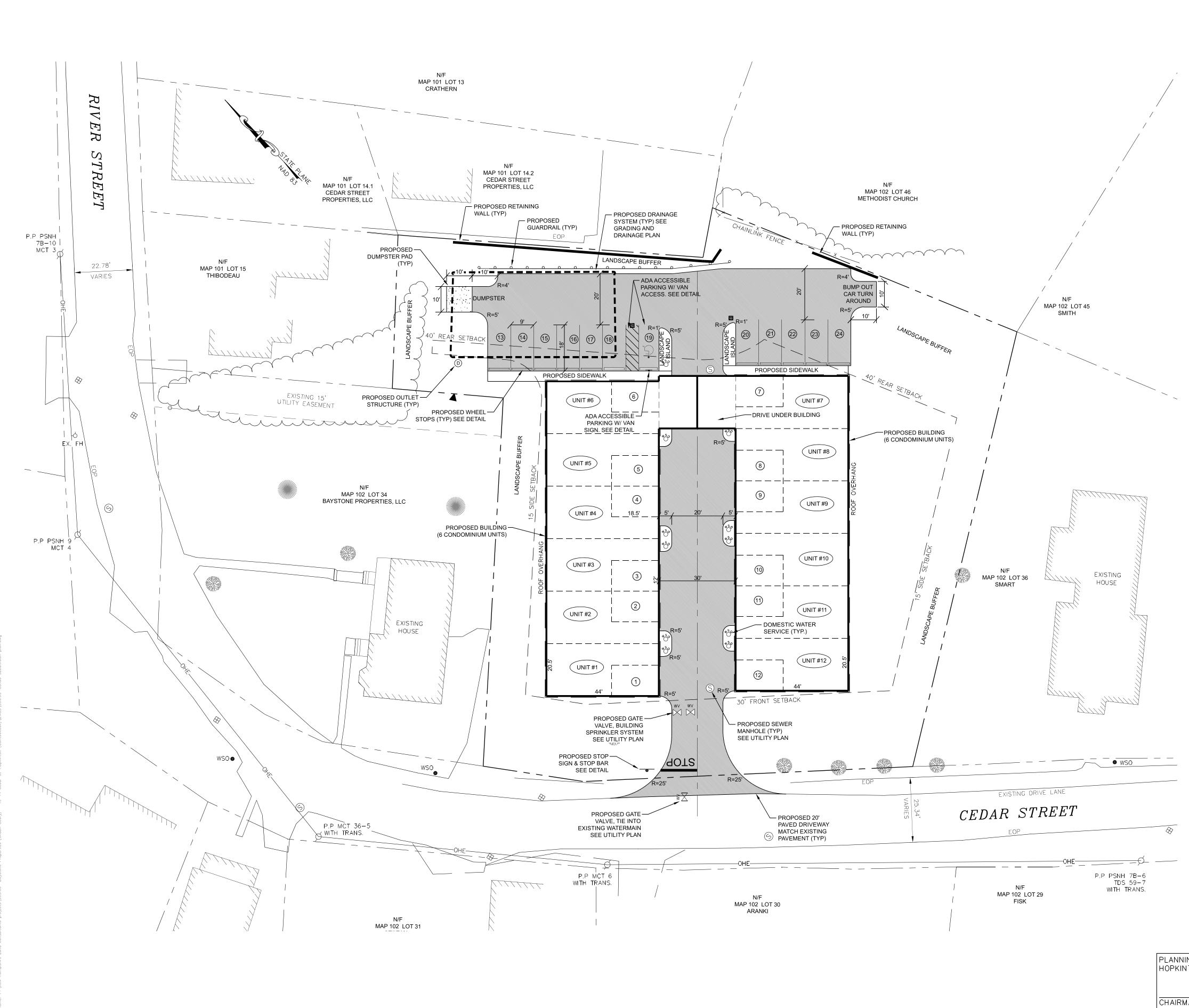


# NOTE: SEE DRAWING A7 FOR GENERAL NOTES AND ADDITIONAL INFORMATION









# RECEIVED: 07/27/2021 Planning Dept.



# <u>LEGEND</u>

EXISTING STONEWALL
ABUTTERS PROPERTY LINES
SUBJECT PROPERTY LINES
EDGE OF PAVEMENT
EXISTING FENCELINE
PROPOSED FENCELINE
WETLANDS BOUNDARY (WETLANDS OVERLAY DISTRICT)
EXISTING TREELINE
EXISTING BLDG SETBACK
EXISTING FENCE LINE
WETLANDS

SOIL TYPE/BOUNDARY DRILL HOLE FOUND REBAR W/ CAP FOUND STONE BOUND FOUND WALL MOUNTED LIGHT FIXTURE

POLE MOUNTED FIXTURES

SMALLER POLE FIXTURES PROPOSED CATCH BASIN PROPOSED SIGN

# - _____ _____ ___ ____ _____ ······ _____ X _____ 166A 0 0 $\Box$

W S3, S4, S5 P3, P4, P5 ----

PSP

SHT. 3 of 16

PLANNING BOARD APPROVAL: HOPKINTON NH PLANNING BOARD

DATE

Received: 07/21/21 Planning Dept.

## **DRAINAGE REPORT**

### A CONDOMINIUM SUBDIVISION PLAN

Tax Map 102 Lots 34 & 35 Hopkinton, NH

June 16, 2021



Prepared For:

**Baystone Properties LLC.** 126 Barton Corner Road Hopkinton NH 03229

Prepared By:

Bernie Temple, P.E. Po Box 7, Gilmanton I W, NH 03837 Phone: 603-630-1008

### **TABLE OF CONTENTS**

**Project Narrative** Introduction Pre-development Conditions Post development Conditions Methodology **Drainage Summary** Storm-water Treatment Conclusion USGS map Aerial Photo Web soil survey map **Extreme Precipitation Tables** Aerial Photograph **BMP** Worksheets Rip-rap apron calculations Pre-Drainage analysis output Pre-development 2, 10, 25, 50-year

Post-Drainage analysis output Post development 2, 10, 25, 50-year Operation and Maintenance

### **Introduction**

This proposed project is located on Map 102, Lots 34 & 35 of the Hopkinton NH tax maps. The two parcels will be combined to provide a .91-acre lot with frontage along Cedar Street. It is located in the VR-1 Village Residential district of Hopkinton. The project consists of the construction of a 2- 5500sf-6-unit condominium buildings with associated utilities and site improvements. Drainage from the developed area will be routed through one underground infiltration system, with roof drains directed to infiltration trenches on each side of the building. The site will be provided with municipal sewer and water.

### **Existing Conditions**

The property is .91 acres as noted above and currently has a garage building on the lot as well as out buildings attached to the existing home. The lot has been subdivided to provide the .91 acres, the garage and out buildings will be razed providing buildable area. The site is moderately sloped with drainage flowing from South to North and ending up in the Cedar Street drainage closed drainage system. There site is primarily grass with small isolated areas of woods. The NRCS web soils maps indicate that the one soil is present and is classified as a 613A Croghan Loamy Fine Sand This soil complex is a well-drained class "A" soil.

### Proposed development

As noted above the project consists of the construction of a 2- 5500sf-6-unit condominium buildings with associated utilities and site improvements. The site has been designed with open drainage sheeting the site to a closed drainage system within the parking lot. The drainage will be directed to three deep sump catch basins for sediment removal and to one oil water separator catch basin number 3 before entering an underground infiltration system. The exterior roof systems will be directed to two infiltration trenches on each side of the building. There is no change proposed to the existing drainage patterns. Most of the site is designed to drain to the one underground system which will outlet toward the Cedar Street closed drainage system. Because of the favorable soil conditions, the infiltration trenches and underground system is designed to retain peak stormwater flows and to infiltrate the water guality volume into the underlying soils. The onsite soils are provided from the most recent NRCS Webb Soil Survey 613A Croghan Loamy Fine Sand This soil complex is a well-drained class "A" soil. The infiltration rate for the soil as shown SSSNNE special publication No. 5 Ksat values for NH soils is 20 inches per hour this rate is then used at 50% or 10 inches per hour for design purposes. The drainage system is designed to mitigate any increase in stormwater runoff as a result from the site development per local regulations.

### **Design methodology**

The drainage analysis in this study was completed using HydroCad Version 10.1, a stormwater modeling program utilizing TR-20 and TR-55 methodology. This program performs both the hydrologic computations for determination of runoff flows, and the hydraulic calculations for pipe, ditch, and pond design. Calculations were performed for 10,25 and checked for flooding for the 50-year return frequency storms in accordance with Town regulations. Rainfall information is provided from the most recent Extreme Precipitation Tables provided from the Northeast Regional Climate Center. The following design parameters were used:

Rainfall distribution: Type III AMC: 2 Extreme Precipitation Estimates 2-year storm rainfall: 2.80 inches 10-year storm rainfall: 4.08 inches 25-year storm rainfall: 5.06 inches 50-year storm rainfall: 5.96 inches

### **Design analysis**

The drainage improvements designed for the proposed project have been designed to conform to Municipal requirements. The proposed underground drainage system is designed to mitigate peak runoff rates such that post-development flows are equal to or less than pre-development flows. The design analysis therefore includes outputs for the 10,25 and checked for the 50-year event.

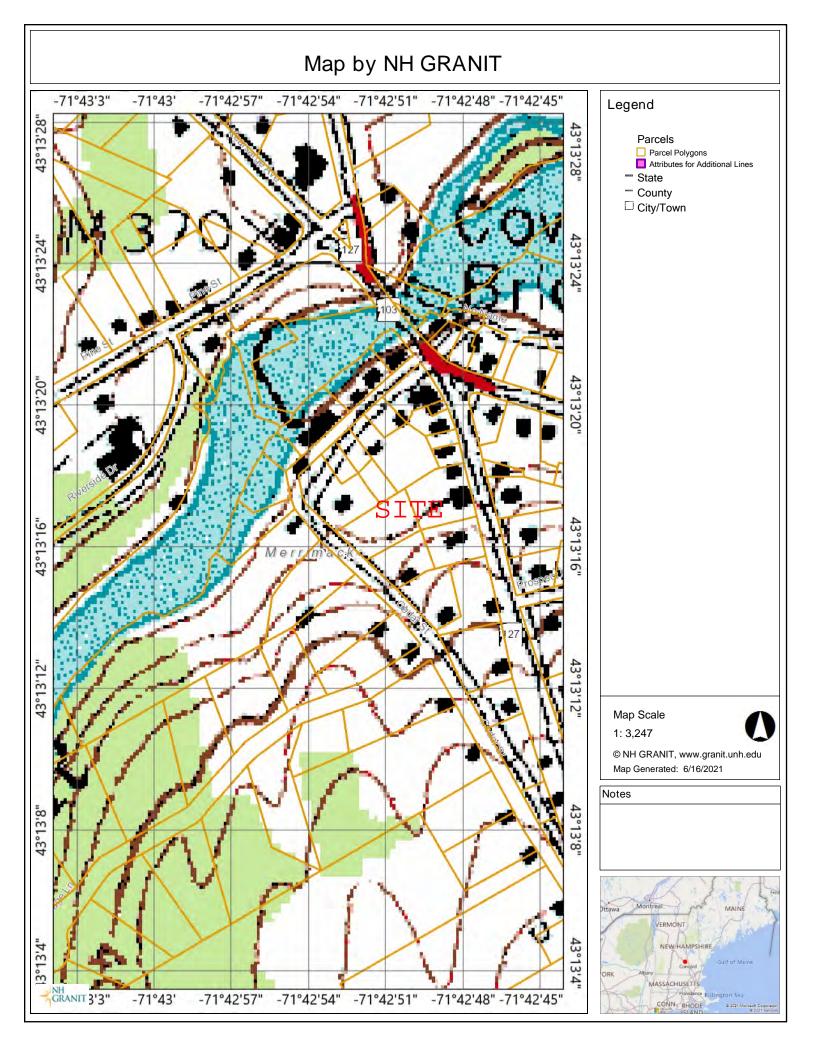
The drainage calculations are included in the appendix of this report. Peak runoff rates of the pre-development and post-development runoff rates are summarized in the following table:

Storm event / Sub-basin	Pre-development (cfs)	Phase 1 Post- development (cfs)
<u>10-year</u>		
1S/1L	.19	.15
<u>25-year</u>		
1S/1L	.65	.19

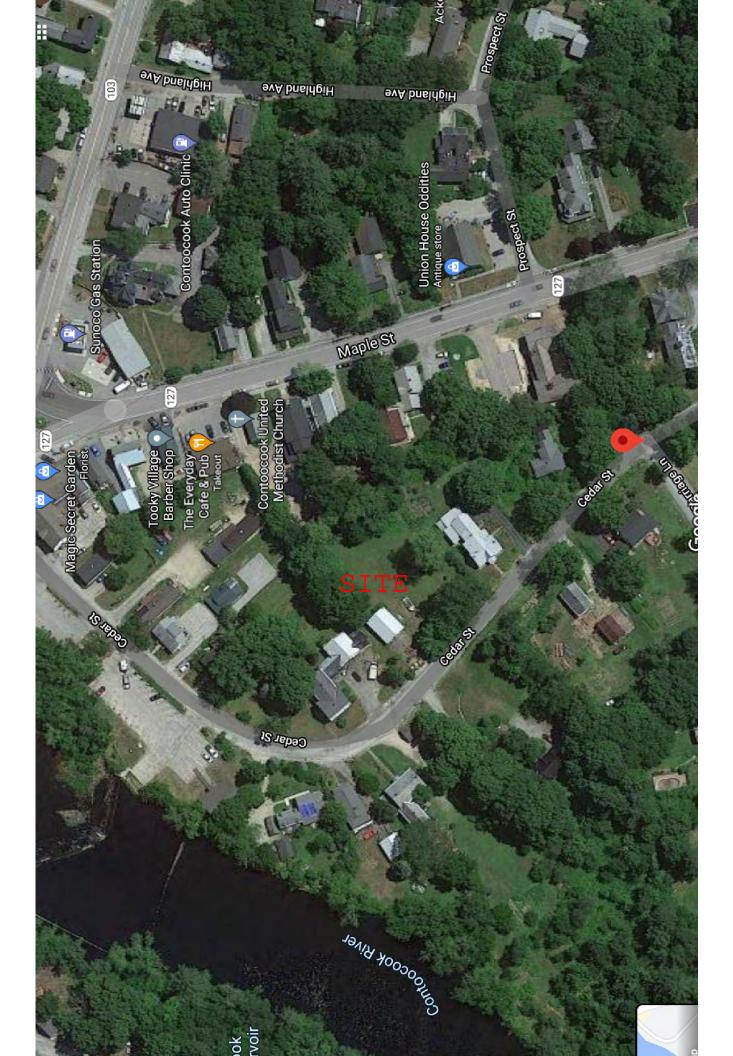
50-year		
1S/1L	1.40	.66

The calculations show that there is a decrease in peak runoff rates for the storm events analyzed. Therefore, there will be no effect on downstream drainage patterns.

## USGS



# USGS



# USGS



United States Department of Agriculture



Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Merrimack and Belknap Counties, New Hampshire



# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

#### Custom Soil Resource Report Soil Map



# **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
459C	Metacomet fine sandy loam, 8 to 15 percent slopes, very stony	1.9	26.1%
613A	Croghan loamy fine sand, 0 to 8 percent slopes, wooded	5.5	73.9%
Totals for Area of Interest		7.4	100.0%

# **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the

development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

# Merrimack and Belknap Counties, New Hampshire

### 459C—Metacomet fine sandy loam, 8 to 15 percent slopes, very stony

#### **Map Unit Setting**

National map unit symbol: 9dpq Elevation: 250 to 2,940 feet Mean annual precipitation: 40 to 50 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 135 days Farmland classification: Farmland of local importance

#### **Map Unit Composition**

Metacomet and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Metacomet**

#### Setting

Landform: Hillslopes Down-slope shape: Linear Across-slope shape: Linear Parent material: Basal melt-out till derived from granite, gneiss, or schist

#### **Typical profile**

Oe - 0 to 1 inches: slightly decomposed plant material

H1 - 1 to 9 inches: fine sandy loam

- H2 9 to 34 inches: fine sandy loam
- H3 34 to 65 inches: sandy loam

#### Properties and qualities

Slope: 8 to 15 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 20 to 36 inches to densic material
Drainage class: Moderately well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.60 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 4.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: C Hydric soil rating: No

#### **Minor Components**

#### Pillsbury

Percent of map unit: 4 percent Landform: Ground moraines Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: Yes

#### Chichester

Percent of map unit: 4 percent Landform: Hillslopes Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Skerry

Percent of map unit: 3 percent Landform: Hillslopes Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Henniker

Percent of map unit: 3 percent Landform: Hills Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Peacham

Percent of map unit: 3 percent Landform: Depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Gilmanton

Percent of map unit: 3 percent Landform: Hillslopes Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### 613A—Croghan loamy fine sand, 0 to 8 percent slopes, wooded

#### Map Unit Setting

National map unit symbol: 2wqp0 Elevation: 150 to 2,300 feet Mean annual precipitation: 40 to 55 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 135 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

*Croghan and similar soils:* 85 percent *Minor components:* 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Croghan**

#### Setting

Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope, base slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Sandy glaciofluvial deposits

#### **Typical profile**

*Oa - 0 to 4 inches:* highly decomposed plant material *E - 4 to 6 inches:* loamy fine sand *Bs - 6 to 17 inches:* loamy fine sand *BC - 17 to 30 inches:* fine sand *C - 30 to 65 inches:* sand

#### **Properties and qualities**

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 14.17 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water capacity: Low (about 4.1 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: A Hydric soil rating: No

#### **Minor Components**

#### Colton

Percent of map unit: 5 percent Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

#### Adams

Percent of map unit: 5 percent Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

#### Naumburg

Percent of map unit: 3 percent Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Footslope, toeslope Landform position (three-dimensional): Base slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Nicholville

Percent of map unit: 2 percent Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Backslope, footslope Landform position (three-dimensional): Side slope, base slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

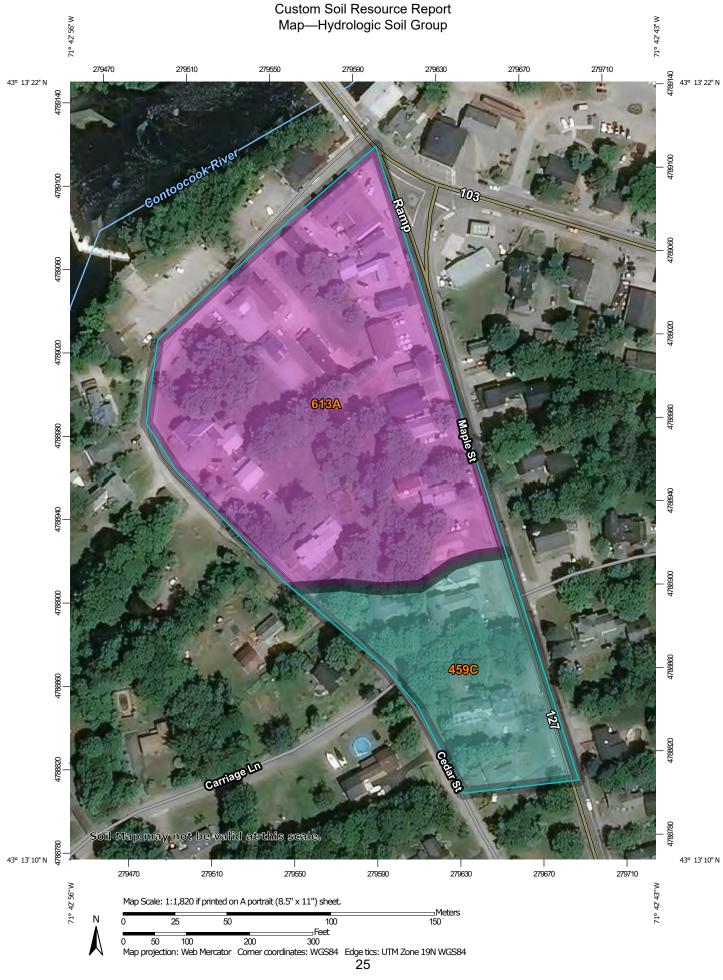
Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

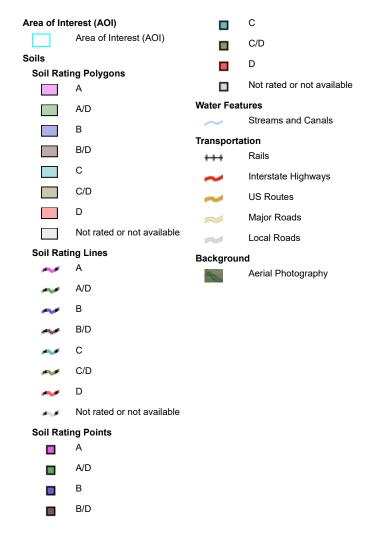
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



### MAP LEGEND



#### **MAP INFORMATION**

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Merrimack and Belknap Counties, New Hampshire Survey Area Data: Version 25, May 29, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 31, 2019—Aug 29, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

## MAP LEGEND

### MAP INFORMATION

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
459C	Metacomet fine sandy loam, 8 to 15 percent slopes, very stony	с	1.9	26.1%
613A	Croghan loamy fine sand, 0 to 8 percent slopes, wooded	A	5.5	73.9%
Totals for Area of Intere	st	7.4	100.0%	

## Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

# EXTREME PRECIPITATION TABLE

# **Extreme Precipitation Tables**

# Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New Hampshire
Location	
Longitude	71.714 degrees West
Latitude	43.220 degrees North
Elevation	0 feet
Date/Time	Fri, 21 May 2021 06:58:17 -0400

# **Extreme Precipitation Estimates**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.82	1.03	1yr	0.70	0.98	1.19	1.49	1.88	2.37	2.55	1yr	2.09	2.45	2.90	3.60	4.12	1yr
2yr	0.31	0.48	0.60	0.79	0.99	1.25	2yr	0.86	1.15	1.44	1.80	2.25	2.80	3.13	2yr	2.48	3.01	3.50	4.18	4.77	2yr
5yr	0.37	0.58	0.72	0.97	1.24	1.57	5yr	1.07	1.44	1.82	2.26	2.81	3.47	3.96	5yr	3.07	3.81	4.40	5.18	5.87	5yr
10yr	0.42	0.66	0.83	1.13	1.47	1.87	10yr	1.27	1.71	2.17	2.70	3.33	4.08	4.73	10yr	3.61	4.55	5.24	6.09	6.87	10yr
25yr	0.50	0.79	1.00	1.38	1.83	2.36	25yr	1.58	2.15	2.74	3.40	4.17	5.06	6.00	25yr	4.47	5.77	6.61	7.56	8.47	25yr
50yr	0.56	0.91	1.16	1.62	2.18	2.81	50yr	1.88	2.56	3.27	4.05	4.94	5.96	7.18	50yr	5.27	6.91	7.88	8.90	9.91	50yr
100yr	0.64	1.04	1.34	1.89	2.58	3.35	100yr	2.23	3.04	3.90	4.83	5.86	7.02	8.60	100yr	6.21	8.27	9.39	10.49	11.61	100yr
200yr	0.74	1.20	1.55	2.22	3.06	3.99	200yr	2.64	3.63	4.65	5.74	6.94	8.27	10.31	200yr	7.32	9.91	11.21	12.37	13.60	200yr
500yr	0.88	1.45	1.89	2.74	3.84	5.03	500yr	3.31	4.57	5.87	7.22	8.70	10.30	13.10	500yr	9.12	12.60	14.16	15.39	16.79	500yr

# **Lower Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.37	0.45	0.60	0.74	0.90	1yr	0.64	0.88	0.98	1.29	1.57	1.91	2.32	1yr	1.69	2.23	2.59	3.13	3.61	1yr
2yr	0.30	0.47	0.58	0.78	0.96	1.15	2yr	0.83	1.12	1.31	1.73	2.22	2.70	3.00	2yr	2.39	2.89	3.35	4.00	4.61	2yr
5yr	0.34	0.52	0.65	0.89	1.13	1.35	5yr	0.98	1.32	1.54	2.00	2.57	3.17	3.60	5yr	2.81	3.46	3.95	4.74	5.43	5yr
10yr	0.37	0.58	0.71	1.00	1.29	1.51	10yr	1.11	1.48	1.73	2.24	2.86	3.55	4.12	10yr	3.14	3.97	4.47	5.34	6.08	10yr
25yr	0.42	0.65	0.80	1.15	1.51	1.76	25yr	1.30	1.72	2.03	2.59	3.29	4.14	4.92	25yr	3.66	4.73	5.25	6.29	7.10	25yr
50yr	0.47	0.71	0.89	1.27	1.71	1.97	50yr	1.48	1.92	2.28	2.91	3.67	4.65	5.62	50yr	4.12	5.40	5.92	7.12	7.60	50yr
100yr	0.51	0.78	0.97	1.41	1.93	2.19	100yr	1.67	2.14	2.56	3.27	4.10	5.24	6.42	100yr	4.64	6.17	6.69	8.07	8.44	100yr
200yr	0.57	0.85	1.08	1.56	2.18	2.44	200yr	1.88	2.39	2.87	3.68	4.59	5.89	7.34	200yr	5.21	7.06	7.55	9.18	9.34	200yr
500yr	0.65	0.96	1.24	1.80	2.56	2.81	500yr	2.21	2.75	3.37	4.33	5.34	6.87	8.77	500yr	6.08	8.43	8.80	10.90	10.66	500yr

# **Upper Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.29	0.45	0.55	0.74	0.91	1.11	1yr	0.78	1.08	1.21	1.60	1.97	2.58	2.82	1yr	2.29	2.71	3.24	3.99	4.52	1yr
2yr	0.33	0.51	0.63	0.85	1.05	1.25	2yr	0.91	1.22	1.42	1.84	2.36	2.93	3.29	2yr	2.60	3.16	3.67	4.37	5.02	2yr
5yr	0.41	0.64	0.79	1.08	1.38	1.61	5yr	1.19	1.57	1.81	2.32	2.94	3.80	4.35	5yr	3.36	4.19	4.90	5.61	6.36	5yr
10yr	0.50	0.76	0.95	1.32	1.71	1.96	10yr	1.47	1.92	2.19	2.77	3.49	4.68	5.42	10yr	4.15	5.22	6.12	6.81	7.67	10yr
25yr	0.64	0.97	1.20	1.72	2.26	2.56	25yr	1.95	2.50	2.84	3.49	4.37	6.12	7.25	25yr	5.42	6.97	8.22	8.79	9.85	25yr
50yr	0.77	1.17	1.45	2.09	2.81	3.14	50yr	2.43	3.07	3.46	4.17	5.19	7.51	9.04	50yr	6.64	8.70	10.27	10.66	12.39	50yr
100yr	0.93	1.41	1.76	2.54	3.49	3.85	100yr	3.01	3.77	4.21	4.97	6.15	9.22	11.29	100yr	8.16	10.86	12.81	12.95	15.12	100yr
200yr	1.13	1.69	2.15	3.11	4.34	4.73	200yr	3.74	4.62	5.13	5.92	7.31	11.32	14.09	200yr	10.02	13.55	16.04	15.71	18.46	200yr
500yr	1.46	2.18	2.80	4.07	5.79	6.21	500yr	5.00	6.07	6.67	7.48	9.18	14.89	18.92	500yr	13.17	18.19	21.61	20.28	24.05	500yr



# **BMP Worksheets**



## INFILTRATION PRACTICE CRITERIA (Env-Wq 1508.06)

ADS SYSTEM

Enter the type of infiltration practice /	a a basin tranch) and the node name	in the drainage analysis, if annliashle
Enter the type of minitration practice (	e.g., basin, trench) and the node name	in the drainage analysis, if applicable.

	-	Have you reviewed Env-Wq 1508.06(a) to ensure that infiltration is allowed?	← yes
0.41	ас	A = Area draining to the practice	
0.41	ас	A _I = Impervious area draining to the practice	
1.00	decimal	I = Percent impervious area draining to the practice, in decimal form	
0.95	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x l)	
0.39	ac-in	WQV= 1" x Rv x A	
1,414	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
353		25% x WQV (check calc for sediment forebay volume)	
deep sump	hooded cb	Method of pretreatment? (not required for clean or roof runoff)	
	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	<u>&gt;</u> 25%WQV
1,516	cf	V = Volume ¹ (attach a stage-storage table)	<u>&gt;</u> WQV
2,000	sf	A _{SA} = Surface area of the bottom of the pond	
10.00	iph	Ksat _{DESIGN} = Design infiltration rate ²	
0.8	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	< 72-hrs
362.30	feet	E _{BTM} = Elevation of the bottom of the basin	
359.50	feet	$E_{SHWT}$ = Elevation of SHWT (if none found, enter the lowest elevation of the test p	oit)
359.00	feet	$E_{ROCK}$ = Elevation of bedrock (if none found, enter the lowest elevation of the test	t pit)
2.80	feet	D _{SHWT} = Separation from SHWT	<u>&gt;</u> * ³
3.3	feet	D _{ROCK} = Separation from bedrock	<u>&gt;</u> * ³
	ft	D _{amend} = Depth of amended soil, if applicable due high infiltation rate	> 24"
	ft	$D_T$ = Depth of trench, if trench proposed	4 - 10 ft
	Yes/No	If a trench or underground system is proposed, has observation well been provid	ed? <b>←yes</b>
	•	If a trench is proposed, does materialmeet Env-Wq 1508.06(k)(2) requirements. ⁴	← yes
	Yes/No	If a basin is proposed, Is the perimeter curvilinear, and basin floor flat?	← yes
	:1	If a basin is proposed, pond side slopes.	<u>&gt;</u> 3:1
363.29	ft	Peak elevation of the 10-year storm event (infiltration can be used in analysis)	
364.14	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
366.00	ft	Elevation of the top of the practice (if a basin, this is the elevation of the berm)	
YES		10 peak elevation $\leq$ Elevation of the top of the trench?	← yes
YES		If a basin is proposed, 50-year peak elevation $\leq$ Elevation of berm?	← yes

1. Volume below the lowest invert of the outlet structure and excludes forebay volume

2. Ksat_{DESIGN} includes a factor of safety. See Env-Wq 1504.14 for requirements for determining the infiltr. rate

3. 1' separation if treatment not required; 4' for treatment in GPAs & WSIPAs; & 3' in all other areas.

4. Clean, washed well graded diameter of 1.5 to 3 inches above the in-situ soil.

5. If 50-year peak elevation exceeds top of trench, the overflow must be routed in HydroCAD as secondary discharge.

#### **Designer's Notes:**

Elevation	Discharge	Discarded	Primary	Elevation	Discharge	Discarded	Primary
(feet)	(cfs)	(cfs)	(cfs)	(feet)	(cfs)	(cfs)	(cfs)
362.30	0.00	0.00	0.00	364.95	4.15	0.47	3.69
362.35	0.46	0.46	0.00	365.00	4.25	0.47	3.78
362.40	0.46	0.46	0.00	365.05	4.34	0.47	3.88
362.45	0.46	0.46	0.00	365.10	4.43	0.47	3.97
362.50	0.47	0.47	0.00	365.15	4.52	0.47	4.06
362.55	0.47	0.47	0.00	365.20	4.61	0.47	4.14
362.60	0.47	0.47	0.00	365.25	4.69	0.47	4.23
362.65	0.47	0.47	0.00	365.30	4.78	0.47	4.31
362.70	0.47	0.47	0.00	365.35	4.86	0.47	4.39
362.75	0.47	0.47	0.00	365.40	4.94	0.47	4.47
362.80	0.47	0.47	0.00	365.45	5.02	0.47	4.55
362.85	0.47	0.47	0.00	365.50	5.10	0.47	4.63
362.90	0.47	0.47	0.00	365.55	5.17	0.47	4.71
362.95	0.47	0.47	0.00	365.60	5.25	0.47	4.78
363.00	0.47	0.47	0.00	365.65	5.32	0.47	4.86
363.05	0.47	0.47	0.00	365.70	5.40	0.47	4.93
363.10	0.47	0.47	0.00	365.75	5.47	0.47	5.00
363.15	0.47	0.47	0.00	365.80	5.54	0.47	5.07
363.20	0.47	0.47	0.00	365.85	5.61	0.47	5.14
363.25	0.47	0.47	0.00	365.90	5.68	0.47	5.21
363.30	0.47	0.47	0.00	365.95	5.75	0.47	5.28
363.35	0.47	0.47	0.00	366.00	5.81	0.47	5.35
363.40	0.47	0.47	0.00	366.05	5.88	0.47	5.41
363.45	0.47	0.47	0.00	366.10	5.95	0.47	5.48
363.50	0.47	0.47	0.00	366.15	6.01	0.47	5.55
363.55	0.47	0.47	0.00	366.20	6.08	0.47	5.61
363.60	0.47	0.47	0.00	366.25	6.14	0.47	5.67
363.65	0.47	0.47	0.00	366.30	6.20	0.47	5.74
363.70	0.47	0.47	0.00	366.35	6.26	0.47	5.80
363.75	0.47	0.47	0.00	366.40	6.32	0.47	5.86
363.80 363.85	0.47 0.47	0.47 0.47	0.00 0.00	366.45 366.50	6.39 <b>6.45</b>	0.47 0.47	5.92 <b>5.98</b>
363.85	0.47	0.47	0.00	300.50	0.45	0.47	5.90
363.90	0.47	0.47	0.00				
364.00	0.47	0.47	0.00				
364.00	0.47	0.47	0.00				
364.10	0.30	0.47	0.32				
364.15	1.06	0.47	0.60				
364.20	1.38	0.47	0.92				
364.25	1.75	0.47	1.28				
364.30	2.15	0.47	1.69				
364.35	2.59	0.47	2.13				
364.40	2.86	0.47	2.39				
364.45	3.00	0.47	2.54				
364.50	3.14	0.47	2.67				
364.55	3.27	0.47	2.80				
364.60	3.40	0.47	2.93				
364.65	3.51	0.47	3.05				
364.70	3.63	0.47	3.16				
364.75	3.74	0.47	3.28				
364.80	3.85	0.47	3.38				
364.85	3.95	0.47	3.49				
364.90	4.05	0.47	3.59				
			I				

### Stage-Discharge for Pond 12P: ug storage for roof

Baystone Properties, LLC 49 & 71 Cedar Street Contoocook, NH Date: 5-15-21

#### Test Pit #1

- 0-12" Topsoil 10 YR 3/3 – Dark Brown
- 12-28" Sand Granular/Friable 7.5 YR 5/6 – Strong Brown
- 28-40" Sand Granular/Loose 2.5 Y 5/6 – Light Olive Brown ESHWT = 28" Roots to 28" No ledge observed Water observed @40" Perc Rate 2 min./inch

#### Test Pit #2

- 0-12" Topsoil 10 YR 3/3 – Dark Brown
- 12-30" Sand Granular/Friable 7.5 YR 5/6 – Strong Brown
- 30-72" Loamy SandGranular/Firm2.5 YR 5/6 Light Olive Brown

ESHWT = 30"

Roots to 30"

No ledge observed

Water observed @72"

Perc Rate 2 min./inch

# **Rip-rap apron calculations**

RIPRAP CALCULATIONSDESIGN STORM: 25YEARSDATE: 06/16/21REVISED:PROJECT NAME: SITE PLAN<br/>LOCATION: HOPKINTON NHJOB NO.

VARIABLES:

Q = DISCHARGE FROM OUTLET Do = PIPE DIAMETER Tw = TAIL WATER La = LENGTH OF RIPRAP Wi = WIDTH OF RIPRAP AT OUTLET We = WIDTH OF RIPRAP DOWNSTREAM FROM OUTLET d50 = RIPRAP SIZE

EQUATIONS:

FOR Tw < 1/2 Do

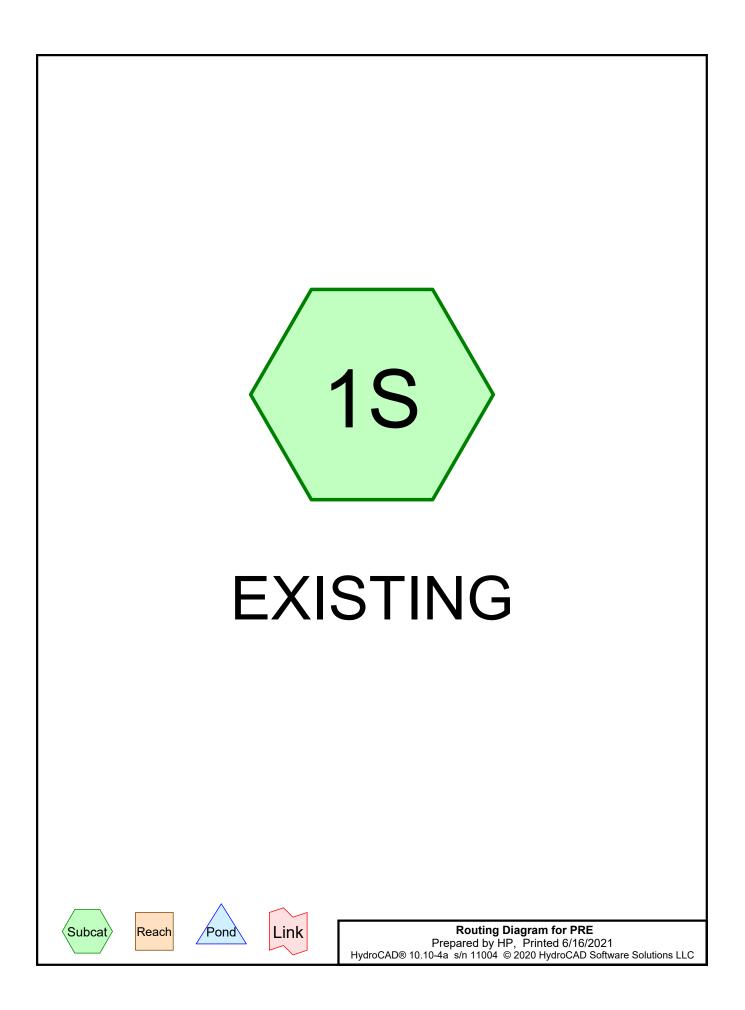
La =  $(1.8Q/Do^{1.5}) + 7Do$ Wi = 3Do We = Do + La d50 =  $(0.02Q^{1.33})/(Tw)(Do)$  FOR Tw > or = 1/2 Do

La =  $(3Q)/(Do^{1.5})$ Wi = 3Do We = Do + 0.4La d50 =  $(0.02Q^{1.33})/(Tw)(Do)$ 

OUTLET	Q CFS	Do FEET	Tw FEET	La FEET	Wi FEET	We FEET	d50 INCHES
FES 1	0.51	1.00	0.10	7.92	3.00	8.92	0.98
			0.20	#DIV/0!	0.00	#DIV/0!	#DIV/0!
			0.10	#DIV/0!	0.00	#DIV/0!	#DIV/0!

# Pre-Drainage analysis output

Pre-development 10, 25, 50-year



## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
1.295	39	>75% Grass cover, Good, HSG A (1S)
0.184	98	Paved parking, HSG A (1S)
0.096	98	Roofs, HSG A (1S)
0.167	30	Woods, Good, HSG A (1S)
1.742	48	TOTAL AREA

## Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
1.742	HSG A	1S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
1.742		TOTAL AREA

### Summary for Subcatchment 1S: EXISTING

Runoff = 0.19 cfs @ 12.36 hrs, Volume= 0.035 af, Depth> 0.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

A	rea (sf)	CN [	Description					
	4,185	98 Roofs, HSG A						
	4,530	98 F	98 Paved parking, HSG A					
	7,270	30 \	Woods, Good, HSG A					
	56,423	39 >	39 >75% Grass cover, Good, HSG A					
	3,468	98 F	98 Paved parking, HSG A					
	75,876	48 \	48 Weighted Average					
	63,693 83.94% Pervious Area							
	12,183	83 16.06% Impervious Area						
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.2	40	0.0600	0.21		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.80"			
3.3	400	0.0850	2.04		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
6.5	440	Total						

### Summary for Subcatchment 1S: EXISTING

Runoff = 0.65 cfs @ 12.16 hrs, Volume= 0.077 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

A	rea (sf)	CN [	Description				
	4,185	98 F	98 Roofs, HSG A				
	4,530	98 F	Paved parking, HSG A				
	7,270	30 V	Woods, Good, HSG A				
	56,423	39 >	>75% Grass cover, Good, HSG A				
	3,468	98 F	98 Paved parking, HSG A				
	75,876	48 V	48 Weighted Average				
	63,693 83.94% Pervious Area						
	12,183	16.06% Impervious Area					
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
3.2	40	0.0600	0.21		Sheet Flow,		
					Grass: Short n= 0.150 P2= 2.80"		
3.3	400	0.0850	2.04		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
6.5	440	Total					

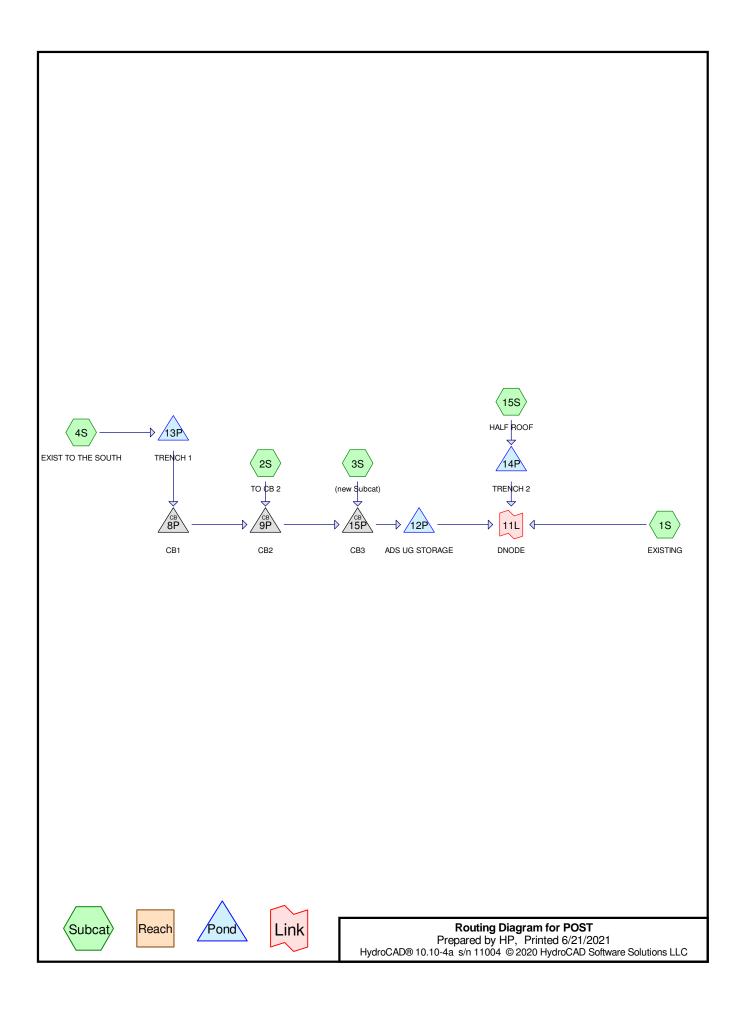
### Summary for Subcatchment 1S: EXISTING

Runoff = 1.40 cfs @ 12.12 hrs, Volume= 0.126 af, Depth> 0.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

A	rea (sf)	CN [	Description				
	4,185	98 F	98 Roofs, HSG A				
	4,530	98 F	Paved parking, HSG A				
	7,270	30 V	Woods, Good, HSG A				
	56,423	39 >	>75% Grass cover, Good, HSG A				
	3,468	98 F	98 Paved parking, HSG A				
	75,876	48 V	48 Weighted Average				
	63,693 83.94% Pervious Area						
	12,183	16.06% Impervious Area					
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
3.2	40	0.0600	0.21		Sheet Flow,		
					Grass: Short n= 0.150 P2= 2.80"		
3.3	400	0.0850	2.04		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
6.5	440	Total					

Post-Drainage analysis output Post development 10, 25, 50-year



## Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
42,200	39	>75% Grass cover, Good, HSG A (1S, 4S, 15S)
4,149	98	Paved parking, HSG A (1S, 4S, 15S)
25,164	98	Roofs, HSG A (1S, 2S, 3S, 4S, 15S)
4,363	30	Woods, Good, HSG A (1S, 4S)
75,876	61	TOTAL AREA

### Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
75,876	HSG A	1S, 2S, 3S, 4S, 15S
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
75,876		TOTAL AREA

#### Summary for Subcatchment 1S: EXISTING

Runoff = 0.15 cfs @ 12.09 hrs, Volume= 557 cf, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

A	rea (sf)	CN	Description		
	1,385	98	Roofs, HSC	λA	
	340	98	Paved park	ing, HSG A	
	2,591	30	Woods, Go	od, HSG A	
	14,087	39	>75% Gras	s cover, Go	bod, HSG A
	18,403		Weighted A	verage	
	16,678	38	90.63% Per	rvious Area	
	1,725	98	9.37% Impe	ervious Area	a
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)	
3.9	50	0.055	0 0.21		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.90"
0.7	183	0.090	0 4.50		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.6	233	Total,	Increased I	o minimum	1 Tc = 6.0 min

#### Summary for Subcatchment 2S: TO CB 2

Runoff = 0.79 cfs @ 12.09 hrs, Volume= 2,640 cf, Depth> 3.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

A	rea (sf)	CN	Description		
	8,850	98	Roofs, HSG	àΑ	
	8,850	98	100.00% In	pervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6.0					Direct Entry,

#### Summary for Subcatchment 3S: (new Subcat)

Runoff	=	0.80 cfs @	12.09 hrs, Volume=	2,678 cf, Depth> 3.58"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

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CN Description
98 Roofs, HSG A
98 100.00% Impervious Area
Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
Direct Entry,

#### Summary for Subcatchment 4S: EXIST TO THE SOUTH

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 1,935 cf, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

Area (sf)	CN	Description
22,564	39	>75% Grass cover, Good, HSG A
3,468	98	Paved parking, HSG A
1,772	30	Woods, Good, HSG A
2,790	98	Roofs, HSG A
30,594		Weighted Average
24,336	38	79.55% Pervious Area
6,258	98	20.45% Impervious Area
Tc Length (min) (feet)	Sloj (ft/	
6.0		Direct Entry,

#### Summary for Subcatchment 15S: HALF ROOF

Runoff	=	0.31 cfs @	12.09 hrs,	Volume=	1,061 cf, Depth> 1.4	11"
--------	---	------------	------------	---------	----------------------	-----

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.08"

A	rea (sf)	CN	Description			
	5,549	39	>75% Gras	s cover, Go	ood, HSG A	
	3,160	98	Roofs, HSC	λA		
	341	98	Paved park	ing, HSG A	A	
	9,050		Weighted A	verage		
	5,549	39	61.31% Per	vious Area	1	
	3,501	98	38.69% Imp	pervious Ar	rea	
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description	
6.0					Direct Entry,	

### Summary for Pond 8P: CB1

Inflow Area =	=	30,594 sf,	20.45% Impervious,	Inflow Depth = 0.00"	for 10-YR event
Inflow =	: 0	.00 cfs @	5.00 hrs, Volume=	0 cf	
Outflow =	: 0	.00 cfs @	5.00 hrs, Volume=	0 cf, Atten	= 0%, Lag= 0.0 min
Primary =	: 0	.00 cfs @	5.00 hrs, Volume=	0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.22' @ 12.69 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	363.04'	<b>12.0" Round Culvert</b> L= 51.0' Ke= 0.500
			Inlet / Outlet Invert= 363.04' / 362.79' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=363.04' TW=362.75' (Dynamic Tailwater)

#### Summary for Pond 9P: CB2

Inflow Area =	39,444 sf, 38.30% Impervious,	Inflow Depth > 0.80" for 10-YR event
Inflow =	0.79 cfs @ 12.09 hrs, Volume=	2,640 cf
Outflow =	0.79 cfs @ 12.09 hrs, Volume=	2,640 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.79 cfs @ 12.09 hrs, Volume=	2,640 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.46' @ 12.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.69'	<b>12.0" Round Culvert</b> L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 362.69' / 362.50' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=363.42' TW=363.36' (Dynamic Tailwater)

#### Summary for Pond 12P: ADS UG STORAGE

Inflow Area =	48,423 sf, 49.74% Impervious,	Inflow Depth > 1.32" for 10-YR event
Inflow =	1.59 cfs @ 12.09 hrs, Volume=	5,318 cf
Outflow =	0.47 cfs @ 12.00 hrs, Volume=	5,321 cf, Atten= 71%, Lag= 0.0 min
Discarded =	0.47 cfs @ 12.00 hrs, Volume=	5,321 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.29' @ 12.41 hrs Surf.Area= 2,013 sf Storage= 915 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 9.0 min (745.4 - 736.5)

#### POST

Type III 24-hr 10-YR Rainfall=4.08" Printed 6/21/2021 Page 7

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Volume	Invert	Avail.Storage Storage Description			
#1	362.30'	1,66	69 cf	f Custom Stage Data (Prismatic) Listed below (Recalc)	
				4,400 cf Overall - 228 cf Embedded = 4,172 cf x 40.0% Voids	
#2	362.50'	22	28 cf		
				L= 290.0'	
#3	362.50'	5	50 cf	4.00'D x 4.00'H Vertical Cone/Cylinder	
		1,94	I7 cf	f Total Available Storage	
				Ŭ	
Elevatio	on Su	rf.Area	Inc	nc.Store Cum.Store	
(fee	et)	(sq-ft)	(cubi	bic-feet) (cubic-feet)	
362.3	30	2,000		0 0	
364.5		2,000		4,400 4,400	
		_,		,	
Device	Routing	Invert	Outl	utlet Devices	
#1	Discarded	362.30'	10.0	0.000 in/hr Exfiltration over Surface area	
#2	Device 3	364.00'		<b>2.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#3	Primary	362.40'		2.0" Round Culvert L= 16.0' Ke= 0.500	
10	. milary	002.10		let / Outlet Invert= 362.40' / 362.24' S= 0.0100 '/' Cc= 0.900	
				= 0.013, Flow Area= 0.79 sf	
			- U	- 0.010, 1100 / 100 - 0.70 01	
<b>Discarded OutFlow</b> Max=0.47 cfs @ 12.00 hrs HW=362.50' (Free Discharge)					

**1=Exfiltration** (Exfiltration Controls 0.47 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=362.30' TW=0.00' (Dynamic Tailwater) -3=Culvert (Controls 0.00 cfs)

**2=Orifice/Grate** (Controls 0.00 cfs)

#### Summary for Pond 13P: TRENCH 1

Inflow Area =	30,594 sf, 20.45% Impervious,	Inflow Depth > 0.76" for 10-YR event
Inflow =	0.56 cfs @ 12.09 hrs, Volume=	1,935 cf
Outflow =	0.18 cfs @ 12.66 hrs, Volume=	1,941 cf, Atten= 68%, Lag= 34.1 min
Discarded =	0.18 cfs @ 12.66 hrs, Volume=	1,941 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 367.92' @ 12.38 hrs Surf.Area= 772 sf Storage= 312 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 7.7 min (753.7 - 746.0)

Volume	Invert	Avail.Stor	age	Storage Description
#1	367.00'	78	0 cf	5.00'W x 130.00'L x 3.00'H Prismatoid
				1,950 cf Overall x 40.0% Voids
#2	367.25'	10	2 cf	12.0" Round Pipe Storage
				L= 130.0'
		88	2 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Discarded	367.00'	10.0	00 in/hr Exfiltration over Surface area
#2	Primary	369.50'	4.0'	long x 4.0' breadth Broad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

**Discarded OutFlow** Max=0.18 cfs @ 12.66 hrs HW=367.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=367.00' TW=363.04' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond 14P: TRENCH 2

Inflow Area =	9,050 sf, 38.69% Impervious,	Inflow Depth > 1.41" for 10-YR event
Inflow =	0.31 cfs @ 12.09 hrs, Volume=	1,061 cf
Outflow =	0.14 cfs @ 12.28 hrs, Volume=	1,062 cf, Atten= 57%, Lag= 11.4 min
Discarded =	0.14 cfs @ 12.28 hrs, Volume=	1,062 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 362.52' @ 12.28 hrs Surf.Area= 586 sf Storage= 120 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 3.7 min (744.4 - 740.7)

Volume	Invert	Avail.Stor	age	Storage Description	
#1	362.00'	38	84 cf	4.00'W x 120.00'L x 2.00'H Prismatoid	
#2	362.25'	94 cf		960 cf Overall x 40.0% Voids <b>12.0" Round Pipe Storage</b> L= 120.0'	
	478 cf Total Available Storage				
Device #1 #2	Routing Discarded Primary	Invert 362.00' 363.50'	<b>10.0</b> <b>4.0'</b> Hea 2.50 Coe	et Devices         00 in/hr Exfiltration over Surface area         long x 4.0' breadth Broad-Crested Rectangular Weir         d (feet)       0.20       0.40       0.60       0.80       1.00       1.20       1.40       1.60       1.80       2.00         0 3.00       3.50       4.00       4.50       5.00       5.50       5.10       5.10       5.10       5.10       5.10       5.10       5.10       5.10       5.10       5.10       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.20       5.	

**Discarded OutFlow** Max=0.14 cfs @ 12.28 hrs HW=362.52' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=362.00' TW=0.00' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond 15P: CB3

Inflow Area =	48,423 sf	, 49.74% Impervious,	Inflow Depth > 1.	.32" for 10-YR event
Inflow =	1.59 cfs @	12.09 hrs, Volume=	5,318 cf	
Outflow =	1.59 cfs @	12.09 hrs, Volume=	5,318 cf,	Atten= 0%, Lag= 0.0 min
Primary =	1.59 cfs @	12.09 hrs, Volume=	5,318 cf	

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.38' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.50'	<b>12.0" Round Culvert</b> L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 362.50' / 362.50' S= 0.0000 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.55 cfs @ 12.09 hrs HW=363.36' TW=362.80' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 1.55 cfs @ 2.88 fps)

#### Summary for Link 11L: DNODE

Inflow Area =	75,876 sf, 38.63% Impervious,	Inflow Depth > 0.09" for 10-YR event
Inflow =	0.15 cfs @ 12.09 hrs, Volume=	557 cf
Primary =	0.15 cfs @ 12.09 hrs, Volume=	557 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment 1S: EXISTING

Runoff = 0.19 cfs @ 12.09 hrs, Volume= 841 cf, Depth> 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

Α	rea (sf)	CN	Description		
	1,385	98	Roofs, HSC	λA	
	340	98	Paved park	ing, HSG A	
	2,591	30	Woods, Go	od, HSG A	
	14,087	39	>75% Gras	s cover, Go	bod, HSG A
	18,403		Weighted A	verage	
	16,678	38	90.63% Per	rvious Area	
	1,725	98	9.37% Impe	ervious Area	a
Tc	Length	Slop		Capacity	Description
(min)	(feet)	(ft/fl	t) (ft/sec)	(cfs)	
3.9	50	0.055	0 0.21		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.90"
0.7	183	0.090	0 4.50		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
4.6	233	Total,	Increased I	o minimum	Tc = 6.0 min

#### Summary for Subcatchment 2S: TO CB 2

Runoff = 0.98 cfs @ 12.09 hrs, Volume= 3,299 cf, Depth> 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

A	rea (sf)	CN	Description		
	8,850	98	Roofs, HSG	àΑ	
	8,850	98	100.00% In	pervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

#### Summary for Subcatchment 3S: (new Subcat)

Runoff	=	1.00 cfs @	12.09 hrs, Volume=	3,347 cf, Depth> 4.47"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

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Area (sf)	CN	Description		
8,979	98	Roofs, HSG	àΑ	
8,979	98	100.00% lm	pervious A	Area
Tc Length (min) (feet) 6.0	Slop (ft/		Capacity (cfs)	Description Direct Entry,

#### Summary for Subcatchment 4S: EXIST TO THE SOUTH

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 2,650 cf, Depth> 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

Area (s	sf) CN	Description		
22,5	64 39	>75% Grass	s cover, Go	iood, HSG A
3,40	68 98	Paved parki	ing, HSG A	A
1,7	72 30	Woods, Goo	od, HSG A	A
2,79	90 98	Roofs, HSG	i A	
30,59	94	Weighted A	verage	
24,3	36 38	79.55% Per	vious Area	a
6,2	58 98	20.45% Impervious Area		
Ta lan	ath Cla		Consoitu	Description
Tc Len	• ·		Capacity	
	eet) (ft/	ft) (ft/sec)	(cfs)	
6.0				Direct Entry,

### Summary for Subcatchment 15S: HALF ROOF

Runoff	=	0.39 cfs @	12.09 hrs,	Volume=	1,383 cf, Depth> 1	.83"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=5.06"

A	rea (sf)	CN	Description			
	5,549	39	>75% Gras	s cover, Go	ood, HSG A	
	3,160	98	Roofs, HSC	λA		
	341	98	Paved park	ing, HSG A	A	
	9,050		Weighted A	verage		
	5,549	39	61.31% Per	vious Area	1	
	3,501	98	38.69% Impervious Area			
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description	
6.0					Direct Entry,	

#### Summary for Pond 8P: CB1

Inflow Area	=	30,594 sf,	20.45% Impervious,	Inflow Depth = 0.00"	for 25-YR event
Inflow =	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Outflow =	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min
Primary =	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.33' @ 13.18 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	363.04'	<b>12.0" Round Culvert</b> L= 51.0' Ke= 0.500
			Inlet / Outlet Invert= 363.04' / 362.79' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=363.04' TW=362.76' (Dynamic Tailwater)

#### Summary for Pond 9P: CB2

Inflow Area	=	39,444 sf	, 38.30% Impervious	, Inflow Depth > [·]	1.00"	for 25-YR event
Inflow =	=	0.98 cfs @	12.09 hrs, Volume=	3,299 cf		
Outflow =	=	0.98 cfs @	12.09 hrs, Volume=	3,299 cf,	Atten=	= 0%, Lag= 0.0 min
Primary =	=	0.98 cfs @	12.09 hrs, Volume=	3,299 cf		-

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.90' @ 12.56 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.69'	<b>12.0" Round Culvert</b> L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 362.69' / 362.50' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.60 cfs @ 12.09 hrs HW=363.54' TW=363.49' (Dynamic Tailwater)

#### Summary for Pond 12P: ADS UG STORAGE

Inflow Area =	48,423 sf, 49.74% Impervious,	Inflow Depth > 1.65" for 25-YR event
Inflow =	1.98 cfs @ 12.09 hrs, Volume=	6,646 cf
Outflow =	0.47 cfs @ 11.95 hrs, Volume=	6,670 cf, Atten= 76%, Lag= 0.0 min
Discarded =	0.47 cfs @ 11.95 hrs, Volume=	6,670 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.89' @ 12.47 hrs Surf.Area= 2,013 sf Storage= 1,423 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 15.1 min (750.1 - 735.0)

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Type III 24-hr 25-YR Rainfall=5.06" Printed 6/21/2021 Page 13

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Volume	Invert	Avail.Storag	je Storage Description
#1	362.30'	1,669	cf Custom Stage Data (Prismatic) Listed below (Recalc)
		,	4,400 cf Overall - 228 cf Embedded = 4,172 cf x 40.0% Voids
#2	362.50'	228	cf 12.0" Round Pipe Storage Inside #1
			L= 290.0'
#3	362.50'	50	cf 4.00'D x 4.00'H Vertical Cone/Cylinder
		1.947	cf Total Available Storage
		.,•	
Elevatio	on Sui	f.Area	Inc.Store Cum.Store
(fee		(sq-ft) (ci	ubic-feet) (cubic-feet)
362.3		2,000	0 0
364.5		2,000	4,400 4,400
00110		2,000	1,100
Device	Routing	Invert C	Dutlet Devices
#1	Discarded	362.30' 1	0.000 in/hr Exfiltration over Surface area
#2	Device 3	002.00	<b>2.0" Horiz. Orifice/Grate</b> $C = 0.600$ Limited to weir flow at low heads
#3			<b>2.0" Round Culvert</b> L= 16.0' Ke= 0.500
10	. may		hlet / Outlet Invert= 362.40' / 362.24' S= 0.0100 '/' Cc= 0.900
			= 0.013, Flow Area = 0.79 sf
Discard	ed OutFlow	Max=0.47 cfs @	2 11.95 hrs HW=362.54' (Free Discharge)
		filtration Contro	
	(		

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=362.30' TW=0.00' (Dynamic Tailwater) **1−3=Culvert** (Controls 0.00 cfs)

**2=Orifice/Grate** (Controls 0.00 cfs)

#### Summary for Pond 13P: TRENCH 1

Inflow Area =	30,594 sf, 20.45% Impervious,	Inflow Depth > 1.04" for 25-YR event
Inflow =	0.69 cfs @ 12.09 hrs, Volume=	2,650 cf
Outflow =	0.18 cfs @ 13.50 hrs, Volume=	2,654 cf, Atten= 74%, Lag= 84.7 min
Discarded =	0.18 cfs @ 13.50 hrs, Volume=	2,654 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 368.63' @ 12.53 hrs Surf.Area= 650 sf Storage= 526 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 16.3 min (774.1 - 757.8)

Volume	Invert	Avail.Stor	age	Storage Description
#1	367.00'	78	0 cf	5.00'W x 130.00'L x 3.00'H Prismatoid
				1,950 cf Overall x 40.0% Voids
#2	367.25'	10	2 cf	12.0" Round Pipe Storage
				L= 130.0'
		88	2 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Discarded	367.00'	10.0	00 in/hr Exfiltration over Surface area
#2	Primary	369.50'	4.0'	long x 4.0' breadth Broad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

**Discarded OutFlow** Max=0.18 cfs @ 13.50 hrs HW=367.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=367.00' TW=363.04' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond 14P: TRENCH 2

Inflow Area =	9,050 sf, 38.69% Impervious,	Inflow Depth > 1.83" for 25-YR event
Inflow =	0.39 cfs @ 12.09 hrs, Volume=	1,383 cf
Outflow =	0.14 cfs @ 12.44 hrs, Volume=	1,385 cf, Atten= 64%, Lag= 21.0 min
Discarded =	0.14 cfs @ 12.44 hrs, Volume=	1,385 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 362.77' @ 12.35 hrs Surf.Area= 600 sf Storage= 197 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 6.3 min (752.0 - 745.8)

Volume	Invert	Avail.Storag	ge Storage Description		
#1	362.00'	384	cf 4.00'W x 120.00'L x 2.00'H Prismatoid		
#2	362.25'	94	960 cf Overall x 40.0% Voids cf <b>12.0" Round Pipe Storage</b> L= 120.0'		
	478 cf Total Available Storage				
Device	Routing	Invert C	Outlet Devices		
#1 #2	Discarded Primary	363.50' 4 2 0	<b>10.000 in/hr Exfiltration over Surface area 4.0' long x 4.0' breadth Broad-Crested Rectangular Weir</b> Head (feet)       0.20       0.40       0.60       0.80       1.00       1.40       1.60       1.80       2.00         2.50       3.00       3.50       4.00       4.50       5.00       5.50         Coef. (English)       2.38       2.54       2.69       2.68       2.67       2.65       2.66       2.68         2.72       2.73       2.76       2.79       2.88       3.07       3.32		

**Discarded OutFlow** Max=0.14 cfs @ 12.44 hrs HW=362.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=362.00' TW=0.00' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond 15P: CB3

Inflow Area =	48,423 sf, 49.74% Impervious,	Inflow Depth > 1.65" for 25-YR event
Inflow =	1.98 cfs @ 12.09 hrs, Volume=	6,646 cf
Outflow =	1.98 cfs @ 12.09 hrs, Volume=	6,646 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.98 cfs @ 12.09 hrs, Volume=	6,646 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.90' @ 12.51 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.50'	<b>12.0" Round Culvert</b> L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 362.50' / 362.50' S= 0.0000 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.93 cfs @ 12.09 hrs HW=363.49' TW=363.06' (Dynamic Tailwater)

#### Summary for Link 11L: DNODE

Inflow Area =	75,876 sf, 38.63% Impervious,	Inflow Depth > 0.13" for 25-YR event
Inflow =	0.19 cfs @ 12.09 hrs, Volume=	841 cf
Primary =	0.19 cfs @ 12.09 hrs, Volume=	841 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment 1S: EXISTING

Runoff = 0.23 cfs @ 12.10 hrs, Volume= 1,196 cf, Depth> 0.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

Α	rea (sf)	CN	Description				
	1,385	98	Roofs, HSC	Roofs, HSG A			
	340	98	Paved park	ing, HSG A			
	2,591	30	Woods, Go	od, HSG A			
	14,087	39	>75% Gras	>75% Grass cover, Good, HSG A			
	18,403		Weighted A	verage			
	16,678	38	90.63% Per	rvious Area			
	1,725	98	9.37% Impe	ervious Area	a		
Tc	Length	Slop	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)			
3.9	50	0.055	0 0.21		Sheet Flow,		
					Grass: Short n= 0.150 P2= 2.90"		
0.7	183	0.090	0 4.50		Shallow Concentrated Flow,		
					Grassed Waterway Kv= 15.0 fps		
4.6	233	Total,	Increased I	o minimum	1 Tc = 6.0 min		

#### Summary for Subcatchment 2S: TO CB 2

Runoff = 1.16 cfs @ 12.09 hrs, Volume= 3,903 cf, Depth> 5.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

A	rea (sf)	CN I	Description			
	8,850	98 I	Roofs, HSG A			
	8,850	98 ⁻	100.00% In	pervious A	Area	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
6.0					Direct Entry,	

#### Summary for Subcatchment 3S: (new Subcat)

Runoff =	1.18 cfs @	12.09 hrs, Volume=	3,960 cf, Depth> 5.29"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

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Area (sf)	CN	Description		
8,979	98	Roofs, HSG	àΑ	
8,979	98	100.00% Im	pervious A	rea
Tc Length (min) (feet)	Slop (ft/ft		Capacity (cfs)	Description
6.0				Direct Entry,

#### Summary for Subcatchment 4S: EXIST TO THE SOUTH

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 3,449 cf, Depth> 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

Area (sf)	CN	Description
22,564	39	>75% Grass cover, Good, HSG A
3,468	98	Paved parking, HSG A
1,772	30	Woods, Good, HSG A
2,790	98	Roofs, HSG A
30,594		Weighted Average
24,336	38	79.55% Pervious Area
6,258	98	20.45% Impervious Area
Tc Length (min) (feet)	Sloj (ft/	
6.0		Direct Entry,

### Summary for Subcatchment 15S: HALF ROOF

Runoff	=	0.46 cfs @	12.09 hrs,	Volume=	1,712 cf,	Depth>	2.27"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 50-YR Rainfall=5.96"

A	rea (sf)	CN	Description	Description				
	5,549	39	>75% Gras	s cover, Go	ood, HSG A			
	3,160	98	Roofs, HSC	λA				
	341	98	Paved park	ing, HSG A	A			
	9,050		Weighted A	Weighted Average				
	5,549	39	61.31% Per	61.31% Pervious Area				
	3,501	98	38.69% Imp	38.69% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
6.0					Direct Entry,			

### Summary for Pond 8P: CB1

Inflow Area =	30,594 sf, 20.45% Impervious,	Inflow Depth = 0.02" for 50-YR event
Inflow =	0.12 cfs @ 12.48 hrs, Volume=	60 cf
Outflow =	0.12 cfs @ 12.48 hrs, Volume=	60 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.12 cfs @ 12.48 hrs, Volume=	60 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 364.19' @ 12.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	363.04'	<b>12.0" Round Culvert</b> L= 51.0' Ke= 0.500 Inlet / Outlet Invert= 363.04' / 362.79' S= 0.0049 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.48 hrs HW=364.18' TW=364.15' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.53 cfs @ 0.74 fps)

#### Summary for Pond 9P: CB2

Inflow Area	1 =	39,444 sf	, 38.30% Impervious	Inflow Depth >	1.21"	for 50-YR event
Inflow	=	1.16 cfs @	12.09 hrs, Volume=	3,963 cf		
Outflow	=	1.16 cfs @	12.09 hrs, Volume=	3,963 cf,	Atten=	= 0%, Lag= 0.0 min
Primary	=	1.16 cfs @	12.09 hrs, Volume=	3,963 cf		

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 364.19' @ 12.38 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.69'	Inlet / Outlet Invert= 362.69' / 362.50' S= 0.0050 '/' Cc= 0.900
			<b>12.0" Round Culvert</b> L= 38.0' Ke= 0.500

Primary OutFlow Max=0.63 cfs @ 12.09 hrs HW=363.65' TW=363.60' (Dynamic Tailwater)

#### Summary for Pond 12P: ADS UG STORAGE

Inflow Area =	48,423 sf, 49.74% Impervious,	Inflow Depth > 1.96" for 50-YR event
Inflow =	2.33 cfs @ 12.09 hrs, Volume=	7,923 cf
Outflow =	0.98 cfs @ 12.31 hrs, Volume=	7,939 cf, Atten= 58%, Lag= 13.2 min
Discarded =	0.47 cfs @ 11.90 hrs, Volume=	7,520 cf
Primary =	0.51 cfs @ 12.31 hrs, Volume=	419 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 364.14' @ 12.31 hrs Surf.Area= 2,013 sf Storage= 1,626 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 16.5 min (750.9 - 734.3)

#### POST

Type III 24-hr 50-YR Rainfall=5.96" Printed 6/21/2021 Page 19

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Volumo	les cort	Avail Sta	raga	e Starage Description			
Volume	Invert	Avail.Sto	rage	e Storage Description			
#1	362.30'	1,6	59 cf	f Custom Stage Data (Prismatic) Listed below (Recalc)			
		,		4,400 cf Overall - 228 cf Embedded = 4,172 cf x 40.0% Voids			
#2	362.50'	2	28 cf				
<i>π</i> <u></u>	002.00		L=290.0'				
#3	362.50' 50 cf 4.00'D x 4.00'H Vertical Cone/Cylinder						
		1.94	17 cf	f Total Available Storage			
		<b>,</b> -	-				
Elevatio		rf.Area	Inc	nc.Store Cum.Store			
(fee	et)	(sq-ft)	(CUDI	bic-feet) (cubic-feet)			
362.3	80	2,000		0 0			
364.5	50	2,000		4,400 4,400			
	-	_,		.,			
Device	Routing	Invert	Outl	utlet Devices			
	<u>v</u>						
#1	Discarded	362.30'		0.000 in/hr Exfiltration over Surface area			
#2	Device 3	364.00'	12.0	2.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#3	Primary	362.40'	12.0	2.0" Round Culvert L= 16.0' Ke= 0.500			
	,		Inlet	let / Outlet Invert= 362.40' / 362.24' S= 0.0100 '/' Cc= 0.900			
				= 0.013, Flow Area= 0.79 sf			
			n= t	= 0.013, FIUW AIEa = 0.13 SI			
Discarde	ed OutFlow	Max=0.47 cf	s @ 1	11.90 hrs HW=362.55' (Free Discharge)			

**1=Exfiltration** (Exfiltration Controls 0.47 cfs)

**Primary OutFlow** Max=0.50 cfs @ 12.31 hrs HW=364.13' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Passes 0.50 cfs of 4.20 cfs potential flow)

**1−2=Orifice/Grate** (Weir Controls 0.50 cfs @ 1.20 fps)

#### Summary for Pond 13P: TRENCH 1

Inflow Area =	30,594 sf, 20.45% Impervious,	Inflow Depth > 1.35" for 50-YR event
Inflow =	0.83 cfs @ 12.09 hrs, Volume=	3,449 cf
Outflow =	0.27 cfs @ 12.48 hrs, Volume=	3,449 cf, Atten= 67%, Lag= 23.4 min
Discarded =	0.18 cfs @ 12.04 hrs, Volume=	3,390 cf
Primary =	0.12 cfs @ 12.48 hrs, Volume=	60 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 369.55' @ 12.48 hrs Surf.Area= 650 sf Storage= 766 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 28.4 min (794.2 - 765.8)

Volume	Invert	Avail.Stor	age	Storage Description
#1	367.00'	78	80 cf	5.00'W x 130.00'L x 3.00'H Prismatoid
				1,950 cf Overall x 40.0% Voids
#2	367.25'	10	)2 cf	12.0" Round Pipe Storage
				L= 130.0'
		88	32 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Discarded	367.00'	10.0	00 in/hr Exfiltration over Surface area
#2	Primary	369.50'	4.0'	long x 4.0' breadth Broad-Crested Rectangular Weir

POST					
Prepared by HP					
HydroCAD® 10.10-4a	s/n 11004	© 2020	<b>HvdroCAD</b>	Software	Solutions LLC

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

**Discarded OutFlow** Max=0.18 cfs @ 12.04 hrs HW=367.72' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=0.11 cfs @ 12.48 hrs HW=369.55' TW=364.18' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.11 cfs @ 0.54 fps)

#### Summary for Pond 14P: TRENCH 2

Inflow Area =	9,050 sf, 38.69% Impervious,	Inflow Depth > 2.27" for 50-YR event
Inflow =	0.46 cfs @ 12.09 hrs, Volume=	1,712 cf
Outflow =	0.14 cfs @ 12.96 hrs, Volume=	1,715 cf, Atten= 70%, Lag= 52.3 min
Discarded =	0.14 cfs @ 12.96 hrs, Volume=	1,715 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 363.13' @ 12.47 hrs Surf.Area= 558 sf Storage= 305 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 10.9 min (760.5 - 749.6)

Volume	Invert	Avail.Stor	age	Storage Description
#1	362.00'	38	4 cf	4.00'W x 120.00'L x 2.00'H Prismatoid
#2	362.25'	9	4 cf	960 cf Overall x 40.0% Voids <b>12.0" Round Pipe Storage</b> L= 120.0'
		47	'8 cf	Total Available Storage
Device #1 #2	Routing Discarded Primary	Invert 362.00' 363.50'	<b>10.0</b> <b>4.0'</b> Hea 2.50 Coe	et Devices <b>00 in/hr Exfiltration over Surface area</b> <b>long x 4.0' breadth Broad-Crested Rectangular Weir</b> d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 0 3.00 3.50 4.00 4.50 5.00 5.50 f. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2 2.73 2.76 2.79 2.88 3.07 3.32

**Discarded OutFlow** Max=0.14 cfs @ 12.96 hrs HW=362.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=362.00' TW=0.00' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond 15P: CB3

Inflow Area	a =	48,423 sf, 49.74% Impervious, Inflo	ow Depth > 1.96" for 50-YR event
Inflow	=	2.33 cfs @ 12.09 hrs, Volume=	7,923 cf
Outflow	=	2.33 cfs @ 12.09 hrs, Volume=	7,923 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.33 cfs @ 12.09 hrs, Volume=	7,923 cf

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 364.18' @ 12.34 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	362.50'	<b>12.0" Round Culvert</b> L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 362.50' / 362.50' S= 0.0000 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=2.06 cfs @ 12.09 hrs HW=363.60' TW=363.31' (Dynamic Tailwater)

#### Summary for Link 11L: DNODE

Inflow Area =	75,876 sf, 38.63% Impervious,	Inflow Depth > 0.26" for 5	50-YR event
Inflow =	0.66 cfs @ 12.30 hrs, Volume=	1,615 cf	
Primary =	0.66 cfs @ 12.30 hrs, Volume=	1,615 cf, Atten= 0%,	, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

A Condominium Site Plan Tax Map 102 Lots 34 & 35 Hopkinton, NH

# **Operation and Maintenance**

# **1.0** Intent of this Plan:

The intent of this plan is to insure that all drainage systems designed, constructed, and approved by the NH Department of Environmental Services Alteration of Terrain Bureau be properly maintained such that there is no detrimental effects, including obstructions, erosion, redirected flow patterns, or any other adverse condition caused by stormwater runoff.

# 2.0 Plan Coordinator and Responsibilities:

Plan Coordinator contact: Baystone Properties LLC 126 Barton Corner Road Hopkinton NH 03229

The Plan coordinator's duties include the following:

- Implement the Plan with the aid of support personnel;
- Oversee maintenance practices on the site;
- Conduct or provide for inspection and monitoring activities;
- Maintain records of maintenance activities; and
- Identify any deficiencies on the site and make sure they are corrected; and

To aid in the implementation of the plan, all personnel will ensure that all housekeeping and monitoring procedures are implemented and will ensure the integrity of the site drainage facilities.

# 3.0 Stormwater Management Controls

The following provides a list of recommendations and guidelines for managing the stormwater controls:

#### Landscaped Areas – Fertilizer Management

*Function* – Fertilizer management involves controlling the rate, timing and method of fertilizer application so that the nutrients are taken up by the plants thereby reducing the chance of polluting the surface and ground waters. Fertilizer management can be effective in reducing the amounts of phosphorus and nitrogen in runoff from landscaped areas, particularly lawns. Soil tests shall be conducted to determine fertilizer application rates.

#### Maintenance

- Have the soil tested by your landscaper or local Soil Conservation Service for nutrient requirements and follow the recommendations.
- Do not apply fertilizer to frozen ground.
- Clean up any fertilizer spills.
- Do not allow fertilizer to be broadcast into water bodies.

• When fertilizing a lawn, water thoroughly, but do not create a situation where water runs off the surface of the lawn.

#### Landscaped Areas – Litter Control

*Function* – Landscaped areas tend to filter debris and contaminates that may block drainage systems and pollute the surface and ground waters.

#### Maintenance

- Litter Control and lawn maintenance involves removing litter such as trash, leaves, lawn clippings, pet wastes, oil and chemicals from streets, parking lots, and lawns before materials are transported into surface waters.
- Litter control shall be implemented as part of the grounds maintenance program.

#### **Deep Sump Catch basins & Drainage Pipes**

*Function* – Culverts deep sump catch basis is to convey stormwater away from buildings, walkways, and parking areas and trap sediment within the deep sumps prior to entering the infiltration system.

#### Maintenance

Culverts and Deep Sump Catch basins shall be inspected semi-annually, or more often as needed, for accumulation of debris and structural integrity. Leaves and other debris shall be removed from the inlet and outlet and sumps to insure the functionality of drainage structures. Debris shall be disposed of on site where it will not concentrate back at the drainage structures or at a solid waste disposal facility.

#### - Underground detention pond system.

Step 1 Inspect inlet for sediment build up in manhole sump

- Check for standing water in system
- If sediment is at or above 3" clean out inlet manholes

#### **De-Icing Chemical Use and Storage**

*Function* – Salt and sand is used for de-icing of walkways, parking lots and drives. Care shall be taken to prevent the over-application of salt for melting ice. Care shall be used with sanding in order to minimize sediment build up in manholes.

#### Maintenance

• Proper storage of salt is critical. Salt is highly water-soluble. Contamination of wetlands and other sensitive areas can occur when salt is stored in open areas. Salt piles shall be covered at all times if not stored in a shed. Runoff from stockpiles shall be contained to keep the runoff from entering the drainage system.

• When parking lots and walkways are free of snow and ice, they shall be swept clean. Disposal shall be in a solid waste disposal facility.

#### **Grass Lined Conveyance Swales / Infiltration Ditches**

*Function* – These swales promote sedimentation, filtration and infiltration of stormwater runoff.

Maintenance

- Periodically mow embankments (one to three times annually). Do not cut shorter than 4 inches.
- Inspect annually for erosion, sediment accumulation, vegetation loss and invasive species. Remove any accumulated sediment or debris.
- Repair any eroded areas, remove invasive species and dead vegetation, reseed as needed
- Ensure stone ditch if free of debris and sediment

# 4.0 Safety

Keep safety considerations at the forefront of inspection procedures at all times. Likely hazards should be anticipated and avoided. Never enter a confined space (outlet structure, manhole, etc) without proper training or equipment. A confined space should never be entered without at least one additional person present.

### 5.0 Inspection and Maintenance Procedures

Visual inspections of all areas of the site will be performed as needed throughout the year, but no less than once in the spring after snow melt-off, once in the fall, and after the end of a storm with rainfall amounts greater than one (1.0) inches. The inspection will be conducted by the Plan coordinator or designated personnel. The inspection will verify that the site drainage as shown on the plan is in good condition, and that there are no erosion problems developing on the slopes or the drainage systems. Any required repairs will be initiated as soon as possible.

# 6.0 Record Keeping

An Inspection and Maintenance Report will be prepared for each inspection performed throughout the year, but no less than once in the spring after snow melt-off, once in the fall. A copy of the report form to be completed is provided herein. Completed forms will be maintained at the facility, or with the Plan Coordinator.

All record keeping required by this I&M Plan shall be maintained by the responsible parties and made available upon request.

Should ownership of the property be transferred, the new owner(s) shall assume responsibility for this Plan.

____

# **Inspection Report**

General Information				
Facility Name:	Baystone Property			
Location:	Hopkinton, NH			
Date of Inspection				
Inspector's Name				

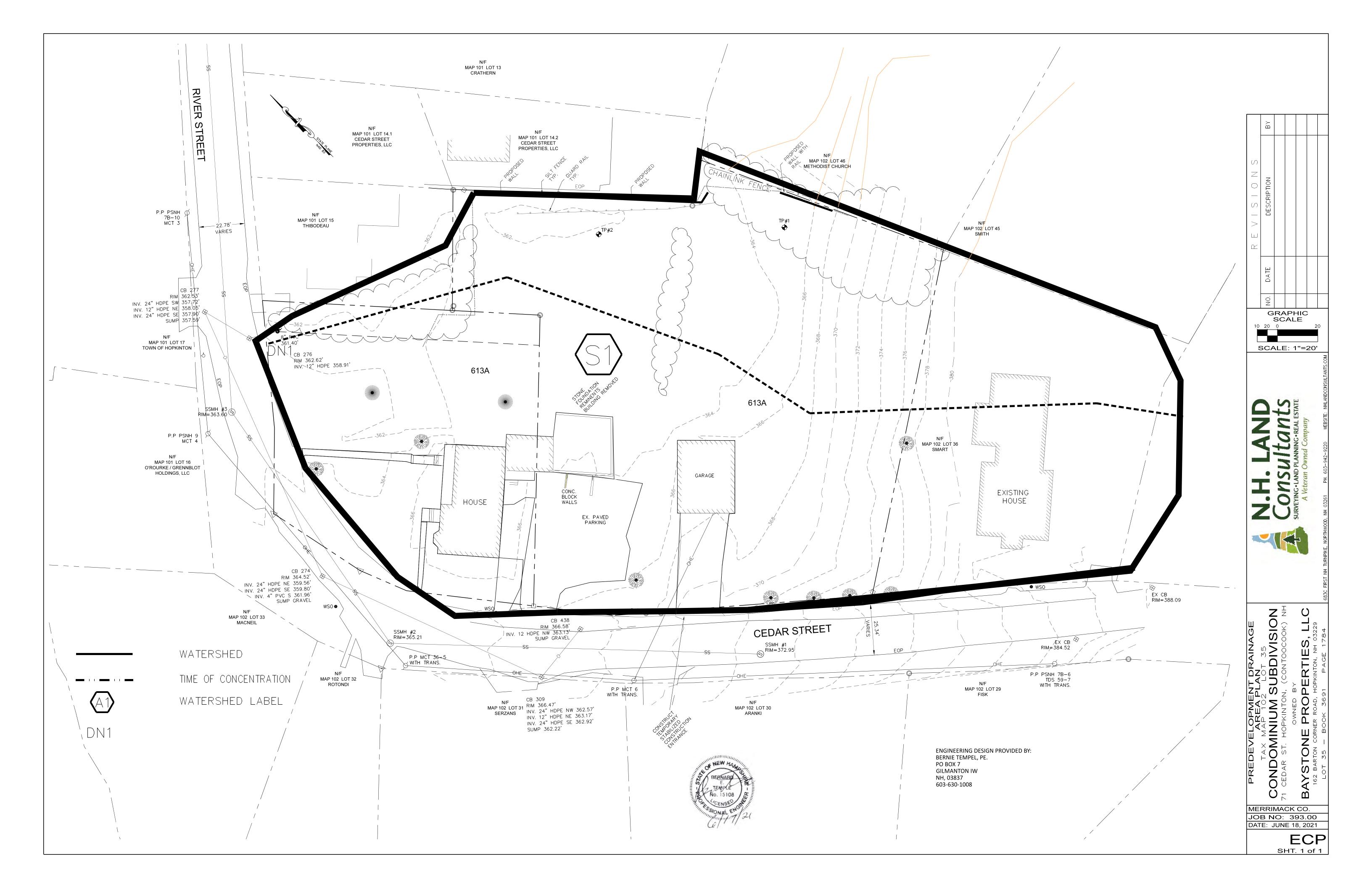
# **Overall Site Drainage Issues:**

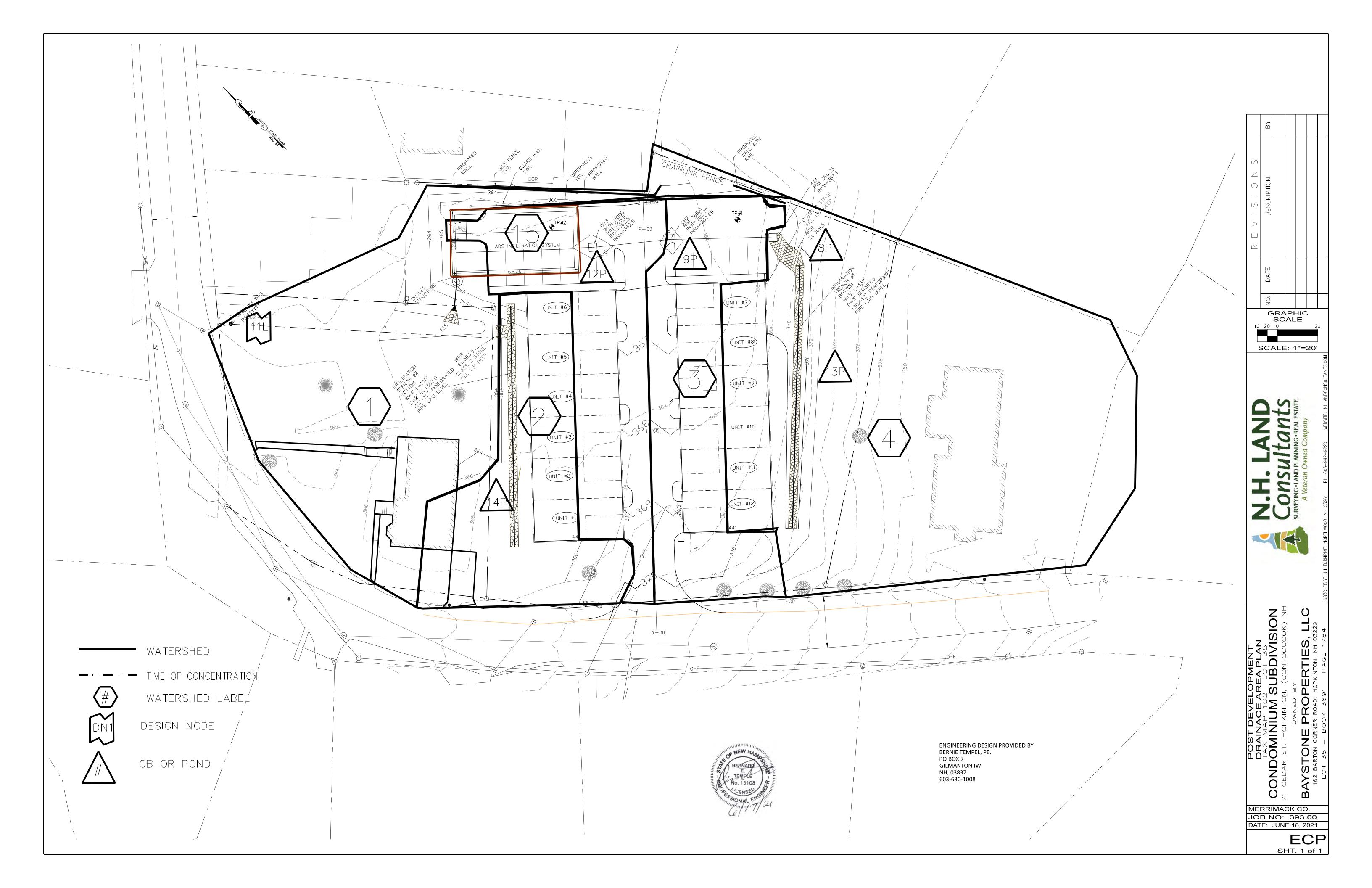
	Source	Adequa	ate?	Maintenance		Corrective Action needed and notes:
				Require	ed?	
1	Are all slopes stable showing no signs of erosion?	□Yes	□No	□Yes	□No	
2	Are ditches, swales, culverts, inlets, and outlets flowing freely?	□Yes	□No	□Yes	□No	
3	Is there any sediment buildup in ditches, swales, or culverts?	□Yes	□No	□Yes	□No	
4	Are catch basin sumps clean of sediment buildup?	□Yes	□No	□Yes	□No	
5	Are detention basins functioning properly?	□Yes	□No	□Yes	□No	
6.	Underground infiltration basins functioning properly?	□Yes	□No	□Yes	□No	
7	Does the site drainage comply with the intent of the I&M Plan	□Yes	□No	□Yes	□No	

Describe any other issues requiring attention not described above:

A Condominium Site Plan Tax Map 102 Lots 34 & 35 Hopkinton, NH

# DRAINAGE AREA PLANS





# Other Business:

- Matt Moynihan, CNHRPC Land Use Chapter *
   Zoning Amendments (Discussion)
- Comments, if any Lynch Driveway Wetland Crossing.

*pending receipt of draft chapter from CNHRPC.

# PROPOSED OBJECTIVES & RECOMMENDATIONS - EXISTING AND FUTURE LAND USE

#### OBJECTIVE 1:

Continue to support a mix of uses in the Harts Corner/West Hopkinton/Maple Street area

- → Promote mixed-use development opportunities in the Commerce and Community Overlay District (CCOD) in West Hopkinton along NH127 (Maple Street) to the north of the US202/NH9/NH127 intersection. Support and promote the appropriate inclusion of multi-family housing as an element of future development proposals
- → (1) Continue efforts to evaluate potential water/sewer service extensions on Maple Street. (2) Utilize the Harts' Corner and Exit 6 Tax Increment Financing (TIF) districts as appropriate to foster mixed use and commercial development and extend water/sewer service as necessary.
- → Promote the availability and use of the Exit 6 Economic Revitalization Zone (ERZ).

#### **OBJECTIVE 2:**

Continue to support actions to revitalize Contoocook Village

- → Continue to support mixed uses and appropriate densities in Contoocook Village.
- → Review and update as appropriate the streetscape design recommendations included in 2000 Contoocook Village Charette. Consider future improvements to the Park Avenue/Pine Street and Fountain Square intersections.

- → Promote the availability and use Contoocook Community Revitalization Tax Relief Incentive (RSA 79-e) and Contoocook Village Economic Revitalization Zone (ERZ).
- → Expand the existing sidewalk networks in Contoocook and Hopkinton villages and promote pedestrian and bicycle transportation in the community.

**OBJECTIVE 3:** Support and promote appropriate commercial and mixed-use development in suitable areas of Hopkinton

- → Examine options to modify permitted uses and zone boundaries within the Burnham-Intervale M-1 zone.
- → Consider the adoption of the Commercial and Industrial Construction Exemption (NH RSA 72: 80-83) within select M-1 zone areas.
- → Consider future zoning changes (Hopkinton Village Gateway) in the vicinity of the US202/Exit 4 Intersection in coordination with possible future improvements to the intersection.

#### **OBJECTIVE 4:**

Utilize incentives and simplify regulations to encourage their use as appropriate

- → Simplify the application procedures for Conservation Subdivisions, notably the calculation of the base number of buildings and dwelling units.
- → Consider the development of an open space ranking system that provides higher allowable densities when protecting more desirable open spaces.

- → Encourage Conservation Subdivision developments that specifically address the issues of affordability by way of a variety of housing type. Consider the enhancement of density incentives specifically for developments that include smaller homes (two and three bedrooms); ranch style homes with one floor; sale of units to Hopkinton seniors who are downsizing; or first-time home buyers.
- → Consider the development of incentives such as a reduced front setback for the incorporation of architectural standards in design

# Town of Hopkinton Planning Department



330 Main Street, Hopkinton NH 03229-2627 - (603) 746-8243 -planzone@hopkinton-nh.gov

Date: July 29, 2021	
From: Karen Robertson, Planning Director	
To: Hopkinton Planning Board	

- 1. Number of Dwelling-Units, Multi-Family Amend 4.4.7 and 8.5.2 to make consistent with Table of Uses 3.6.A and/or reconsider the total number of units permitted in a multi-family dwelling, including property density requirements under 3.6.A, 4.2, 4.3, 4.4.7, and 8.5.2.
- 2. Noise Ordinance, Section V Update criteria and restrictions to make consistent with current industry standards. Adopted 1988
- Parking Requirements, Section VI Review and revise where needed so consistent with industry parking size, design requirements, and consistent with Site Plan Review Regulations, Section IX. For example, electric vehicle charging stations. Adopted 1988, amended 1994, 1995, 2001, 2002, 2003, and 2016.
- **4.** Conservation Subdivision, Section VIII Simplify formula and prioritize types of open space as an incentive. Consider the design standards and incentives offered in Section XIX.
- 5. Wetlands Conservation District (Overlay), Section XII Add setback and in-field delineation requirements. Adopted 1988, amended 1994, 1999, and 2016.
- 6. Growth Management and Innovative Land Use Control, Section XIII Consider how other communities address potential growth. The Ordinance cannot be based simply on believing that the Hopkinton is growing too fast or faster than surrounding towns. Hopkinton must demonstrate the lack of capacity to accommodate growth, such as *insufficient* infrastructure or *inadequate* municipal services. When reviewing, discuss Impact Fees, Off-Site Exaction Fees, and Phasing Standards. The Ordinance expires in 2022.
- Affordable Housing Innovative Land Use Control, Section XVI Amend to provide realistic housing opportunities. Consider promoting design standards and incentives offered in Section XIX to encourage developers to set aside a share of rental units or houses within the same development that is affordable to households of different incomes. Adopted 1988, Amended 1989, 1995, 2002, 2004, and 2015.
- 8. Outdoor Lighting Ordinance, Section XVIII Update criteria and restrictions to ensure consistency with current lighting types and design standards. Adopted 2005, amended 2016.



# T.F. BERNIER, INC.

Land Surveyors~Designers~Consultants

#2021-8 nvironmental Permitting State and Local Permitting Land Surveying Aerial Mapping Aerial Photography

50 Pleasant Street, P.O. Box 3464 Concord, NH 03302-3464 Tel. (603) 224-4148 Fax (603) 224-0507

July 14, 2021

Dan Rinden, Chair Hopkinton Zoning Board of Adjustment 330 Main Street Hopkinton, NH 03229 For discussion should PB wish to comment per 12.7.2. KR Scheduled for ZBA 9/07/2021 Mtg.

RE: Application for Special Exception Map 240 Lot 51 Gould Hill Road & Briar Hill Road John H. Lynch Irrevocable Trust of 2012

Dear Chair Rinden and Members of the Board:

Please find enclosed an application for a Special Exception to allow a driveway to be constructed through the Wetland Conservation Overlay District. This request is being submitted in accordance with Article 12 Section 7.2 of the Hopkinton Zoning Ordinance. The proposed driveway is being constructed over a portion of the existing access drive and will impact 510 square feet of the Wetland Conservation Overlay District.

Attached is a plan set prepared by this office showing the proposed project and the construction of the driveway.

Thank you for your time and consideration of this request. If you have any questions or need additional information, please give us a call.

Sincerely, T.F. BERNIER, INC Timothy Bernier PLS LLS CWS

Timothy Bernier PLS LLS CWS President

enclosures

cc: file 663-01



# **Town of Hopkinton**

330 Main Street • Hopkinton, New Hampshire 03229 • www.hopkinton-nh.gov Tel: 603-746-3170 Fax: 603-746-3049

HOPKINTON ZONING BOARD OF ADJUSTMENT APPLICATION FOR APPEAL Brayshaw \$ 265 CK 1309 T

Ten completed copies of the application with all supporting documentation must be submitted.

Name of Applicant; T. F. Bernier, Inc Timothy Bernier	
Mailing Address: PO Box 3464 Concord NH 03302	
Telephone (days): 603-224-4148	
Name of Property Owner: John H. Lynch Trust, Susan Lynch & William Steele Trust	stees
Mailing Address: 2 Watchtower Road Hopkinton NH 03229	31000
Telephone (days):603-219-1347	
Tax Map: 240     Lot: 51     Location of Property: Gould Hill & Bria	ar Hill Roads
Zoning of property in question (circle one): R-1 (R-2) R-3 R-4 B-1 M-1 VR-1	1 VB-1 VM-1
Section of Hopkinton Zoning Ordinance under which your application was denied or you b proposal relates to: Section: <u>IV</u> Paragraph/Table: <u>4.2 Table of Dimensional I</u> A copy of your denied Building/Use Application or administrative decision must be a	Requirements
This application is for: Variance Special Exception Equitable Waiver Admir The undersigned hereby requests a Variance, Special Exception, Equitable Waiver, and Appeal to permit the following: The construction of a driveway impacting 510 square feet of the Wetland Conserva overlay district. The driveway will provide access to the upland building area.	Administrative
<ul> <li>NOTE: Additional information may be supplied on a separate sheet if the space provided</li> <li>1. Hearing, Abutter, Notification Fees: <ul> <li>Variance - \$100.00</li> <li>Special Exception - \$100.00</li> <li>Equitable Waiver - \$100.00</li> <li>Administrative Appeal - \$100.00</li> <li>Rehearing - \$100.00</li> </ul> </li> </ul>	1=\$100.00
<ul> <li>Notification of each Owner, Applicant, Agent, Abutter – \$5.00</li> <li>Published Notice – \$75.00</li> </ul>	18=\$ 90.00 1=\$ 75.00 Total = \$265.00

2. List of names and mailing addresses of all abutters to the property as defined by NH RSA 672:3. Supply information on separate sheet. Abutter is any person whose property adjoins or is directly across the street or stream from the land under consideration.

- 3. Attach location map showing exact location of property in relation to at least one prominent landmark (road junction, business, town building, etc.). Include north arrow and label road names. Indicate with an X the location of the property in question.
- 4. Attach site plan of property showing: Boundaries and area of parcel; north point, scale, legend, and location, size and type of all existing and proposed buildings, uses, parking, signs, roadways, screening, etc. Map submitted to included one full-size and ten 11" x 17" or less.
- 5. List provisions to be made for septic disposal, fire protection, water supply, parking, noise, smoke, surface drainage, etc. Supply information on separate sheet.
- 6. Letter of Authorization to allow an Agent or Attorney to represent Applicant, if applicable.
- 7. Copy of property deed of the subject property.
- 8. Any other pertinent information that you feel the Board may need to assist in their decisionmaking process.

You must appear at the public hearing or be presented by an authorized agent or attorney for the Board to act on your application. The application will be terminated or tabled for failure to appear at a scheduled public hearing, without first providing written notification to the Planning Department.

You are fully responsible for researching and knowing any and all laws, which may be applicable and affect the outcome of the Board's decision on your application request. The Town of Hopkinton assumes no responsibility or liability relating to your failure to research and know all applicable laws including, but not limited to, state, federal and local laws, codes, land development regulations and comprehensive plan. The Town of Hopkinton strongly encourages all applicants to consider consulting an attorney regarding their application.

You are encouraged to review the attached Rules of Procedures used by the Board of Adjustment at the public hearing.

I/we being duly sworn, depose and say that I am/We are the owner(s)/lessee(s) of land included in the application and that the foregoing statements herein contained and attached, and information or attached exhibits thoroughly to the best of my/our ability represent the arguments on behalf of the application herewith submitted and that the statements and attached exhibits referred to are in all respect true and correct to the best of my/or knowledge and belief.

In addition, I/We understand this application must be filed with all pertinent information as it pertains to the requirements of the Town of Hopkinton Zoning Ordinance and all other information requested or required by the Zoning Board of Adjustment in order to be considered complete. I/We understand that this application will not be filed until all required information has been received, and do further understand that the Town of Hopkinton reserves the right to postpone this request until such time as the requirements are met.

Furthermore, I/We understand that I/We, our representative as stated on the application, should appear at the public hearing. If photographs, documents, maps or other materials are provided to the Board as evidence at the public hearing, said evidence will become property of the Town of Hopkinton and will remain on file for future reference.

Also, I/We recognize and understand that the public hearing before the Board of Adjustment regarding land development is considered <u>quasi-judicial in nature</u>. State and local law strictly prohibits applicants and/or interested parties from taking part in ex-parte communications with Board members in person, by phone, e-mail, or in writing before the application is discussed at a public hearing.

# Town of Hopkinton Planning Department



330 Main Street, Hopkinton NH 03229-2627 - (603) 746-8243 -planzone@hopkinton-nh.gov

# **BUILDING/USE APPLICATION**

Completed application <u>must be returned to the Planning Office by 12 Noon on Friday</u>. The application must be accompanied by a check payable to the Town of Hopkinton. No refund will be made if the application is denied. More than one permit may be applied for using the same form; however, the permits being sought must apply to the same piece of property. For questions, please contact the Planning Department at (603) 746-8243 or email planzone@hopkinton-nh.gov.

Permit No	Phasing Applicability		Subdivision:
	Driveway Permit		Shoreland Protection
Application Received//	Septic Approval	Floodplain	Code Enforcement
By: Fee:	Fire/Life Safety	ZBA/PB	CVP Water Commission

Street Address Gould Hill Road	Tax Map 240	Tax Lot 51	Zoning District R2	
🗌 Demolition 🔲 Residential 🗌 Industr	rial 🗌 Commercial	Accessory Other		
Is Lot located in 100-year Flood Plain Area (S	Special Flood Hazard)?	Yes No		
What is the Flood Insurance Rate Map (FIRM) Community Panel #:				
is the portion of the property to be developed under a Current Land Use (CLU) Assessment? 🗌 Yes 🗌 No				
If yes, a new CLU map must be submitted to the Assessing Department (603-746-8258).				

Applicant's Name:	T. F. Bernier, Inc.	Owner's Name:	John H. Lynch Tr.,
Mailing Address:	PO Box 3464	Mailing Address:	Susan Lynch & Wm. Steele, Trustees
City/State/Zip:	Concord, NH 03302	City/State/Zip:	2 Watchtower Rd., Hopkinton, NH 03229
Phone (days):	603-224-4148	Phone (days):	603-219-1347
Email:		Email:	

Note: Please indicate whether you would prefer your PERMIT emailed or mailed (circle one).

	Name of Contractor/License #	Address/City/State/Zip	Telephone
Architect			
General Contractor			
Electrical			
Plumbing			
Sewer/Septic			
Mechanical			
Sprinkler			
Fire Alarm		· · ·	
		ESCRIPTION	

DESCRIPTION

Construction of a driveway impacting 510 SF of the Wetland Conservation Overlay District. Driveway will

provide access to the upland building area.

Value of Work: \$____

Town of Hopkinton Planning Department



330 Main Street, Hopkinton NH 03229-2627 - (603) 746-8243 -planzone@hopkinton-nh.gov

# **BUILDING/USE PERMIT DENIAL**

Permit: <u>2021-</u> Tax Map/Lot: <u>240/51</u> Name of Applicant/Owner: <u>T. F. Bernier, Inc.</u> Street Address: <u>Gould Hill Rd</u> District: <u>R2</u>

Denied: Special Exception required - 12.7.2 Wetland Conservation Overlay District.

1 Kol Karen Robertson

Planning Director

Date:_	7	Ho	a	1	
_				-	

## SECTION XII WETLANDS CONSERVATION DISTRICT (OVERLAY)

**<u>12.6.5</u> Poorly Drained Soils:** Soil series and land types commonly associated with wetlands, as described by the Soil Survey of Merrimack County, New Hampshire, include the following "**poorly drained**" soils:

Au Gres (AgA, AgB, AuB)	
Limerick variant (Lm)	
Ridgebury (RdA, RdB, RbA, RdB)	
Rumney (Ru)	

# 12.7 PERMITTED USES

**12.7.1 General:** Permitted uses are those uses which will not require the erection or construction of any structures or buildings, will not alter the natural surface configuration by the addition of fill or by dredging, and uses that otherwise are permitted by the Zoning Ordinance. Such uses may include the following:

- (a) Forestry and tree farming, using best management practices in order to protect streams from damage and to prevent sedimentation.
- (b) Cultivation and harvesting of crops according to recognized soil conservation practices, including the protection of wetlands from pollution caused by fertilizers, pesticides and herbicides used in such cultivation.
- (c) Wildlife refuges.
- (d) Parks and recreation uses consistent with the purpose and intent of this Ordinance.
- (e) Conservation areas and nature trails.
- (f) Open spaces as permitted or required by the Subdivision Regulations or the Zoning Ordinance.

**12.7.2 Special Exceptions**: Special exceptions may be granted by the Board of Adjustment, after proper public notice and public hearing, for undertaking the following uses in the Wetlands Conservation District when the application has been referred to the Planning Board and the Conservation Commission, for review and comment at least thirty (30) days prior to the hearing:

- (a) Streets, roads and other access ways and utility right-of-way easements, including power lines and pipe lines, if essential to the productive use of land not so zoned and if so located and constructed as to minimize any detrimental impact of such uses upon the wetlands.
- (b) Water impoundments.
- (c) The undertaking of a use not otherwise permitted in the Wetlands Conservation District, if it can be shown that such proposed use is not in conflict with the purposes and intentions listed in Paragraph 12.1 of this Section.

### 12.7.3 Special Provisions:

#### SPECIAL EXCEPTION (Section XV)

In order to secure a variance, the Zoning Board of Adjustment must determine by law that your Special Exception request satisfies the following criteria of the Zoning Ordinance. <u>Please provide a written response</u> along with any other supporting documentation for each of the following criteria. Please note that all criteria must be satisfied and supported by the Zoning Board of Adjustment in order for a Special Exception to be granted. Should the space provided be inadequate, please attach additional pages to this application.

- 1. Standards provided by this Ordinance for the particular use permitted by special exception. <u>The request is being made in accordance with 12.7.2 Special Exceptions: to allow an "other</u> <u>access ways" persuant to the Hopkinton Zoning Ordinance.</u>
- 2. No hazard to the public or adjacent property on account of potential fire, explosion or release of toxic materials.

The proposed driveway will not produce or cause to be released any toxic materials, and presents no risk of fire or explosion.

3. No detriment to property values in the vicinity or change in the essential characteristics of a residential neighborhood on account of the location or scale of buildings and other structures, parking areas, access ways, odor(s), smoke, gas, dust, or other pollutant, noise, glare, heat, vibration, or unsightly outdoor storage of equipment, vehicles or other materials.

The proposal is to construct a new driveway over the historically used access. The crossing of the WCOD will occur at the location of the current crossing.

4. No creation of a traffic safety hazard or a substantial increase in the level of traffic congestion in the vicinity.

5. No excessive demand on municipal services, including, but not limited to, water, sewer, waste disposal, police and fire protection, and schools.

The driveway is on private property and will be maintained by the land owner. The driveway impact to the WCOD will not result in any requirement for municipal

services of any kind

- 6. No significant increase of storm water runoff onto adjacent property or streets. <u>The crossings will be made with properly engineered culverts, designed to pass stormwater</u> <u>at its naturally occurring rate and will not either increase or decrease runoff from the property</u>.
- 7. An appropriate location for the proposed use. The driveway location was selected for safety first and then to minimize impact to the WCOD. This is consistent with state law and best environmental practice.

8. Not affect adversely the health and safety of the residents and others in the area and not be detrimental to the use or development of adjacent or neighboring properties. The crossing will have no affect on residents or others in the area.

9. In the public interest and in the spirit of the ordinance. <u>The crossing provides safe and appropriate access to the upland, non-conservation areas of</u> their property, all uses of this non conservation area will be consistent with all local and state ordinances and or regulations.



**T.F. BERNIER, INC.** Land Surveyors~Designers~Consultants

50 Pleasant Street, P.O. Box 3464 Concord, NH 03302-3464

Environmental Permitting State and Local Permitting Land Surveying Aerial Mapping Aerial Photography

> Tel. (603) 224-4148 Fax (603) 224-0507

Abutters List John H. Lynch Irrevocable Trust of 2012 Zoning Special Exception Assessors Map 240 Lot 51 Gould Hill Road & Briar Hill Road Hopkinton, NH

<u>MAP</u>	LOT	OWNER
240	51	John H. Lynch Irrevocable Trust of 2012 2 Watchtower Road Hopkinton, NH 03229
239	21	Virginia L. Pastuszczak Timothy D. Sweatt 373 Old Stagecoach Road Contoocook, NH 03229
239	22	Five Rivers Conservation Trust 10 Ferry Street Suite 311A Concord, NH 03301
240	29	Alison Josefiak Christopher Navarro 257 Gould Hill Road Contoocook, NH 03229
240	30	Thomas R. & Hilary A. Chapman 283 Gould Hill Road Contoocook, NH 03229
240	31	Bradford W. & Ann McClane Kuster 331 Gould Hill Road Contoocook, NH 03229
240	32	Jane D. W. & Frederic Bradstreet P.O. Box 149 333 Gould Hill Road Contoocook, NH 03229
240	36	Irvin D. Gordon 63 Roberts Road Hopkinton, NH 03229

240	49	April Dunn 59 Blaze Hill Road Hopkinton, NH 03229
240	52	Rix Family Trust of 2016 248 Gould Hill Road Hopkinton, NH 03229
240	54	Richard Jones Irrevocable Trust 18 Green Street Newport, NH 03773
241	38-1	Kirk Hemphill 831 Briar Hill Road Contoocook NH 03229
241	38-2	R and J Case Trust 1030 Briar Hill Road Hopkinton, NH 03229
241	43	Jeanne C. Dwyer GST Exempt Trust P.O. Box 600 Concord, NH 03302
249	4	Kurt K. & Betsey F. Rhynhart 675 Briar Hill Road Hopkinton, NH 03229
249	5	The Viking Trust 745 Briar Hill Road Contoocook, NH 03229
249	8	S. Wayne & Elizabeth A. Clarke 812 Briar Hill Road Hopkinton, NH 03229

### **Professional Consultant**

Timothy F Bernier, LLS, CWS T F Bernier, Inc. PO Box 3464 Concord NH 03302-3464 EFiled 202000019931 Recorded in Merrimack County, NH In the Records of Susan Cragin, Register BK: 3696 PG: 1739, 9/16/2020 12:15 PM LCHIP S25.00 RECORDING \$34.00 SURCHARGE \$2.00

> Return to: McLane Middleton, Professional Association RAW/ cem 59370 P.O. Box 326 Manchester, NH 03105

#### WARRANTY DEED

I, JOHN H. LYNCH, a married individual with a mailing address of 2 Watchtower Road, Hopkinton, New Hampshire 03229, grant to SUSAN E. UPTON LYNCH and WILLIAM G. STEELE, JR., CPA, Trustees of THE JOHN H. LYNCH IRREVOCABLE TRUST OF 2012, a New Hampshire trust u/d/t dated December 15, 2012, with a mailing address of 2 Watchtower Road, Hopkinton, New Hampshire 03229, with WARRANTY COVENANTS:

A certain parcel, with the improvements thereon, located in The Town of Hopkinton, County of Merrimack, State of New Hampshire, described as follows:

#### Tract I:

That portion of a certain property, located in The Town of Hopkinton, County of Merrimack, State of New Hampshire, and known as the Gage Place, which is situated westerly of the highway leading from Hopkinton Village to Tyler Station, but

EXCEPTING from the said Premises conveyed a parcel of land situated on said highway surrounding the buildings which are situated thereon, being ten (10) acres, more or less, which ten acre parcel is bounded and described as follows:

Beginning on the southerly side of the lane leading to the woodland on the westerly side of the road from Hopkinton Village to Tyler Station at the corner of the wall on the south side of said lane; westerly along this wall about five hundred and fifty (550) feet to the easterly side of an opening in that wall; thence southerly in a straight line to a point in the south line of said property at a point approximately five hundred four (504) feet west of the above-mentioned highway; easterly along said wall about five hundred four (504) feet to the highway; northerly along said highway to the point of beginning.

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Said premises are shown on a plan entitled, "TYPE MAP OF THE BRIER HILL FARM WOODLOT, JULY, 1916," filed in the Registry of Deeds as Map #750, the said granted premises being bounded and hatched in red on said plan.

The above parcel is believed but not warranted to be further described as follows:

A certain tract or parcel of land with all improvements and appurtenances situate on the east side of Gould Hill Road and westerly of Briar Hill Road in the Town of Hopkinton, County of Merrimack, and State of New Hampshire, as shown on a plan entitled, "THE GOULD HILL TRUST, WILLIAM G. STEELE, JR., TRUSTEE", prepared by Bristol, Sweet & Associates, Inc., dated September 22, 1999, recorded as Plan #14886 in the Merrimack County Registry of Deeds (the "Plan"), which tract or parcel is more particularly bounded and described as follows:

- Beginning at an intersection of two stone walls at an iron rod on the easterly sideline of Gould Hill Road at the westernmost corner of the within premises and the northwesterly corner of land now or formerly of Philip C. and Gloria F. Martin;
- running in a northerly directly along a stone wall and the easterly sideline of Gould Hill Road a distance of 986.5 feet, more or less, to an iron pipe at an intersection of stone walls at land now or formerly of Arnold C. & Alice R. Coda, (shown as Tax Lot #240-50 on the Plan), which iron pipe is North 11° 33' 55" East a distance of 983.17 feet from the previously mentioned iron rod;
- turning and running in an easterly direction along a stone wall and said Coda land a distance of 431.0 feet, more or less, to an iron pipe at an intersection of stone walls and a barbed wire fence, which iron pipe is North 87° 44' 23" East a distance of 430.88 feet from the previously mentioned iron pipe;
- turning and running along land now or formerly of Erik Leadbeater, (shown as Tax Lot #240-49 on the Plan), North 88° 00' 35" East a distance of 1071.76 feet to a 1" iron rod at the beginning of a barbed wire fence;
- turning and running still along said Leadbeater land North 07° 11' 20" West a distance of 713.15 feet to an iron rod set in a drill hole at the end of a stone wall at the end of the barbed wire fence;
- continuing along the stone wall and land of Leadbeater North 09° 22' 17" West a distance of 153.76 feet to an iron rod set in a drill hole in the stone wall at land now or formerly of Walter W. Dwyer Jr. 1998 Trust, (shown as Tax Lot #241-43 on the Plan);
- turning and running along said Dwyer Trust land North 49° 02' 30" East a distance of 448.94 feet to an iron rod at a bend in a barbed wire fence;

- turning and running still along said Dwyer Trust land, North 80° 55' 36" East a distance of 757.50 feet to a drill hole at the end of a stone wall near a corner of barbed wire fences, at land now or formerly of Mary H. Small, (shown as Tax Lot #241-38.2 on the Plan);
- turning and running along said Small land South 16° 39' 06" East a distance of 898.18 feet to a drill hole at the end of a stone wall;
- continuing along the stone wall and said Small land a distance of 469.60 feet to a drill hole in the stone wall, which drill hole is South 16° 40' 35" East, and a distance of 469.60 feet from the next previously mentioned drill hole;
- continuing along the stone wall and land now or formerly of David L. & Judith Poole, (shown as Tax Lot #241-38.1 on the Plan) a distance of 541.50 feet to a drill hole at the end of the stone wall, which drill hole is South 16° 24' 19" East, and a distance of 541.46 feet from the next previously mentioned drill hole;
- continuing in a southeasterly direction along said Poole land South 16° 36' 36" East, a distance of 82.47 feet to a drill hole at the end of a stone wall;
- continuing in a southeasterly direction along the stone wall and said Poole land a distance of 257.10 feet, more or less, to a drill hole in the stone wall, which drill hole is South 16° 20' 41" East, a distance of 256.98 feet from the next previously mentioned drill hole;
- continuing in a southeasterly direction along the stone wall and said Poole land a distance of 288.90 feet, more or less, to a drill hole at an intersection of stone walls, which drill hole is South 16° 40' 26" East, a distance of 287.63 feet from the next previously mentioned drill hole;
- turning and running in an easterly direction along a stone wall and said Poole land a distance of 392.30 feet, more or less, to a drill hole 3.85 feet easterly of a corner of stone walls at the westerly sideline of Briar Hill Road, which drill hole is North 83° 41' 42" East, a distance of 392.31 feet from the next previously mentioned drill hole;
- turning and running South 01° 05' 05" West, a distance of 45.39 feet along the westerly sideline of Briar Hill Road to a disk set in a drill hole at an intersection of stone walls at land now or formerly of Sandra Schneider, (shown as Tax Lot #249-5 on the Plan);
- turning and running in a westerly direction along a stone wall and said Schneider land a distance of 558.50 feet, more or less, to an iron pipe in a gap in the stone wall 1.86 feet westerly of the end of the stone wall, which iron pipe is

South 83° 26' 17" West, a distance of 557.61 feet from the disk referred to in the previous course;

- turning and running still along said Schneider land South 06° 37' 11" East, a distance of 1123.91 feet to an iron rod in a stone pile on a stone wall at land now or formerly of Donald & Sandra P. Saxon, (shown as Tax Lot #249-4 on the Plan);
- turning and running along a stone wall and said Saxon land North 63° 16' 34" West, a distance of 225.14 feet to a point at an intersection of stone walls at land now or formerly of Robert A. & Nancy N. Sweatt, (shown as Tax Lot #239-22 on the Plan);
- turning and running in a westerly direction along the stone wall and said Robert Sweatt land a distance of 559.60 feet, more or less, to a drill hole at an intersection of stone walls at land now or formerly of Dana L. & Alice Sweatt, (shown as Tax Lot #239-21 on the Plan), which drill hole is North 86° 26' 54" West, a distance of 559.33 feet from the next previously mentioned point of intersection of stone walls;
- turning and running along said Dana Sweatt land North 65° 05' 14" West, a distance of 690.41 feet to an iron rod in a drill hole at the end of a stone wall at land now or formerly of Martha Houston Jones Revocable Trust of 1997, (shown as Tax Lot #240-54 on the Plan);
- continuing in a northwesterly direction along the stone wall and said Jones Trust land a distance of 323.80 feet, more or less, to a drill hole at an intersection of the stone wall and a row of stones, which drill hole is North 66° 21' 18" West, a distance of 319.37 feet from the last mentioned iron rod;
- continuing in a northwesterly direction along the stone wall and said Jones Trust land a distance of 909.90 feet, more or less, to a drill hole at the end of the stone wall, which drill hole is North 64° 01' 06" West, a distance of 901.84 feet from the next previously mentioned drill hole;
- continuing in a northwesterly direction along said Jones Trust land a distance of 300.40 feet, more or less, to an iron pipe at the end of a stone wall at land now or formerly of Philip C. & Gloria F. Martin, (shown as Tax Lot #240-52 on the Plan), which iron pipe is North 63° 47' 13" West, a distance of 300.30 feet from the last mentioned drill hole;
- continuing in a northwesterly direction along the stone wall and said Martin land a distance of 648.20 feet, more or less, to the point of beginning, which point is North 68° 12' 49" West, a distance of 647.78 feet from the last mentioned iron pipe.

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#### Tract II:

A certain tract of land with the improvements situated thereon located on the northeasterly side of Gould Hill Road, in the Town of Hopkinton, County of Merrimack, State of New Hampshire, and more particularly bounded and described as follows:

- Commencing at a point marking the intersection of stone walls, which said point is 323 feet, more or less, northeasterly from the northeasterly line of the Gould Hill Road, so-called;
- running northeasterly along a stone wall and the southeasterly line of land now or formerly of Concord Kitchen Corporation (said land being formerly owned by one Shreve and by one Sweatt) and by land formerly of one Loverin, to a stake and stones marking the line of land now or formerly of one Hopkins, of one Sanborn and of one Loverin;
- running southeasterly along line of land now or formerly of Hopkins, Sanborn and Loverin and land now or formerly of the Gage heirs (said latter land being formerly owned by the Stephen Sibley heirs) to a stake and stones at corner of land of said Gage heirs (formerly Sibley heirs);
- running southwesterly along line of said land of said Gage heirs to an intersection of stone walls which said intersection is 439 feet, more or less, northeasterly from the northeasterly line of said Gould Hill Road;
- running northwesterly along land now or formerly of George L. Butterfield, Jr. and Ann S. Butterfield, 475 feet, 4 inches, more or less, to an iron pipe driven in the ground, said iron pipe lying within the right of way hereinafter described;
- continuing in the same direction 40 feet, more or less, to another iron pin driven in the ground, said iron pin marking the northeasterly corner of said right of way hereinafter described;
- continuing in the same direction along other land of said Butterfields 580 feet, 8 inches, more or less to the point of beginning.

TOGETHER WITH a RIGHT OF WAY 50 feet in width leading from Gould Hill Road to the above described land, said right of way being bounded and described as follows:

Commencing at an iron pipe driven into the ground on the northeasterly line of Gould Hill Road, which said iron pin is 540 feet, 8 inches southeasterly from the northwesterly corner of land of said Butterfields and the southwesterly corner of land of Concord Kitchen Corporation, said corners joining on the northeasterly line of said Gould Hill Road; running northeasterly through said Butterfields land to an iron pipe driven in the ground and referred to as the northeasterly corner of said right of way in the above described land;

- running southeasterly 40 feet to an iron pipe driven in the ground; and referred to as lying within said right of way in the above described land;
- continuing southeasterly an additional ten feet (10') to a point which is the southeasterly corner of said right of way;
- running southwesterly 50 feet from and parallel to the first line described in this right of way, to the northeasterly line of Gould Hill Road;

running northwesterly along said Gould Hill Road 10 feet to an iron pipe;

continuing northwesterly along said Gould Hill Road 40 feet to the point of beginning.

The Premises are conveyed together with all appurtenant rights and easements.

These premises are conveyed subject to the restriction that only a single-family residence may be constructed on the premises conveyed herein. This restriction shall run with the land and bind future grantees or successors in interest.

SUBJECT TO and TOGETHER WITH all reservations, restrictions and/or covenants, easements, liens, encumbrances and mortgages of record, if any, insofar as the same may now be in force and applicable.

MEANING AND INTENDING to describe and convey the same property conveyed to John H. Lynch by deed of John H. Lynch and Susan E. Upton Lynch as Trustees of The John H. Lynch Trust, of near or even date and recorded herewith.

This instrument was prepared from information supplied by the Grantor herein and no independent title search has been conducted.

This transfer is exempt from transfer tax pursuant to RSA 78-B:2, IX.

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Signed this 31st day of August, 2020.

JOHN H. L

#### STATE OF NEW HAMPSHIRE COUNTY OF MERRIMACK

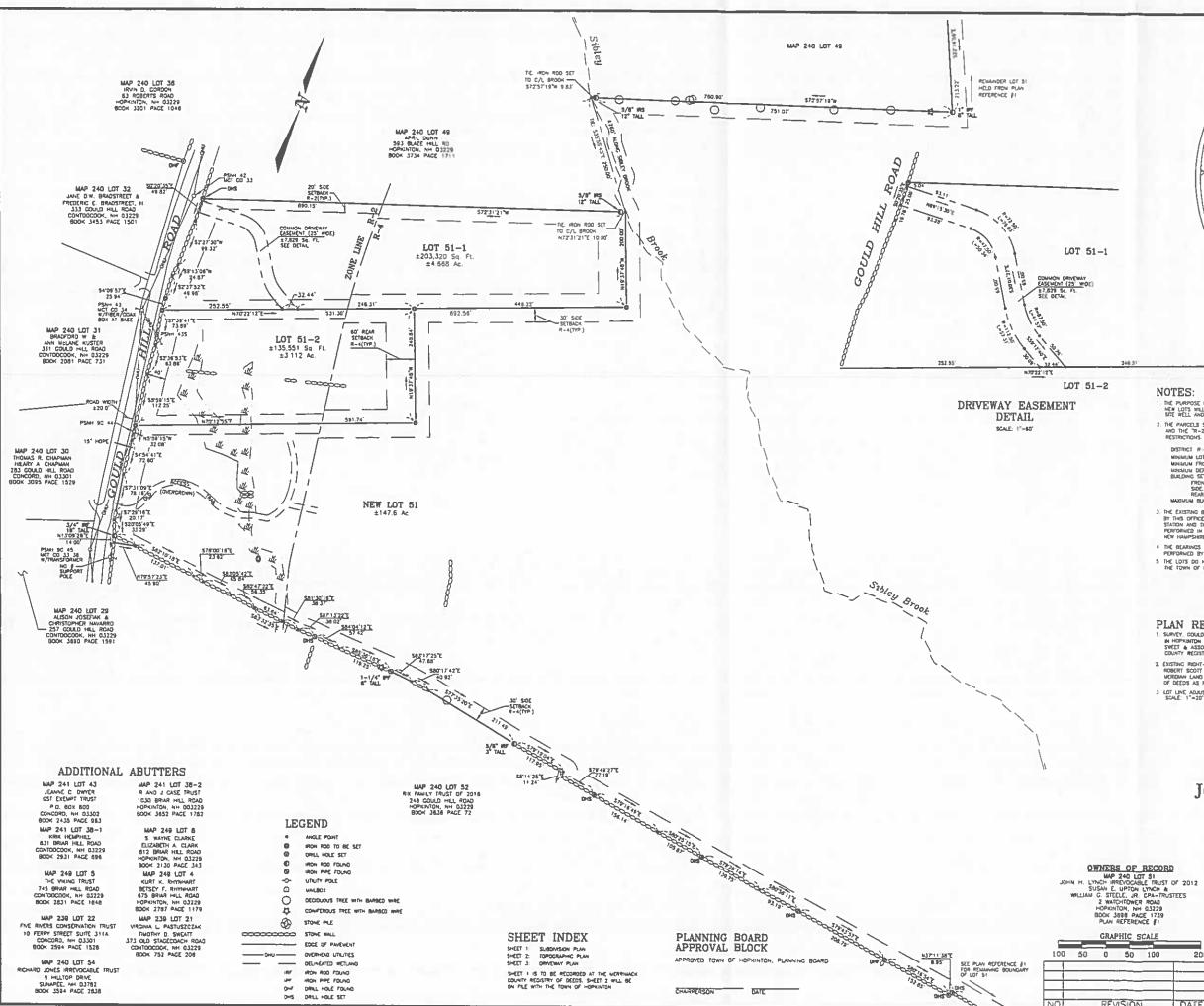
This instrument was acknowledged before me on the 31st day of August, 2020, by John H. Lynch.

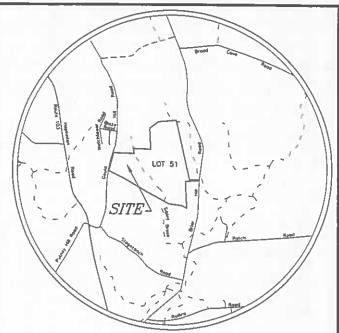
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Notary Public/Justice of the Peace Printed Name: My Commission Expires:

> ROBERT A. WELLS, Notary Willie My Commission Expires Junuary 48 202





LOCUS SCALE: 1"=2,000'

#### NOTES:

- THE PURPOSE OF THIS PUAN IS TO SUBDANDE MAP 240 LDT 31 WITO J NEW RESIDENTIAL LDTS. THE NEW LDTS WILL HAVE 250' OR MORE FRONTACE ON COULD HILL ROAD AND WILL BE SERVICED BT ON STE WELL MAD SEPTIC STREAM
- THE PARCELS SHOWN HEREON ARE LOCATED IN THE "R-4" RESIDENTIAL/AGRICULTURAL ZONING DISTRICT NO THE "R-2" MEDIUM DENSITY RESIDENTIAL DISTRICT, AND ARE SUBJECT TO THE FOLLOWING DIMENSIONA RESTRICTIONS.
  - DISTRICT R-2 MINIMUM LOT SIZE. MINIMUM FRONTAGE 60,000 S F. (UPLAND) 250' 140' NINIMUM DEPTH: BUILDING SETBACKS FRONT SIDE. REAR. AXXIVUM BUILDING HEIGHT 35'

DISTRICT R 4 MINIMUM LOT SIZE: MINIMUM DEPTH: BUILDING SETBACKS FROM SIDE: REAR MAXIMUM BUILDING HEIGHT 35'

120,000 S.F. (UPLAND) 300' 200'

- 3. THE EXISTING BOUNDARY UNES SHOWN HOREON ARE FROM PLAN REFERENCE (). A FIELD SURVEY PERFORMED BY THIS OFFICE IN JUNE 2021, AND RECORD DEEDS THE FIELD SURVEY WAS PERFORMED USING A TOTAL STATION AND THE TRAVERSE HAS AN EARCH OF CLOSURE OF ONE FART IN 13,000 OR BETTER. THE SURVEY WAS PERFORMED IN ACCORDANCE WITH THE REDURENCENTS OF A STANDARD PROPERTY SURVEY AS DEFINED IN THE MEN HAMPSHIRE CODE OF ADMINISTRATIVE RULES LON SOO.
- 4 THE BEARINGS SHOWN HEREON ARE REFERENCED TO NAU 83/11 NH STATE PLANE, BASED ON GPS OBSERVATIO PERFORMED BY THIS OFFICE IN JUNE 2021
- S THE LOTS DO NOT FAIL IN THE 100 YEAR FLOOD ZONE AS SHOWN ON THE FLOOD INSURANCE RATE WAP FOR THE TOWN OF HOPKINTON, MAP NO 33013CS10E, WITH EFFECTIVE DATE 4/19/2010.

#### PLAN REFERENCE

- SURVEY, COULD HELL TRUST, WILLAM C STEELE, JR TRUSTEE, COULD HELL ROAD & BRAR HELL ROAD IN HORKINTON N.H. TAX MAY 240-351 SCALE: 1⁺-2007, DATE: 9/22/1996 PREPARED BY BRISTOL, SYEET & ASSOCIATES, ING OF NORTH SUTTON, NEW HAMPSHIRE AND RECORDED AT THE WERRIMACK COUNTY RECISTRY OF DEEDS AS PLAN \$14886.
- 2. EXISTING RIGHT-OF-WAT PLAN DVER TAX WAP PARCEL 240-48 LAND OF JOANNE G. LUNDEN PREPARED FOR ROBERT SCOTT HOUES, HOPKNTON, NEW HAMPSHIRE, SCALE: 1"-50" DATED SEPTEMBER 22, 2003. PREPARED BY WERDIAN LAND SERVICES MIC: IN AMPERST NEW HAMPSHIRE AND RECORDED AT THE MERBAACK COUNTY REDISTRY OF DEEDS AS PLAN (1895)
- 3 LOT LINE ADJUSTMENT BETWEEN LIDTS 31 & 52 ON TAX WAP 240 COULD HILL ROAD HOPKINTON, NEW HAMPSHIRE SCALE: 1"=00" DATE: JULY 2021. PREPARED BY THIS OFFICE.

SUBDIVISION PLAN LAND OF

JOHN H. LYNCH IRREVACABLE **TRUST OF 2012** 

> ASSESSORS MAP 240 LOT 51 GOULD HILL ROAD & BRIAR HILL ROAD

HOPKINTON, NEW HAMPSHIRE SCALE: 1"=100' DATE: JULY 2021 SHEET 1 OF 3

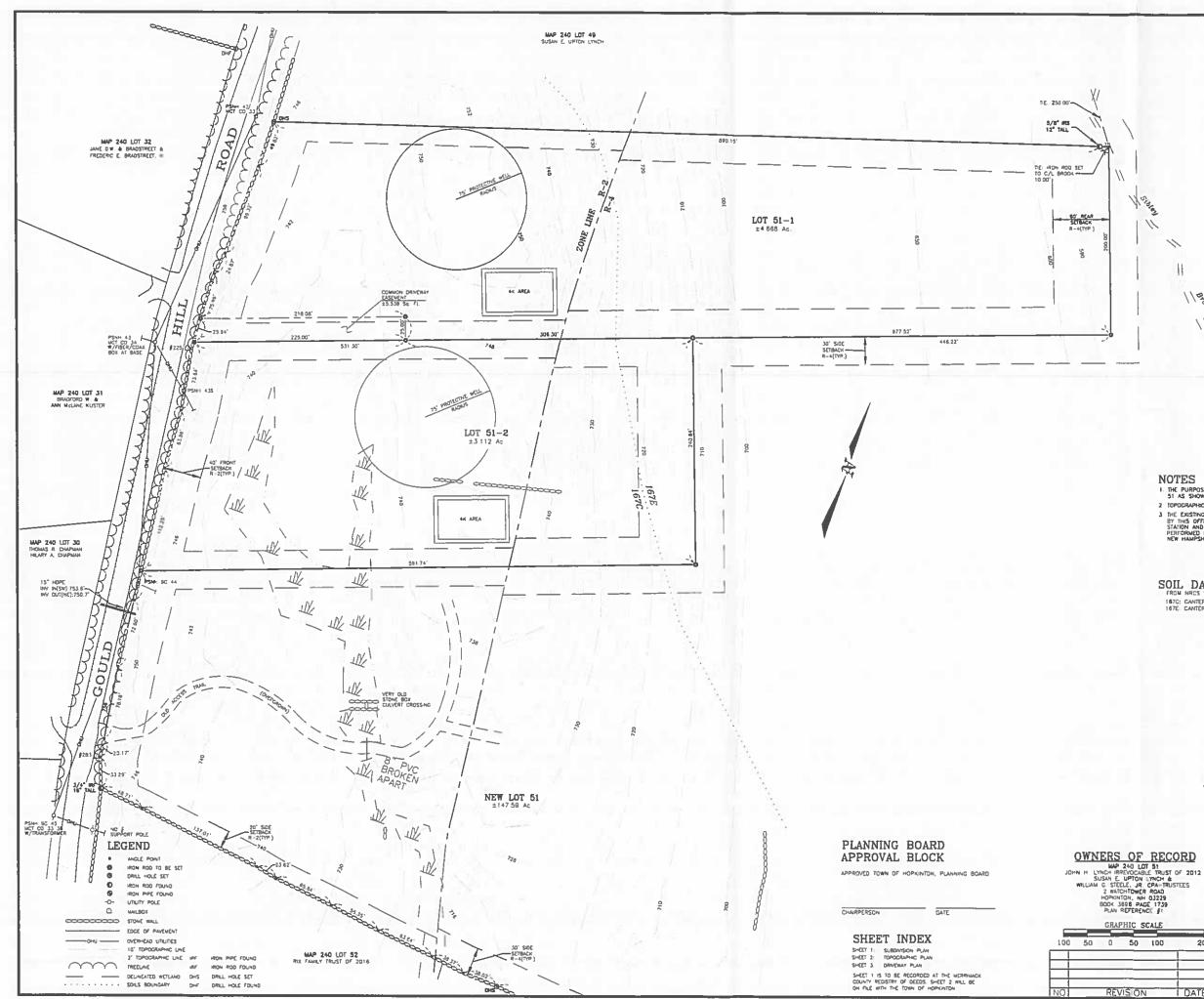
T. F. BERNIER, INC. Land Surveyors - Designers - Consultants



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GRAPHIC SCALL

50 PLEASANT STREET - P.O. BOX 3464 CONCORD, NEW HAMPSHIRE 03302-3464 Tet:(603)224-4148 - Fox:(603)224-0507 OESIGNED BY ORAWN BY CHECKID BY F.B. PG. J08 # 663-01 REVISION DATE



#### ADDITIONAL ABUTTERS

MAP 241 LOT 43 JEANNE C. DWYER MAP 241 LOT 38-1 KRIC HEMPHIL

HAP 249 LOT 5 THE VIKING TRUST

MAP 239 LOT 22 FIVE RIVERS CONSERVATION TRUST

MAP 240 LOT 54 RICHARD JONES IRREVOCABLE TRUST HAP 240 LOT 29 AUSON JOSEFIAK & CHRISTOPHER NAVARR

# MAP 241 LOT 38-2 R AND J CASE TRUST

MAP 249 LOT 8 S. WAYNE CLARKE ELIZABETH A. CLARK MAP 249 LOT 4 KURT K. RHYNHART BETSEY F. RHYNHART MAP 239 LOT 21 VIRGINA L. PASTUSZCZAK TINOTHY D. SWEATE MAP 240 LOT 36

#### NOTES

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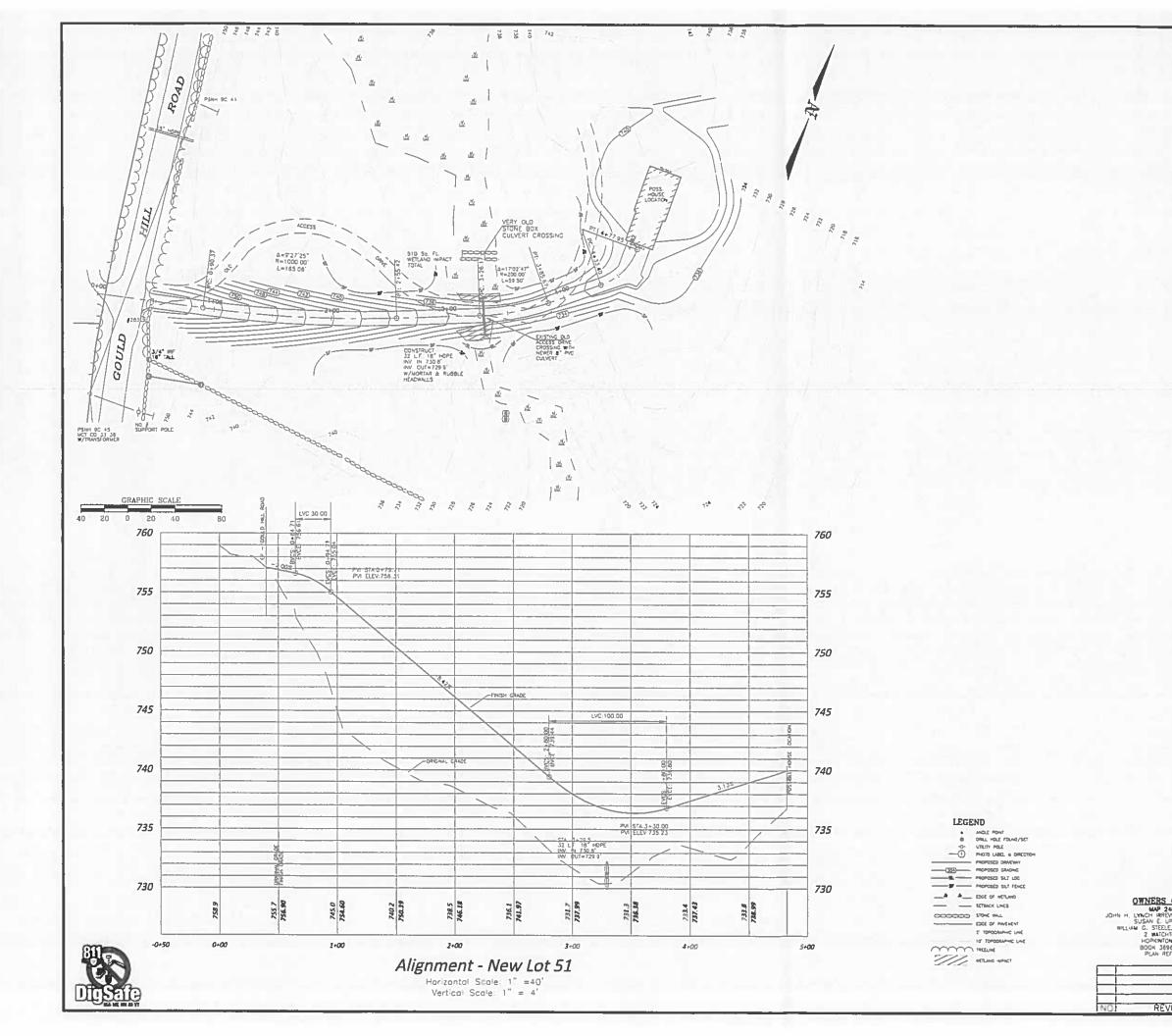
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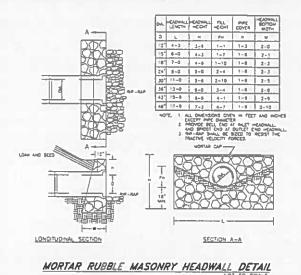
- I, THE PURPOSE OF THIS PLAN IS TO SHOW THE TOPOCRAPHIC AND PHYSICAL FEATURES ON A PORTION OF LOT 51 AS SHOWN ON ASSESSORS MAP 240 2 TOPOGRAPHIC INFORMATION SHOWN HEREON IS FROM A FIELD SURVEY PERFORMED BY THIS OFFICE IN JUNE 2021
- THE DESCRIPTION OF THE SHOWN HEREON ARE FROM PLAN REFERENCE IN A DESCRIPTION OF A DESCRIPTI

SOIL DATA FROM NRCS WE SOIL SURVEY 187C: CANTERBURY FINE SANDY LOAM, 8 TO 10 PERCENT SLOPES, VERY STORY 187C: CANTERBURY FINE SANDY LOAM, 25 TO 35 PERCENT SLOPES, VERY STORY

TOPOGRAPHIC PLAN LAND OF JOHN H. LYNCH IRREVACABLE **TRUST OF 2012** ASSESSORS MAP 240 LOT 51 GOULD HILL ROAD & BRIAR HILL ROAD HOPKINTON, NEW HAMPSHIRE SCALE: 1"=50' DATE: JULY 2021 SHEET 2 OF 3 T. F. BERNIER, INC. B Land Surveyors - Designers - Consultants 200 50 PLEASANT STREET - P.O. BOX 3484 CONCORD, NEW HAMPSHIRE 03302-3484 Tel:(603)224-4148 - Fax:(603)224-0507

		UCALINEU SY	DRAWN BY	CHECKED BY	F.B.	PG. J	38 🦸	663-01
REVISION	DATE I	769	779 BRK, JRC	TER	224	54		000-01
			DRAWING NAME					





#### NOTES:

- In de notat di had Ruin di 10 Sunde het indenditio wittund Categolard foit in witt generater die with latter in generationale understanding borden referen is the REAL of in activat. PGD Sunner water primer and in et vol. The Mankerd Showing Hostern Mit water (21): 64520 de 15% Substantias duck fin has Dirtic in winks, 201 3 PK 4983 (Chi PK Mankerd Showing Hostern Mit water (21): 64520 de 15% Substantias duck fin has Dirtic in winks, 201 4 struktud Showin reference refere touristical and Phattern // Ethiert Categola in and Aut 2021 in Accommunity with Service 31 and 16.2
- 4 VCTUARDS INCOME VEX.000 CTUBELATER IN PC. THEORY IS INTORE IN COMPLEX WITH WARRAWS WITH A VCTUARD VCTUARD

# PLANNING BOARD APPROVAL BLOCK

APPROVED TOWN OF HOPKINTON, PLANNING BOARD

CHAIRPERSON DATE

# DRIVEWAY CROSSING PLAN LAND OF JOHN H. LYNCH IRREVACABLE **TRUST OF 2012**

ASSESSORS MAP 240 LOT 51 GOULD HILL ROAD HOPKINTON, NEW HAMPSHIRE SCALE: 1"=40' DATE: JULY 2021 SHEET 3 OF 3





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